Route 1A-Vinnin Square Priority Corridor Study in Marblehead, Salem, and Swampscott









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Marblehead, Salem, and Swampscott

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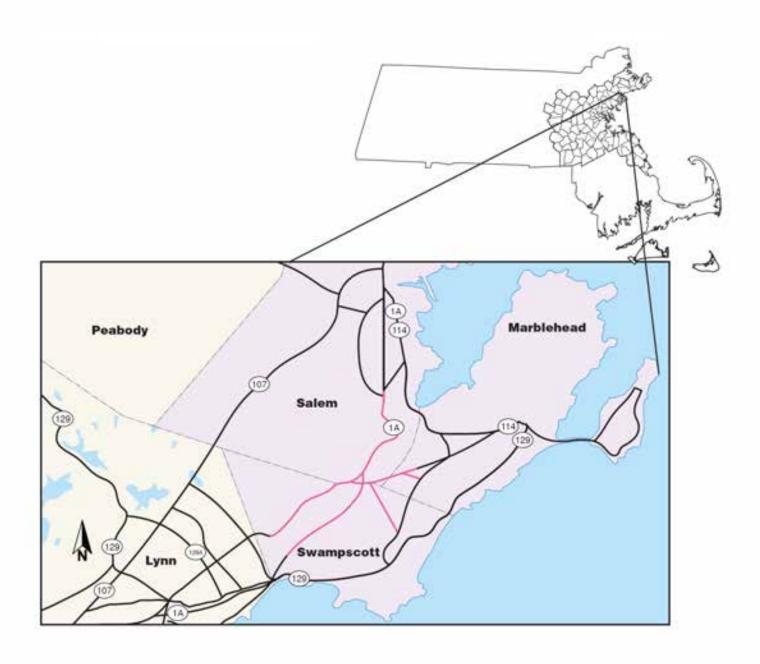
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ABSTRACT

The Boston Region Metropolitan Planning Organization (MPO) selected Route 1A, and other roadways in and surrounding the Vinnin Square area in the Town of Swampscott, City of Salem, and Town of Marblehead, as the subject of a corridor study in federal fiscal year (FFY) 2016. The *Route 1A-Vinnin Square Priority Corridor Study* focuses on one of the locations identified in a regional needs assessment—conducted as part of the MPO's Long-Range Transportation Plan, *Charting Progress to 2040*—used to guide investment decisions regarding transportation infrastructure improvements in the Boston region. The MPO prioritized this location for study after considering a number of factors: the need to address poor safety conditions and traffic congestion; the desire to enhance multimodal transportation; the need to maintain regional travel capacity; the interest in ensuring that, over time, corridor studies are funded in all subregions of the MPO's planning area; and the potential for recommendations from the study to be implemented.

Vinnin Square is a large commercial district and residential area of multi-family homes located primarily in Swampscott and including a section of Salem. Traffic from Vinnin Square directly impacts Marblehead. Commercial and residential development has significantly increased in the past decade, and will possibly continue to expand into Marblehead. As a result, there are a growing number of pedestrians and bicyclists in Vinnin Square; however, the current roadway configurations there inhibit walking and bicycling, and traffic safety, congestion, and mobility have become challenging issues.

The MPO staff, working with the study's advisory task force, developed improvements that would transform Route 1A and the ancillary streets around it into pedestrian- and bicyclist-friendly roadways, as well as a transportation corridor that serves all modes of transportation and maintains regional travel capacity. This study provides the City of Salem, the Towns of Marblehead and Swampscott, the Massachusetts Department of Transportation (MassDOT), and other stakeholders an opportunity to review, at a conceptual level, what would be required to address the deficiencies of Route 1A and the ancillary streets, before committing design and engineering funds to a roadway improvement project.

This report summarizes the analyses and recommendations from the study. The first sections describe the existing conditions in the study area and the problems that were identified. The following sections provide an assessment of the safety and operational problems, and discuss the potential improvements. The report also includes technical appendices, which cite the methods used and data applied in the study, including detailed reports about intersection capacity

analyses. If implemented, the report's recommendations would result in an improved roadway corridor where it is safe to walk or bicycle to the shops and businesses in Vinnin Square, recreational areas, and workplaces, and where traffic operates efficiently.

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Chapter 1—Introduction

1.1 ORIGIN OF STUDY

During the past five years, the Boston Region Metropolitan Planning Organization (MPO) has conducted five studies of priority corridors identified through the Needs Assessment of the Long-Range Transportation Plan (LRTP), and municipalities have been receptive to them. The studies provide cities and towns with the opportunity to review what is needed to improve a specific arterial segment, starting at the conceptual level, before committing design and engineering funds to a project. After reviewing the options, if a city or town initiates a project that qualifies for state and federal funds, the study's documentation may be useful to both the Massachusetts Department of Transportation (MassDOT) and the project proponent. The information provided in the study's report is useful for completing MassDOT Highway Division's project initiation forms, identifying problems along the corridor, justifying the need for improvements and for funding resources, and providing improvement concepts to advance into preliminary design and engineering.

The existing needs for all transportation modes in the MPO region were identified as part of the Needs Assessment of the LRTP, which guides the process of deciding which projects to fund in future Transportation Improvement Programs (TIPs). Current mobility needs in the MPO region, listed in the current LRTP, are as follows:

- Maintaining and modernizing roadways with high levels of congestion and safety problems
- Increasing the quantity and quality of walking and bicycling facilities
- Improving the efficiency of transit service and adherence to schedules

Based on previous and ongoing transportation-planning work—including the MPO's Congestion Management Process (CMP) and MPO planning studies—MPO staff identified several priority arterial roadway segments that require maintenance, modernization, and safety and mobility improvements, and listed them in the LRTP. To address the problems that exist in some of these arterial segments, a study was included in the federal fiscal year (FFY) 2016 Unified Planning Work Program (UPWP).² Through this study, MPO staff recommend

¹ Boston Region Metropolitan Planning Organization, *Charting Progress to 2040: The New Long-Range Transportation Plan of the Boston Region Metropolitan Planning Organization*, endorsed by the Boston Region MPO on July 30, 2015.

² Boston Region Metropolitan Planning Organization, Unified Planning Work Program, Federal Fiscal Year 2016, endorsed by the Boston Region Metropolitan Planning Organization on July 30, 2015.

conceptual improvements for one or more corridors, or several small sections within a corridor, which were identified in the CMP and LRTP Needs Assessment process. MPO staff select locations for study—considering municipal, subregional, and other public feedback—and collect data, conduct technical analysis, and recommend improvements. Recommendations from the study are sent to implementing agencies, which may choose to fund improvements through various federal, state, and local sources, separately or in combination.

By focusing on arterial segments rather than intersections, planners can evaluate multimodal transportation needs comprehensively with the goal of creating "complete streets." A holistic approach to analyzing problems and forming recommendations ensures that the needs of all roadways users—including pedestrians, bicyclists, motorists, and transit riders—are considered. Ultimately, this will result in roadways where it is safe to cross the street and walk or cycle to shops, schools, train stations, and recreational facilities, and where buses can run on time. Typically, the recommended improvements are within a roadway's right-of-way and they take into account the needs of abutters and roadway users, and the interests and support of stakeholders.

Chapter 2—Background and Objectives

2.1 SELECTION PROCESS

Following a selection process based on safety conditions,³ congested conditions,⁴ multimodal significance,⁵ regional significance,⁶ regional equity,⁷ and implementation potential,⁸ the *Route 1A-Vinnin Square Priority Corridor Study* in the City of Salem and Towns of Marblehead and Swampscott was selected and approved for study on April 2, 2015, by the MPO. The study location was selected from a list of 54 arterial segments in 39 municipalities in the MPO region.⁹ A copy of the technical memorandum about the selection process is included in Appendix A. MassDOT Highway Division District 4, MassDOT Office of Transportation Planning, the City of Salem, and the Towns of Marblehead and Swampscott supported the study of the Vinnin Square area. They participated by collecting data needed for the analyses, reviewing documentation on existing conditions, identifying problems, and developing improvements to mitigate the problems.

³ Safety Conditions: The location has a higher-than-average crash rate for its functional class; contains a crash cluster that makes it eligible for Highway Safety Improvement Program (HSIP) funding; contains a crash location on MassDOT Highway Division's Top High Crash Locations Report; or has a significant number of pedestrian and bicycle crashes (two or more per mile).

⁴ Congested Conditions: The travel time index is at least 1.3.

⁵ Multimodal Significance: The roadway carries one or more bus routes or is adjacent to a transit stop or station; the roadway supports bicycle or pedestrian activities or there is a project planned that will support these activities; there is a need to accommodate pedestrians and bicyclists and improve transit on the roadway; or there is a significant amount of truck traffic on the roadway serving regional commerce.

⁶ Regional Significance: The roadway is on the National Highway System; carries a significant portion of regional traffic (Average Daily Traffic of 20,000 vehicles or more); lies within 0.5 miles of environmental-justice transportation analysis areas or zones; or is essential for the region's economic, cultural, or recreational development.

⁷ Reginal Equity: To ensure that, over time, all subregions in the MPO's planning area receive UPWP planning studies, during each funding cycle MPO staff select no more than one location per subregion for potential study, and choose a location in a different subregion from the location studied in the preceding cycle.

⁸ Implementation Potential: The study location is proposed by the jurisdictional agency or agencies for the roadway; proposed or prioritized by a subregional group; or identified as a priority for improvement by other stakeholders.

⁹ Technical Memorandum, dated April 2, 2015, to the Boston Region Metropolitan Planning Organization, Federal Fiscal Year (FFY) 2016 Priority Corridors for Long-Range Transportation Plan (LRTP) Needs Assessment: Selection of Study Locations.

2.2 STUDY LOCATION

Figure 1 shows a regional map with the arterial roadway segments in the study area indicated in red (all figures are included at the end of the report). Vinnin Square is a large commercial district—made up of three large shopping areas with large chain stores, restaurants, and medical and office complexes—and residential area with numerous multi-family developments located primarily in Swampscott and a section of Salem. Traffic from Vinnin Square directly impacts Marblehead.

Vinnin Square is largely served by Route 1A (Paradise Road in Swampscott and Loring Avenue in Salem) and ancillary streets, including Essex Street, Tedesco Street, and Vinnin Street. All of the streets have two-lanes and widen at the major signalized intersections to include turn lanes. Over the past two decades, Vinnin Square has seen a high concentration of commercial and multi-family development. Swampscott, most noticeably, has seen a marked increase in development activity, which the town has encouraged in order to improve the viability of this commercial district. As a result, there is a growing number of pedestrians and bicyclists in Vinnin Square, and traffic safety, congestion, and mobility have become challenging issues. In addition, substantially more people from the neighboring residential areas on Route 1A, Loring Avenue, Essex Street, and Tedesco Street are walking, bicycling, and riding buses, and conditions for these users are expected to worsen in the future because of increasing congestion and lack of pedestrian and bicycle amenities.

2.3 STUDY VISION AND GOALS

The City of Salem and the Towns of Marblehead and Swampscott are envisioning a safe transportation network that would transform Vinnin Square into a vibrant area linking all three communities and that would balance the needs of motorists with the needs of pedestrians and bicyclists by increasing the quantity and quality of infrastructure for walking, biking, and bus transit.¹⁰

The goal is to improve the existing transportation system and make it more efficient by reducing congestion, increasing safety for motorists, pedestrians, and bicyclists; improving connectivity by closing gaps in the sidewalk network; providing continuous and usable shoulders or bike lanes; and connecting people to places to support economic activities and livable communities. Towards that end, the objectives of this study were as follows:

The Healthy Transportation Compact is a key requirement of the landmark transportation reform legislation, signed into law in June 2009, that aims to facilitate transportation decisions that balance the needs of all transportation users, expand mobility, improve public health, support a cleaner environment, and create stronger communities.

- Document existing problems.
- Examine traffic flow and capacity.
- Analyze safety for pedestrians, bicyclists, motorists, and bus riders.
- Determine pedestrian, bicyclists, motorists, and bus riders' needs.
- Develop multimodal transportation improvements.

2.4 PUBLIC PARTICIPATION

An advisory task force—composed of representatives from the City of Salem, Towns of Swampscott and Marblehead, MassDOT, and interest groups—was established to participate in this study. MPO staff met with the task force twice. In the first meeting they discussed the work scope and existing problems, such as lack of accommodation for bicyclists, long crosswalks, lack of pedestrian refuge areas, and high speeds of vehicles. In the second meeting, MPO staff presented the existing conditions, analyses, proposed improvements, and obtained comments. This report reflects the task force's feedback. Appendix A includes a list of task force members and their comments.

Chapter 3—Characteristics of the Corridor

3.1 ROADWAYS

The roadways which were the focus of this study are shown in Figure 2 and listed below:

- Route 1A (Paradise Road) in Swampscott
- Route 1A (Loring Avenue) in Salem
- Essex Street and Loring Avenue in Salem and Swampscott
- Vinnin Street in Swampscott
- Tedesco Street in Marblehead
- Salem Street in Swampscott

Figure 3 shows the roadway jurisdiction, which identifies the authority and obligation of agencies to administer, control, construct, maintain, and operate a highway subject to the provisions of the Commonwealth of Massachusetts. When an agency has jurisdiction of a street or highway, that agency is responsible for the upkeep of that highway, including reconstruction, signing, and maintenance. All of these responsibilities remain with the agency until the jurisdiction is transferred to another authority.

Figure 4 shows the functional class of the roadways in and around the study area. The functional class identifies a roadway according to the character of traffic service that it is intended to provide and the degree of access to the roadway (access control). There are three roadway functional classifications: arterial, collector, and local roads. Arterial roadways provide the highest level of service for the longest uninterrupted distance, with some degree of access control. Collector streets provide a less highly developed level of service at a lower speed for shorter distances by collecting traffic from local roads and connecting them with arterials. Local roads primarily provide direct access to abutting land parcels, such as residential areas.

3.1.1 Route 1A

The 2.5 mile section of Route 1A (Paradise Road and Loring Avenue) in the study area is a two-lane undivided roadway with turn lanes at designated locations (mostly at signalized intersections). The roadway is directionally designated as a north-south principal arterial roadway under the jurisdiction of MassDOT. It connects Salem and several communities to the north—Danvers, Peabody, and Wenham—as well as Swampscott and several cities to the south—Lynn, Revere, Boston, Chelsea, and Everett. In addition, Route 1A provides access to Marblehead to the east via Tedesco Street. Route 1A is part of the National Highway System (NHS) program; as such, projects to improve

Route 1A are eligible for federal funds. Route 1A serves regional and local traffic, the right-of-way varies between 50 and 60 feet wide in the study area, and the posted speed limit varies from 30 to 35 miles per hour (mph) in both directions. ¹¹The land uses adjacent to the roadway are mixed—commercial, residential, and recreational.

3.1.2 Essex Street and Loring Avenue

The segment of Essex Street and Loring Avenue in the study area is approximately 0.75 miles long. The two roadways create a continuous artery, and thus are both directionally designated as north-south arterial roadways under the jurisdictions of the City of Salem (for Loring Avenue) and Town of Swampscott (for Essex Street). Both are two-lane undivided roadways with turn lanes at the signalized intersections. They connect to Vinnin Square and serve regional and local traffic. Both roadways are classified as minor arterials and they are not part of the NHS program. The right-of-way varies between 46 and 55 feet wide, and the posted speed limit is 30 mph in both directions. The land uses adjacent to the roadway are mixed—commercial, residential, and cemetery.

3.1.3 Tedesco Street and Vinnin Street

The segment of Tedesco Street and Vinnin Street in the study area is approximately 0.8 miles long. The two roadways create a continuous artery, and thus are both directionally designated as east-west arterial roadways under the jurisdictions of the Town of Marblehead (for Tedesco Street) and Town of Salem (for Vinnin Street). Both are two-lane undivided roadways with turn lanes at the signalized intersections. Both roadways, east of Route 1A, are classified as principal arterials on the NHS program, thus projects to improve them are eligible for federal funds. The right-of-way varies between 40 and 48 feet wide, and the posted speed limit is 35 mph in both directions. The land uses adjacent to both streets are mixed— residential, recreational, and commercial.

3.1.4 Salem Street

The approximately 0.3 mile section of Salem Street in the study area is a two-lane undivided roadway. The roadway is directionally designated as a north-south minor arterial roadway under the jurisdiction of the Town of Swampscott. Salem Street provides access to Vinnin Square and it connects via Vinnin Street and Sunbeam Lane. The roadway is classified as a minor arterial and it is not on the NHS program. The right-of-way is about 50 feet wide and the posted speed limit is 30 mph in both directions. The land uses adjacent to the roadway are mixed—commercial, recreational, and residential.

¹¹ Right-of-way is defined as the land or interest therein, acquired for or devoted to a highway.

3.2 MAJOR INTERSECTIONS

Several minor arterials, collector streets, business driveways intersect Route 1A, Essex Street, and Tedesco Street to form signalized intersections. These intersections, shown in Figure 5, are described below. The intersections are listed from south to north.

3.2.1 Route 1A and Ellis Road Intersection

Ellis Road is a city-owned local street in Swampscott that intersects Route 1A to form a four-leg signalized intersection. MassDOT has jurisdiction over this intersection and is responsible for implementing improvements to the intersection. Each of the approaches to the intersection has one travel lane. The intersection is equipped with an Eagle DP 300 signal controller and has a semi-actuated traffic-control system with functioning push-button pedestrian signals. The signal equipment lacks an Opticom system for emergency preemption. The traffic signal heads are mounted on a mixture of mast-arm and post mounts. There are crosswalks on all legs of the intersection; however, the curb ramps do not meet MassDOT or American with Disabilities Act (ADA) standards as they lack detectable warning plates. A bus stop is located on each side of Route 1A south of the intersection. The land uses adjacent to the intersection are primarily residential, although, the Clarke Elementary School is located in the northwest corner of the intersection.

3.2.2 Route 1A and Vinnin Liquor Driveway Intersection

The driveway to Vinnin Liquors and Whole Foods is privately owned. It intersects Route 1A to form a three-leg signalized intersection. MassDOT, which has jurisdiction over this intersection, is responsible for implementing improvements to it. Each of the Route 1A approaches has one through travel lane in each direction and an exclusive left-turn lane on the southbound approach. The Vinnin Liquor and Whole Foods driveway has two travel lanes on its approach: one for left turns and one for right turns. The intersection is equipped with a Peek 3000E signal controller and has a fully actuated and coordinated traffic control system with functioning push-button pedestrian signals and an Opticom system for emergency preemption. The signal heads are mounted on a mixture of mast-arm and post mounts. There are crosswalks with curb ramps, but they do not meet MassDOT or ADA standards because they lack detectable warning plates. A bus stop is located on each side of Route 1A, but there are no bus stop signs, shelters, or benches. The intersection curb radii are adequate for trucks and buses servicing commercial and retail businesses in the area. The land uses adjacent to the intersection are mixed—commercial and residential.

3.2.3 Route 1A and Swampscott Mall and Vinnin Square Plaza Driveways Intersection

The driveways to Swampscott Mall and Vinnin Square Plaza are privately owned. They intersect Route 1A to form a four-leg signalized intersection. MassDOT has jurisdiction over this intersection. It is one of the critical intersections in the corridor as there are high traffic volumes on both Route 1A and the driveways. Near the intersection, Route 1A has two lanes on the northbound approach (an exclusive left-turn lane and through/right-turn lanes) and three lanes on the southbound approach (an exclusive left-turn lane, through lane, and exclusive right-turn lane). The Swampscott Mall driveway has three travel lanes on the approach (an exclusive left-turn lane, through lane, and exclusive right-turn lane), while the Vinnin Square Plaza driveway has two travel lanes (an exclusive leftturn lane and through/right-turn lanes). The intersection is equipped with a Peek 3000E signal controller and has a fully actuated and coordinated traffic signal system with an Opticom system for emergency preemption. The signal heads are mounted on a mixture of mast-arm and post mounts. There are functioning pushbutton pedestrian signals only for crossing Route 1A. There are no crosswalks or push-button pedestrian signals for crossing either of the mall driveways. The curb ramps do not meet MassDOT's standards because they lack detectable warning plates. A bus stop is located on each side of Route 1A, but neither has a shelter. The intersection curb radii are adequate for trucks and buses servicing commercial and retail businesses in the area. The land use in the vicinity is commercial.

3.2.4 Essex Street and Swampscott Mall Driveway Intersection

Essex Street and the Swampscott Mall driveway intersect to form a three-leg signalized intersection. The Town of Swampscott has jurisdiction over this intersection. Each of the Essex Street approaches has two travel lanes: on the southbound approach (an exclusive left-turn lane and a through lane) and on the northbound approach (an exclusive right-turn lane and a through lane). The mall driveway has two travel lanes at the approach: one for left turns and one for right turns. The intersection is equipped with a Peek 3000E signal controller and has a fully actuated traffic signal system with functioning pedestrian signals. The signal heads are mounted on a mixture of mast-arm and post mounts. There are crosswalks with curb ramps, but the ramps do not meet MassDOT or ADA standards because they lack detectable warning plates. A bus stop is located on each side of Essex Street, but neither has a shelter. The intersection curb radii are adequate for trucks and buses. The land uses near the intersection are mixed—commercial and industrial.

3.2.5 Route 1A and Vinnin Street Intersection

Vinnin Street is a city-owned minor collector in Salem that intersects Route 1A to form a four-leg signalized intersection. MassDOT has jurisdiction over this intersection. Each of the Route 1A approaches has two travel lanes (a shared through/left-turn lane and a shared through/right-turn lane). Vinnin Street has two travel lanes on the eastbound approach (an exclusive left-turn lane and a through/right-turn lane) and three lanes on the westbound approach (an exclusive left-turn lane, through lane, and exclusive right-turn lane). The intersection is equipped with a Peek 3000E signal controller and has a fully actuated and coordinated traffic control system with functioning push-button pedestrian signals. An Opticom system for emergency preemption has been installed. The signal heads are mounted on mast-arm mounts. There are crosswalks with curb ramps, but they do not meet MassDOT or ADA standards because they lack detectable warning plates. A bus stop is located on each side of Route 1A, but neither has a shelter with a bench. The intersection curb radii are adequate for trucks and buses servicing commercial business activities. The land uses in the area are commercial—mostly retail services.

3.2.6 Vinnin Street and Loring Avenue Intersection

Vinnin Street intersects Loring Avenue, a city-owned street, to form a four leg signalized intersection. The City of Salem has jurisdiction over the roadways and the intersection. Loring Avenue's southbound approach has two travel lanes (an exclusive left-turn lane and through/right-turn lane) and three travel lanes on the northbound approach (an exclusive left-turn lane, through lane, and exclusive right-turn lane). Vinnin Street has two lanes on the westbound approach (an exclusive left-turn lane and through/right-turn lane) and one lane on the eastbound approach for all traffic movements. The intersection is equipped with a Peek 3000E signal controller and has a fully actuated and coordinated traffic signal system with functioning push-button pedestrian signals. The signal equipment has an Opticom system for emergency preemption. The signal heads for the traffic movements are mounted on mast-arm mounts. There are crosswalks with curb ramps, but the curb ramps do not meet MassDOT or ADA standards because they lack detectable warning plates. A bus stop is located on each side of Loring Avenue, but neither has a bus shelter. The intersection curb radii are adequate for trucks and buses. The land uses in the area are mixed commercial and residential.

3.2.7 Route 1A (Paradise Road) and Route 1A (Loring Avenue) Intersection

Route 1A's local name changes at the Salem-Swampscott line from Loring Avenue on the Salem side to Paradise Road on the Swampscott side. Loring

Avenue intersects Route 1A to form a three-leg signalized intersection. MassDOT has jurisdiction over this intersection. Each of the approaches at the intersection has two travel lanes: the northbound approach has an exclusive left-turn lane and through lane; the southbound approach has a through lane and exclusive right-turn lane; and the eastbound approach has an exclusive left-turn lane and exclusive right-turn lane. The intersection is equipped with a Peek 3000E signal controller and has a fully actuated and coordinated traffic signal system with functioning push-button pedestrian signals. The signal heads are mounted on a mixture of mast-arm and post mounts and the equipment includes an Opticom system for emergency preemption. There are crosswalks with curb ramps at all corners of the intersection, but the curb ramps do not meet MassDOT or ADA standards because they lack detectable warning plates. A bus stop is located on each side of Route 1A north of the intersection, but neither has a bus shelter. The intersection curb radii are adequate for trucks and buses. The land uses near the intersection are commercial and residential.

3.2.8 Vinnin Street and Salem Street Intersection

Salem Street is a city-owned minor arterial in Salem that intersects Vinnin Street to form a three-leg signalized intersection. The City of Salem has jurisdiction over this intersection. Each approach of Vinnin Street has two lanes: the eastbound approach has an exclusive right-turn lane and through lane, and the westbound approach has a shared through/left-turn lane and through lane. Salem Street's northbound approach has two lanes, an exclusive left-turn lane and exclusive right-turn lane. The intersection is equipped with a Peek 3000E signal controller and has a fully actuated traffic signal system with functioning pedestrian signals and an Opticom system for emergency preemption. The signal heads are a mixture of mast-arm and post mounts. There are crosswalks with curb ramps, but the curb ramps do not meet MassDOT or ADA standards because they lack detectable warning plates. A bus stop is located on each side of Salem Street. The intersection curb radii are adequate for trucks and buses. The land uses near the intersection are commercial and recreational.

3.2.9 Tedesco Street and Leggs Hill Road Intersection

Leggs Hill Road is a town-owned collector street in Marblehead that intersects Tedesco Street to form a three-leg unsignalized intersection. The Town of Marblehead has jurisdiction over this intersection. All of the Tedesco Street approaches and Leggs Hill Road's southbound approach have one lane for all traffic movements. Tedesco Street is the major street at the intersection and its traffic is not controlled. Leggs Hill Road is the minor street and its traffic is controlled by a stop sign. The Town of Marblehead recently installed a crosswalk on Tedesco Street that is equipped with a rectangular rapid flash beacon, but Leggs Hill Road still lacks a crosswalk. There are curb ramps at the intersection,

but they do not meet MassDOT's standards since they do not have detectable warning plates. The land use near the intersection is residential.

3.2.10 Route 1A and Leggs Hill Road Intersection

Leggs Hill Road is a city-owned collector road in Salem that intersects Route 1A to form a three-leg unsignalized intersection. MassDOT has jurisdiction over this intersection. The intersection is located in a sharp horizontal curve that limits sight lines and distances for drivers on Route 1A. At the intersection, each approach of Route 1A has two lanes: the northbound approach has a through lane and right-turn lane, and the southbound approach has a through lane and left-turn lane. Leggs Hill Road westbound has one lane on its approach serving all traffic movements. Route 1A is the major street at the intersection and its traffic is not controlled. Leggs Hill Road is the minor street and its traffic is controlled by a stop sign. There is a sidewalk on the east side of Route 1A, but none on the west side. The sidewalk has curb ramps at Leggs Hill Road, but the curb ramps do not meet MassDOT or ADA standards because they lack detectable warning plates. The crosswalk on Route 1A is located south of the intersection because of a horizontal curve that limits sight distance on Route 1A. There is no crosswalk on Leggs Hill Road connecting the sidewalks. A bus stop is located on each side of Route 1A. The intersection curb radii are adequate for trucks and buses. The land uses near the intersection are recreational and educational.

3.2.11 Route 1A and Harrison Road Intersection

Harrison Road is a city-owned road in Salem that intersects Route 1A to form a three-leg signalized intersection. MassDOT has jurisdiction over this intersection. The Route 1A approaches at the intersection have two lanes: the northbound approach has an exclusive left-turn lane and through/right-turn lane, and the southbound approach has an exclusive right-turn lane and through lane. Harrison Road's eastbound approach has one lane serving all traffic movements. The intersection is equipped with a TCT LMD 9200 signal controller and has a fully actuated and uncoordinated traffic signal system with functioning pedestrian signals, and an Opticom system for emergency preemption. The signal heads are mounted on mast-arm mounts. There are sidewalks on both sides of Route 1A and Harrison Road and crosswalks with curb ramps for crossing both streets, but the curb ramps do not meet MassDOT or ADA standards because they lack detect warning plates. A bus stop is located on each side of Route 1A south of the intersection; despite the large number of bus boardings and alightings at these stops, there are no shelters. The intersection curb radii are adequate for trucks and buses. The land uses near the intersection are residential and educational; Harrison Road provides access to the Salem State University South

Campus, which in the future will become the new site for the Horace Mann Laboratory School.

3.2.12 Route 1A and Pickman Road Intersection

Pickman Road is a city-owned street that intersects Route 1A to form a three-leg signalized intersection. MassDOT has jurisdiction over this intersection. The Route 1A approaches have one lane serving all traffic movements at each approach. Pickman Road's eastbound approach has one lane serving all traffic movements. The intersection is equipped with an EPAC 300 signal controller and has a semi-actuated, uncoordinated traffic signal system with functioning pedestrian signals. The signal heads are mounted on both mast-arm and post mounts. The signal system has no Opticom system for emergency preemption. There are sidewalks on both sides of Route 1A and Pickman Road and crosswalks with curb ramps for crossing both streets, but the curb ramps do not meet MassDOT or ADA standards because they lack detectable warning plates. A bus stop is located on each side of Route 1A at the intersection. The intersection curb radii are adequate for trucks and buses. The land use near the intersection is primarily residential.

3.3 LAND USE AND DEVELOPMENT

The map in Figure 6 shows the general land-use designation for the area surrounding the roadways that are the focus of this study. The land uses include, but are not limited to, residential, commercial, educational, park and recreation, and cemetery. Based on discussions with representatives from Marblehead, Salem, and Swampscott, the following are the recent and planned developments in the area that impact the area's traffic flow.

3.3.1 Potential Redevelopment of Existing and Vacant Parcels

The City of Salem and the Towns of Marblehead and Swampscott expect redevelopment of some of the existing land parcels in the vicinity of Vinnin Square. Over the past decade, Vinnin Square has seen a high concentration of commercial and multi-family home development. Most notably, Swampscott has seen a marked increase in development activity, which the town has encouraged in order to improve the viability of this commercial district. In addition, the vacant parcel of land adjoining the Tedesco Country Club, which is part of an overlay district in both Marblehead and Salem, is zoned to allow for high-density residential development of 30 units per acre. Redevelopment of existing and vacant parcels is expected to continue at Vinnin Square into the future.

3.3.2 Salem School Swap

Salem State University and the City of Salem have plans to move the

Horace Mann Laboratory School, which is currently located off of Route 1A (Loring Avenue) at the North Campus, to the South Campus off Harrison Road. According to the Salem News, Sasaki Associates, a planning firm based in Watertown, has been hired by the University to help put together plans for the University's North Campus. 12 Moving the Horace Mann Laboratory School to the South Campus would impact traffic on Route 1A, as access to Harrison Road is only available via Route 1A. The move would require that a comprehensive Safe Routes to School (SRTS) study be undertaken to identify appropriate routes to the new school, the needs of pedestrians and bicyclists, and safety improvements that would encourage students and parents to walk and bike to school. As Route 1A is the primary roadway connecting to Harrison Avenue, this section of Route 1A would need improvements to accommodate school buses and the additional traffic to the site during school openings and closings. Safety for pedestrians and bicyclists when crossing Route 1A is a priority and should be evaluated in light of the school swap. This study looked at improvements on Route 1A that would benefit or facilitate the school swap, but a comprehensive SRTS study is needed to address impacts on other streets.

3.4 PLANNED PROJECTS AND STUDIES

Figure 7 shows the planned projects and previous studies that addressed the study area or its surroundings. The improvements developed in this study considered and incorporated recommendations from the previous studies.

3.4.1 Canal Street Rail Trail Construction (Phase 2)

The Canal Street Rail Trail project will construct 0.6 miles of shared-use, off-road path to close the gap between the Marblehead Rail Trail and the shared-use path that extends from Canal Street to Mill Street in Salem. The new trail will be 10 feet wide with two-foot shoulders. The project includes the relocation of a railroad spur. Funding for the project is programmed in the Boston Region MPO's FFY 2019 TIP. As of March 16, 2016, the project was at the 25 percent design stage. ¹³

3.4.2 Reconstruction on Canal Street, from Washington Street and Mill Street to Loring Avenue and Jefferson Avenue

The improvements to Canal Street include reconstruction of the roadway pavement, curbing, and sidewalks. Wheelchair ramps and pedestrian crossings will be added where appropriate to improve pedestrian safety. Additional

http://www.salemnews.com/...s/salem-state-unveils-early-vision-for-school-campus-swap/article 9e2cd695-1683-5215-8d77-8d3cde7d9cb8.html[11/6/2016 12:36:54 PM]

¹³ MassDOT Highway Division, Project Information Database, based on data queried on November 7, 2016. http://www.massdot.state.ma.us/highway/ProjectInfo.aspx

improvements, such as tree plantings and ornamental lighting, and curb extensions will be incorporated. Pavement markings will be painted to define the parking areas and provide defined shoulder areas for use by bicyclists. Drainage improvements will be constructed, the roadway crown will be adjusted to provide a consistent cross slope, and locations that have settled will be repaired. Driveway access issues will be more clearly defined so that ways to improve safety for vehicles entering and exiting local businesses can be identified. Traffic signals at Mill Street and Washington Street and at Loring Avenue and Jefferson Avenue will be updated. This project was under construction as of October 15, 2016. 14

3.4.3 Road Safety Audit, Route 1A (Loring Avenue), Salem

In collaboration with the City of Salem, the Salem State University Police Department, and MassDOT, a road safety audit (RSA) was conducted along the segment of Route 1A between Intervale Road and Harrison Road in Salem. The RSA was conducted in response to concerns from the city and the university regarding safety along this section of roadway. Thirty-three crashes occurred on the roadway segment in the span of approximately three and a half years, including a fatal crash in June 2013 in which a pedestrian died. The crash rate for the segment is about 2.0 crashes per million vehicles entering, which is below the state average for principal arterials. The purpose of this RSA was to identify both short- and long-term safety improvements that could be made to increase safety along the corridor. Potential short-term, low-cost improvements could be considered by the responsible agency for immediate implementation, as appropriate. The *Route 1A-Vinnin Square Priority Corridor Study* considered the recommendations of the RSA in developing proposed improvements for that section of Route 1A.

3.4.4 Road Safety Audit, 450 Paradise Road (Route 1A) at Swampscott Mall in Swampscott

MassDOT identified the segment of Route 1A (Paradise Road) at the Swampscott Mall as a high crash cluster location for vehicles, based on 2010-12 crash data. Because it is a high-crash location, MassDOT required a RSA to be conducted. Recommendations from the RSA are expected to be incorporated into MassDOT project number 607761, which primarily consists of improvements to Route 1A at the intersection of Swampscott Mall and Vinnin Square Plaza. The

¹⁴ Ibid

¹⁵ Road Safety Audit, Loring Avenue, Salem, prepared for Massachusetts Department of Transportation, November 2013.

Road Safety Audit, 450 Paradise Road (Route 1A) at Swampscott Mall in Swampscott, Massachusetts, Prepared for Massachusetts Department of Transportation, August 27, 2015.

Route 1A-Vinnin Square Priority Corridor Study considered the recommendations of the RSA in developing proposed improvements for that section of Route 1A.

3.4.5 Traffic Impact Study, Assisted-Living Residential Development

The purpose of this traffic impact study is to evaluate existing and projected traffic, operational, and safety conditions in the vicinity of the proposed assisted-living residential development located at 224 Salem Street in Swampscott, and identify measures to mitigate potential project-related traffic impacts on the surrounding roadways, if necessary. McMahon Associates reviewed the existing traffic operations and potential traffic impacts from the new facility and, based on the analysis, determined that the projected traffic increases associated with both the background traffic growth (traffic volumes projected for the area) and the traffic generated by the new assisted-living facility could be accommodated on the roadways in the area. In addition, the study determined that with the proposed facility in place, safe and efficient access will be provided to the site.¹⁷

¹⁷ Assisted Living Residential Development, Traffic Impact Study, Swampscott, Massachusetts, January 2015.

Chapter 4—Existing Transportation

4.1 DATA COLLECTION

MassDOT Highway Division's Traffic Data Collection Section performed turning movement counts (TMCs) at the intersections in the study area in April 2016, while schools were in session. The counts were conducted during the weekday AM peak travel period (7:00 AM–9:00 AM), the weekday PM peak travel period (4:00 PM–6:00 PM), and the Saturday midday travel period (12:00 AM–2:00 PM). Heavy vehicles such as school buses, transit buses, and trucks were counted separately. Pedestrian and bicycle counts were conducted simultaneously with the TMCs.

In addition, the Traffic Data Collection Section conducted automatic traffic recorder (ATR) counts at five locations on Route 1A, Essex Street, Tedesco Street, and Salem Street. The ATR counts are continuous 48-hour traffic counts used to determine the average weekday traffic (AWDT) on a roadway. The Traffic Data Collection Section also collected spot-speed data at the same five locations. Similar to the ATR counts, the spot-speed data are continuous 48-hour records. The TMC, AWDT, and spot-speed data are included in Appendix B.

4.2 DAILY TRAFFIC VOLUMES

Figure 8 shows the AWDT at the five locations. The AWDT value ranges are as follows:

- 19,000 to 20,000 vehicles per day on Route 1A
- 18,000 to 19,000 vehicles per day on Essex Street and Loring Avenue
- 15,000 to 16,000 vehicles per day on Tedesco Street
- 6,000 to 7,000 vehicles per day on Salem Street

Figure 9 shows the daily distribution of the hourly traffic volumes at the five locations. The daily distributions show peak-period volumes in the range of 800 to 1,000 vehicles per hour (vph) in both directions of Route 1A. Outside of the AM and PM peak periods, the traffic volumes in each direction of Route 1A are in the range of 400 to 600 vph. The estimated capacity of a two-lane roadway is about 800 to 1000 vph per direction, and the capacity of a four-lane roadway is about 1,600 to 1,800 vph per direction. Therefore a two-way, two-lane roadway should be adequate for traffic on Route 1A and the ancillary streets, except at the signalized intersections where turn lanes are justified.

4.3 TURNING MOVEMENT VOLUMES

Figure 10 shows the turning movement volumes at the major intersections during the weekday AM peak hour (7:30–8:30 AM), weekday PM peak (4:45 PM–5:45 PM), and Saturday PM peak (12:00–1:00 PM). Based on the turning movement volumes, MPO staff determined the following:

- The traffic in the corridor consists of pass-thorough commuter traffic and traffic going to shopping, school, and recreational destinations in the study area.
- The peak flow direction is southbound during the AM peak period and northbound during the PM peak period.
- The following are the critical intersections controlling traffic flow in the corridor:
 - Route 1A at the driveway to Vinnin Liquor and Whole Foods
 - Route 1A at the driveway to Swampscott Mall
 - Route 1A at Vinnin Street
 - Route 1A at Loring Avenue

4.4 PEDESTRIAN TRAFFIC VOLUMES

Table 1 presents the number of pedestrians observed at the major intersections in the study area during the two-hour weekday AM peak period and the two-hour weekday PM peak period on Tuesday, April 12, 2016, and the two-hour Saturday PM peak period on April 9, 2016. These volumes may be low because of the colder than usual weather in April and the high traffic volume during peak periods. Nonetheless, significant pedestrian activity was observed in the corridor as people walked to access residential and recreation areas, schools, and commercial services.

TABLE 1
Pedestrian and Bicycle Volumes

Intersection	Pedestrians	Bicyclists
Route 1A at Ellis Road	98	7
Route 1A at Vinnin Liquor Driveway	42	4
Route 1A at Swampscott Mall	58	3
Route 1A at Vinnin Street	42	2
Route 1A at Loring Avenue	18	7
Route 1A at Leggs Hill Road	11	7
Route 1A at Harrison Road	32	11
Route 1A at Pickman Road	24	0
Essex Street and Swampscott Mall Driveway	30	8
Loring Avenue at Vinnin Street	21	4
Vinnin Street at Salem Street	8	5
Tedesco Street at Leggs Hill Road	9	4

Source: Central Transportation Planning Staff.

4.5 BICYCLE TRAFFIC VOLUMES

Table 1 presents the number of bicyclists observed at the major intersections in the study area during the two-hour weekday AM peak period and the two-hour weekday PM peak period on Tuesday, April 12, 2016, and the two-hour Saturday PM peak period on April 9, 2016. These volumes may be low because of the colder than usual weather in April, high traffic volume during peak periods, and the lack of amenities that provide safety and comfort for bicyclists, such as functioning shoulders or bike lanes. Despite these adverse conditions, the counts indicate that bicyclists are using the area roadways.

4.6 HEAVY VEHICLES VOLUMES

The percentage of heavy vehicles (light-goods vehicles, buses, single-unit trucks, and semi-trucks) observed at the intersections ranges between 2.0 and 5.0 percent of the total traffic on a weekday and between 1.0 and 3.0 percent on a Saturday. These rates are not considered particularly high for peak-period traffic conditions. The percentages of heavy vehicles are included in the TMC in Appendix B.

4.7 SPOT SPEEDS

Figure 11 shows the results of the spot-speed data collected on the roadways in the study area. Spot speeds are vehicle speeds observed at a specific location. The data gathered in spot-speed studies are useful for making decisions about safety applications, such as setting speed limits, evaluating speed problems, and assessing speed as a contributing factor in crashes. The average spot speeds observed in the corridor range between 25 and 32 mph, close to the 30 mph posted speed limit, except for the segment of Route 1A in the vicinity of Leggs Hill Road in Salem, where the average spot speed was 36 mph. In addition, 85 percent of the drivers were travelling at 37 mph or slower, except for the segment of Route 1A in the vicinity of Leggs Hill Road, where the drivers travelled at 42 mph. The spot-speed data indicated that about 62 percent of the drivers travel between 29 and 39 mph, which is known as the 10-mph-pace speed. The data and analysis shows that the observed speeds on the Route 1A segment in the vicinity of Leggs Hill Road are considerably higher than the posted speed limit.

4.8 SIGNAL TIMING AND LAYOUT INFORMATION

MassDOT provided the MPO staff with the existing signal timings, as-built traffic signal plans, and signal-phase sequences of the signalized intersections (included in Appendix C). MPO staff used Google Maps and field visits to identify recent modifications to the intersection layouts and signal plans. The information was used to analyze existing traffic operations conditions.

4.9 BUS SERVICE

There are several public transportation services operating in the study area, including bus and commuter rail services. These are displayed in Figure 12, a transit service map. The MBTA operates four bus routes in the study area:

- Route 441: Marblehead Haymarket (Boston) or Wonderland Station (Revere) via Paradise Road
- Route 448: Marblehead Downtown Crossing (Boston)
- Route 455: Salem Depot Wonderland (Revere) via Central Square (Lynn)
- Route 459: Salem Depot Downtown Crossing via Logan Airport (Boston) and Central Square (Lynn)

These services connect the communities in the study area to Central Square Station in Lynn on the Newburyport Commuter Rail Line; Wonderland Station on the Blue Line in Revere; Logan Airport in East Boston; and downtown Boston, including South Station. Routes 441 and 448 operate Monday through Friday every 15 minutes from 5:13 AM to 12:28 AM, and Saturday and Sunday every 30 minutes from 6:06 AM to 12:39 AM. Routes 455 and 459 operate Monday through Friday every 20 minutes from 5:10 AM to 12:30 AM, and Saturday and Sunday every 30 minutes from 6:05 AM to 11:35 PM. The schedules of the four bus routes are included in Appendix D.

Figure 13 shows the bus stop locations and number of riders (boarding and alighting) at the heavily used stops. Only one stop in the entire study area had a bus shelter (Essex Street at Carol Way). The lack of bus shelters creates inconveniences for passengers, especially during inclement weather.

Table 2 shows the results of the evaluation of bus routes as measured against the MBTA's service standards for span of service (hours during which the service operates), frequency of service, vehicle loading (passenger crowding based on the number of passengers to seats), schedule adherence, daily ridership, and average number of passengers per trip. These standards establish acceptable levels of service required to meet the MBTA's service objectives for accessibility, reliability, safety and comfort, and cost effectiveness of service. The evaluation was based on the MBTA's 2010 service delivery policy standards and the spring 2011 schedule.

The performance evaluation showed that MBTA bus Routes 441, 455, and 459 all had high levels of ridership, but all failed the frequency of service and vehicle loading standards; additional resources would be required to bring those routes up to the standard. Bus Route 448 had low ridership and failed the span of service standard.

TABLE 2
Bus Service Evaluation, 2010–11

Route	Route Description	Span of Service	Frequency of Service	Vehicle Loading	Schedule Adherence	Daily Ridership	Average Number of Passengers per Trip
	Marblehead – Haymarket (Boston) or Wonderland						
	Station (Revere) via						
441	Paradise Road	Pass	Fail	Fail	49.0%	1,442	37
	Marblehead - Downtown						
448	Crossing (Boston)	Fail	Pass	Pass	49.0%	162	32
	Salem Depot - Wonderland (Revere) via						
455	Central Square (Lynn)	Pass	Fail	Fail	42.0%	1,797	42
	Salem Depot – Downtown Crossing via Logan Airport (Boston)						
459	and Central Square (Lynn)	Pass	Fail	Fail	37.0%	1,085	43

Notes: The performance standards listed in this chart are defined in the MBTA's 2010 Service Delivery Policy.

The evaluation was based on the MBTA's spring 2011 schedule.

"Pass" means the bus service meets the performance standards established for that service standard. "Fail" means the bus service does not meet the performance standards established for that service objective. "Span" is based on the 2010 service delivery policy standard for the route type and spring 2011 schedule; the goal is local routes weekday span from 7:00 AM to 6:30 PM; correcting this failure would always require additional resources.

"Frequency" is based on 2010 service delivery policy standard for the route type and spring 2011 schedule; the goal is local routes AM and PM peak, 30-minute headway; correcting this failure would always require additional resources.

"Loading" is based on the 2010 service delivery policy and same ridership data used above; the standard is less than 140 percent of seated load averaged over 30-minute period during peak periods and less than 100 percent of seated load averaged over 60-minute period during off-peak periods; correcting this failure would always require additional resources.

"Schedule Adherence" is based on the 2010 service delivery policy (the definition of this service objective varies by frequency of service and time point crossings for start/mid/endpoints of the bus route); percentage shown is the proportion of all time point crossings during fall 2010, which were on time; the goal is 75 percent on time. Correcting this failure would NOT always require additional resources. Source: Massachusetts Bay Transportation Authority.

4.10 COMMUTER RAIL SERVICE

The MBTA's Newburyport/Rockport Commuter Rail Line has stations in Swampscott and Salem. Many of the commuter rail riders from the communities in the study area access the commuter rail stations via Route 1A or Essex Street and Loring Avenue. The MBTA operates a full schedule Monday through Friday from 4:55 AM to 12:00 AM and an abbreviated service on Saturday and Sunday from 7:00 AM to 11:11 PM. (The full train schedules are included in Appendix D.) Peak-period frequency for both the inbound and outbound trains is approximately 40 minutes. The typical weekday boarding (inbound trains) at the Salem Station and Swampscott Station are 2,122 and 884 passengers, respectively.

At Salem Station, the MBTA provides 329 parking spaces; 67 percent of the spaces are utilized on an average day. The City of Salem offers another 120 parking spaces near the station; 88 percent are utilized on an average day. At Swampscott Station the MBTA provides 125 spaces; 90 percent are utilized on an average day. In addition, the Town of Swampscott provides 18 spaces; 100 percent are utilized on an average day. The parking rate is \$4.00 daily at both Salem Station and Swampscott Station.

Chapter 5—Existing Conditions Analyses

5.1 SAFETY ANALYSIS

MPO staff used crash data from MassDOT's Registry of Motor Vehicles database and from municipal police departments for the time period from January 2011 through December 2014 to evaluate safety for motorists, pedestrians, and bicyclists in the study area. The following sections describe the analyses and results of this safety assessment.

5.1.1 Crash Summary

Figure 14 shows the motor-vehicle crash clusters in the study area, and Figure 15 shows the pedestrian- and bicycle-crash clusters. Table 3 presents the crash summaries and crash rates for each cluster. The summary indicates the severity of the crashes; manner of collision; road-surface, ambient-light, and weather conditions at the time of the crashes; number of bicyclists and pedestrians involved; and time of occurrence. The crash data for each individual crash in each cluster is included in Appendix E.

TABLE 3
2011–14 Crash Summary and Crash Rates

Crash Variable	Cluster 1	Cluster 2	Cluster 3	Cluster 4	Cluster 5	Cluster 6	Cluster 7
Crash Severity							
Fatal injury	0	0	0	0	1	1	0
Non-fatal injury	26	7	24	3	7	17	33
Property damage only	35	14	47	12	17	48	11
Unknown/not reported	12	0	3	2	0	2	7
Manner of Collision							
Rear-end	32	11	15	5	13	42	13
Angle	24	0	38	5	5	13	25
Single vehicle crash	7	4	5	6	5	7	4
Sideswipe, same direction	5	2	6	0	1	3	3
Head-on	3	2	8	1	0	0	1
Sideswipe, opposite direction	2	0	2	0	1	2	1
Not reported/unknown	0	0	0	0	0	2	4
Road Surface Conditions							
Dry	59	13	50	15	21	46	36
Wet	13	5	21	2	5	22	11
Sand, dirt, gravel, and water	0	0	0	0	0	0	0
Snow/ice	0	0	3	0	0	0	1
Not reported/unknown	1	3	0	0	0	0	3
Ambient Light Conditions							
Daylight	62	15	53	9	22	49	34
Dark, lighted roadway	10	3	15	8	1	16	9
Dark, unlighted roadway	0	1	1	0	1	0	1
Dawn	0	0	2	0	1	0	1
Dusk	0	0	3	0	0	3	4

Crash Variable	Cluster 1	Cluster 2	Cluster 3	Cluster 4	Cluster 5	Cluster 6	Cluster 7
Not reported/unknown	1	2	0	0	0	0	2
Weather Conditions							
Clear	43	12	46	15	19	46	33
Cloudy	17	5	6	1	3	6	8
Rain	10	2	19	1	3	13	7
Snow/ice/freezing rain	0	0	3	0	0	2	1
Not reported/unknown	0	2	0	0	0	0	2
Bicyclists and Pedestrians							
Involved							
Bicyclist	0	0	1	2	1	0	0
Pedestrian	1	2	1	0	1	2	2
Time Period							
Peak period	15	3	18	6	2	20	10
Off-peak period	58	18	56	11	23	48	41
Total crashes	73	21	74	17	25	68	51
Four-year average (rounded)	18	5	19	4	6	17	13
Segment crash rate	7.33	2.55	4.33	3.38	2.57	6.13	2.11
Average statewide crash rate							
for roadway segment (based							
on functional classification of							
roadway)	3.49	3.65*	3.49	3.49	3.49	3.49	3.49

Note: The AM peak period is 7:00 AM-9:00 AM; the PM peak period is 4:00 PM-6:00 PM.

Cluster 2 is on a minor arterial.

Shading denotes segment crash rate higher than the statewide average.

Source: Central Transportation Planning Staff.

According to the analysis results presented in Table 3, the predominant crash types were rear-end, angle, and single-vehicle crashes. Together, those types of crashes constituted between 75 percent and 85 percent of the crashes in the corridor. Some of the crashes occurred because of motorists failing to yield right-of-way, following too close, and being inattentive or distracted. However, lack of turn lanes contributed to the high number of turn-related crashes on Route 1A at the Swampscott Mall area and Vinnin Square, as seen in Clusters 1 and 3. High vehicular speeds, a sharp horizontal curve, limited sight distance, and inadequate signage also contributed to the crashes in Cluster 5. Also, recurring traffic queues contributed to the high number of rear-end collisions in Clusters 1, 6, and 7. Eleven pedestrian and bicycle crashes occurred at midblock locations or at intersections.

The crash rates for the roadway segments within the crash clusters (segment crash rates) ranged between 2.11 and 7.33 crashes per million vehicles-miles traveled (MVMT). The most recent 2016 statewide average crash rate for an urban principal arterial was 3.49 crashes per MVMT and 3.65 for a minor

arterial. 18 Crash Clusters 1, 3, and 6 had segment crash rates that exceeded the statewide average for a principal arterial. The segment crash rate for Cluster 2 was below the state average for minor arterials. The segment crash rates for Clusters 4, 5, and 7 were below the state average for a principal arterial.

5.1.2 Collision Diagrams

Figures 16-21 show the collision diagrams for crashes within each crash cluster. MPO staff used available police crash reports to prepare the collision diagrams; however staff were not able to obtain police crash reports for every crash. Therefore, the figures only show those crashes for which a police-drawn sketch of the crash scene was available. Collision diagrams are useful for examining patterns and developing safety strategies. The numbers in the collision diagram uniquely identify each crash and may be used to cross-reference the crash records. The collision diagrams, along with the crash records, are included in Appendix E.

5.2 TRAFFIC OPERATIONS ANALYSES

Staff conducted traffic operations analyses consistent with the Highway Capacity Manual (HCM) methodologies. ¹⁹ HCM methodology is used to assess traffic conditions at signalized and unsignalized intersections and rate the level-of-service (LOS) from A to F. LOS A represents the best operating conditions (little to no delay), while LOS F represents the worst operating conditions (long delay). LOS E represents operating conditions at capacity (the limit of acceptable delay). Table 4 presents the control delays (standards for comparison) associated with each LOS for signalized and unsignalized intersections.

TABLE 4
Intersection Levels-of-Service Criteria, 2010

intersection bevers of our vice official, 2010			
Level of	Signalized Intersection	Unsignalized Intersection	
Service	Control Delay (seconds per vehicle)	Control Delay (seconds per vehicle)	
Α	<10	<10	
В	10-20	10-15	
С	20-35	15-25	
D	35-55	25-35	
E	55-80	35-50	
F	> 80	> 50	

Source: Highway Capacity Manual 2010.

Published by MassDOT, based on crash information queried on January 8, 2016.
Massachusetts Department of Transportation website, crash rates,
http://www.massdot.state.ma.us/highway/Departments/TrafficandSafetyEngineering/CrashData/CrashRates.aspx

¹⁹ Highway Capacity Manual 2010, Transportation Research Board of the National Academies, Washington, DC, December 2010.

Using the traffic and signal data collected, MPO staff built traffic analysis networks for the weekday AM and PM peak hours and the Saturday PM peak hours. Synchro traffic simulation software was used to assess the capacity and quality of traffic flow. ²⁰ Figures 22-24 show the results of the existing conditions analyses in terms of LOS and delays for the weekday AM, weekday PM, and Saturday PM peak hours, respectively. The existing conditions LOS analysis worksheets are included in Appendix F.

With the exception of the intersection of Route 1A (Paradise Road) and Vinnin Street, none of the intersections in the corridor appeared to be failing. In general, traffic congestion in the study area was restricted to peak travel periods—because of the high volume of commuter and shopping trips—and confined to the following locations:

- Route 1A at Swampscott Mall
- Vinnin Square (Route 1A, Loring Avenue, and Vinnin Street)
- Leggs Hill Road at Tedesco Street

The high weekday PM peak period delay on Route 1A at Pickman Road is the result of traffic congestion and queues created at the intersection of Route 1A and Canal Street and Jefferson Avenue. At the time this report was published, this intersection was under construction and the proposed improvements were expected to reduce congestion and queues.

²⁰ Trafficware Inc., Synchro Studio 9.1, Synchro plus SimTraffic, Build 909, Revision 20, Sugar Land, Texas.

Chapter 6—Problems and Issues

6.1 OVERVIEW

As Vinnin Square developed commercially and residentially in recent years, traffic volumes and the number of pedestrians and bicyclists in the area increased. As a result, traffic safety, congestion, and mobility have become challenging issues. In addition, there is a growing need to improve mobility for residents of the neighborhoods adjacent to Vinnin Square, and to provide connections to enable them to access Vinnin Square and other destinations, such as recreational areas and schools.

Because the study area is large and specific segments of the area's roadways have unique problems, MPO staff divided the roadways in the study area into segments based on roadway character, land use, crash experience, and jurisdiction and ownership. The resulting roadway segments from south to north are as follows:

- 1. Route 1A from Ellis Street to Longwood Drive
- 2. Route 1A at Swampscott Mall
- 3. Essex Street and Loring Avenue, south of Vinnin Square
- 4. Vinnin Square
- 5. Tedesco Street, from Vinnin Square to Leggs Hill Road
- 6. Route 1A from Vinnin Square to Leggs Hill Road
- 7. Route 1A from Leggs Hill Road to Sumner Road

For each of the six roadway segments, MPO staff identified the problems and developed recommendations for addressing them. A summary of the problems identified on each segment are presented in Figure 25. They were identified through analyses of traffic and safety data, field reconnaissance, and discussions with the advisory task force. They include, but are not limited to, pedestrian and bicyclist issues, traffic safety and operations problems, and access management and control issues. Figures 26-31 show photographs of some of the problems identified on the roadway segments.

6.2 PEDESTRIAN AND BICYCLIST PROBLEMS

Figures 26, 29, and 30 show some of the challenges facing pedestrians and bicyclists in the study area. The following are some of the reasons why the roadways are considered unfriendly for pedestrians and bicyclists:

 Wide roadways create inequity by placing too much emphasis on vehicular use and by encouraging higher vehicle speeds, which puts pedestrians and bicyclists at risk.

- A lack of shoulders or bike lanes makes the roadways uncomfortable for bicyclists.
- Sidewalks are too close to the travel lanes, which creates an uncomfortable environment for pedestrians.
- A lack of crosswalks at some major intersections and side streets makes crossing challenging for pedestrians and puts them at risk.
- Obstructions in crosswalks, non-ADA compliant curb ramps, broken sidewalks, and sidewalk connectivity problems (gaps) create an unfriendly environment for pedestrians, especially for people with disabilities.
- A lack of bus shelters at heavily used stops creates problems for riders, especially during inclement weather.

6.3 TRAFFIC SAFETY AND OPERATIONS PROBLEMS

Figures 28 and 31 show some of the traffic congestion and operations problems facing motorists. They include the following:

- High vehicular speeds at the sharp horizontal curve on Route 1A at Leggs Hill Road results in many crashes.
- Outdated signal-timing plans need to be updated to make the flow of traffic efficient through the study area.
- High volumes of traffic on Route 1A and Vinnin Street creates congestion at Vinnin Square and the Swampscott Mall area.
- Lack of turn lanes and traffic queues cause a high number of crashes on Route 1A at the Swampscott Mall, Vinnin Square, and Route 1A between Harrison Road and Sumner Road.

6.4 ACCESS CONTROL AND MANAGEMENT PROBLEMS

Developing Vinnin Square and the Swampscott Mall into a vibrant destination with direct linkage to surrounding neighborhoods would require roadway, streetscape, and landscape improvements to accommodate all road users. Presently, the numerous driveways along the corridor contribute to many crashes in the square, and a lack of trees and greenery makes an unwelcoming environment for pedestrians and bicyclists.

Chapter 7—Improvements

7.1 TIME FRAME

MPO staff worked with the study's advisory task force to develop short-term, medium-term, and long-term strategies for addressing the pedestrian and bicyclist issues, traffic safety and operations problems, and access management issues identified in the corridor. The time frame categorized as "short-term" is typically less than five years. Short-term improvements are relatively uncomplicated and inexpensive to implement, and require minimal design efforts. Often maintenance or special funds are used to pay for these improvements. Typical examples of short-term improvements are pavement striping and sign installations. The time frame categorized as "medium-term" is typically between five and 10 years. Medium-term improvements are more complicated than their short-term counterparts and require more funding resources and design and engineering efforts. Examples of medium-term improvements are adding turn lanes at intersections, adding sidewalks, and signal retiming and coordination projects. Long-term improvements are complicated and the time frame for implementation is typically 10 or more years. They require more design and engineering efforts, environmental permitting, and large funding resources. Typical examples of long-term improvements are adding lanes and roadway reconstruction.

7.2 FUTURE TRAFFIC CONDITIONS

Planners typically use a planning model to systematically forecast future traffic volumes based on changes in the transportation network or land use. For this study, staff used the Boston Region MPO's regional travel demand model set, which was recently adopted for the development of the LRTP. This model's socioeconomic components are derived from forecasts produced by the Metropolitan Area Planning Council (MAPC). The model is calibrated at a regional level for 164 cities and towns, which includes the 101 cities and towns in the MPO's planning region. Using this model, staff projected that traffic on Route 1A, Essex Street, and Tedesco Street would grow by five percent between now and 2040. To test the impact the proposed improvements would have on future traffic conditions, staff increased the existing peak-hour turning movement volumes by five percent to develop 2040 projections.

7.3 CONCEPTS FOR IMPROVEMENTS

Most of the concepts for improvements that MPO staff developed would be carried out within the existing roadway's right-of-way—they require no land takings and take into account the needs of abutters and roadway users. The majority of the improvements could be completed in the short-term or medium-

term to make the study area's roadways safer and more attractive to pedestrians and bicyclists, while serving the needs of commuters and supporting economic activities and livable communities. A few of the proposed improvements would involve construction outside of the roadway's right-of-way. The following sections describe the proposed improvements on each roadway segment, their traffic operations and safety performances, advantages and disadvantages, and cost estimates.

7.4 ROUTE 1A FROM ELLIS STREET TO LONGWOOD DRIVE

7.4.1 Roadway Setting

Figure 32 shows the improvements developed for the segment of Route 1A between Ellis Street and Longwood Drive. The proposed improvements would reconfigure the roadway to accommodate bicyclists and make it safer for both pedestrians and bicyclists. The roadway would be reconfigured to include two 11-foot travel lanes, bike lanes, and sidewalks on both sides of the roadway. Additional improvements include reconstruction of the curb ramps to MassDOT's standards. The proposed roadway would have high visibility crosswalks across all town-owned streets that intersect Route 1A. It is expected that the renovations would make it easier and safer to walk and bike, and fulfill the vision of connecting the neighborhoods to Vinnin Square. The bike lanes would provide greater protection and higher visibility for both bicyclists and motorists, improving safety for all road users.

7.4.2 Operational Features

In addition to the renovations discussed above, providing the following features would modernize the roadway and make it safer and more efficient for all users:

- Street lighting to improve visibility at night
- Additional signs and pavement markings to designate the bike lanes
- Additional speed limit signs to alert drivers of reduced speed zones

7.4.3 Advantages

- Requires no land takings
- Makes Route 1A safer and more pedestrian- and bicyclist-friendly
- Transforms Route 1A into a neighborhood facility that supports livable communities and vibrant economic activities
- Reduces crashes in this segment, especially pedestrian- and bicyclistrelated collisions and rear-end collisions
- Roadway functions well and would not cause delays to motorists
- Maintains traffic flow and promotes multimodal transportation

- Consistent with MassDOT's Healthy Transportation Compact; the sidewalks and bike lanes would attract more people who would walk and bike
- Bike lanes appeal to people of various ages and bicycling ability; as such, adding bike lanes could increase bicycling volumes by as much as 10 to 20 percent ^{21, 22}

7.4.4 Disadvantages

There are no disadvantages associated with the proposed improvements except for impacts on traffic flow during construction that would affect commuters and business activities.

7.4.5 Cost

Based on the costs of similar reconstruction projects recorded in MassDOT's project information database, MPO staff estimate that the proposed improvements would cost between \$2 million and \$3 million. This estimate includes drainage improvements, sidewalks and ADA-compliant ramp construction, landscape and streetscape renovation, paving and markings, signing, and street lighting.

7.5 ROUTE 1A AT SWAMPSCOTT MALL

7.5.1 Roadway Setting

Four alternatives were developed for the segment of Route 1A from Longwood Drive to Vinnin Street.

Alternative 1

Figure 33 shows the proposed improvements in Alternative 1, which would renovate and reconfigure the roadway to make it safer for pedestrians and bicyclists. Alternative 1 would construct a two-way left-turn lane on the segment of Route 1A between the Swampscott Mall driveway and Vinnin Street to make traffic flow safely and more efficiently. The proposed roadway would have two 11-foot travel lanes, an 11-foot two-way left-turn lane, bike lanes, sidewalks on both sides of the roadway, and curb ramps reconstructed to MassDOT's standards. The proposed roadway would have high visibility crosswalks at the signalized intersections and across all town-owned streets and major driveways that intersect Route 1A. MPO staff recommend the installation of bus shelters at

²¹ New York City Department of Transportation, *Prospect Park West: Bicycle Path and Traffic Calming Update*, http://www.nyc.gov/html/dot/downloads/pdf/2012_ppw_trb2012.pdf

J. Parks, P. Ryus., A. Tanaka, C. Monsere, M. McNeil, J. Dill, and W. Schultheiss, District Department of Transportation Bicycle Facility Evaluation, Project No. 11404, (2012), http://ddot.dc.gov/node/477212

the heavily used MBTA bus stops on Route 1A at the Swampscott Mall intersection. The Town of Swampscott has expressed strong support for this alternative.

Alternative 2

Figure 34 shows the improvements proposed in Alternative 2. This alternative has all of the improvements proposed in Alternative 1, except that the two-way left-turn lane would be replaced with a median and an exclusive left-turn lane at the Summit Estates.

Alternative 3

Figure 35 shows the proposed improvements in Alternative 3, which would renovate and reconfigure Route 1A to increase capacity and make traffic flow efficiently. Alternative 3 would convert Route 1A into a four-lane roadway by adding a travel lane in each direction beginning at the site of Vinnin Liquor and Whole Foods and ending at Vinnin Street. The proposed roadway would have high visibility crosswalks at the signalized intersections and across all townowned streets and major driveways that intersect Route 1A. In addition, MPO staff recommend the installation of bus shelters on Route 1A at the heavily used MBTA bus stops at the Swampscott Mall.

Alternative 4

Alternative 4, shown in Figure 36, has all of the improvements proposed in Alternative 3, in addition to a median and an exclusive left-turn lane at the Summit Estates on the segment between the driveway to Swampscott Mall and Vinnin Street. The median would provide a pedestrian refuge area and make it safer and easier for pedestrians to cross Route 1A.

7.5.2 Operational Features

In addition to the proposed improvements outlined above, providing the following features would increase safety and modernize Route 1A to serve all users efficiently:

- New timing and coordination plans for traffic signals
- Accessible pedestrian signals at the signalized intersections
- Countdown timers for pedestrian crossings at the signalized intersections
- Detection for bicycles at the signalized intersections
- Street lighting to improve visibility at night
- Speed limit signs
- Signs and pavement markings to designate the bike lanes

7.5.3 Level of Service

The expected performance of the intersections, after implementation of the proposed improvements, is shown in Figures 37-42. Estimates of LOS and delay are provided for the weekday AM and PM and Saturday PM peak hours.

For each of the alternatives, the signalized intersections on Route 1A at Swampscott Mall would operate satisfactorily during the peak hours. The LOS analysis worksheets are included in Appendix F.

7.5.4 Advantages

Alternatives 1 and 2

- Transforms Route 1A into a safer and more pedestrian- and bicyclistfriendly transportation corridor
- Transforms Route 1A into a neighborhood amenity that supports livable communities and vibrant economic activities
- Consistent with MassDOT's Healthy Transportation Compact; the sidewalks and bike lanes would attract more people who would walk and bike, and better support the mixed land uses at Vinnin Square
- Roadway functions well and would not cause delays to motorists; traffic flow would be maintained while promoting multimodal transportation
- Reduces the high number of crashes that occur in the segment, especially left-turn-related crashes and angle and rear-end collisions
- Maximizes access to business driveways
- Requires minimal land takings

Alternatives 3 and 4

- Roadway functions well and would not cause delays to motorists; traffic flow would be maintained while serving multimodal transportation
- Reduces the high number of crashes that occur in the segment, especially left-turn-related crashes and angle and rear-end collisions
- Maximizes access to business driveways

7.5.5 Disadvantages

Alternatives 1 and 2

Construction and traffic management issues associated with Alternatives 2, 3, and 4 would impact traffic flow and business activities.

Alternatives 3 and 4

- Not bicyclist-friendly and does not support livable communities
- Requires significant land takings along Route 1A to accommodate the lane additions, as they would not fit into the existing right-of-way

- Costs considerably more to build compared to Alternatives 1 and 2, because of the relocation of utilities and land takings
- Construction and traffic management rerouting would impact traffic flow and business activities more than in Alternatives 1 and 2

7.5.6 Cost

Based on the costs of similar reconstruction projects recorded in MassDOT's project information database, MPO staff estimate that Alternatives 1 and 2 would each cost between \$4 million and \$6 million, and that Alternatives 3 and 4 would each cost between \$7 million and \$10 million.

The estimates exclude the cost of land takings required for the improvements. The estimates include the costs of drainage improvements, paving and markings, signing, accommodation and relocation of utilities, sidewalks and ADA-compliant ramp construction, streetscape amenities, traffic signals and equipment upgrades, and street lighting.

7.6 ESSEX STREET AND LORING AVENUE SOUTH OF VINNIN SQUARE

7.6.1 Roadway Setting

Figure 43 shows the improvements developed for Essex Street and Loring Avenue south of Vinnin Square. The proposed improvements would renovate and reconfigure the roadway to make travel safer and easier for pedestrians and bicyclists. The proposed roadway would include two 11-foot travel lanes, and bike lanes and sidewalks on both sides of the roadway. Sidewalks and curb ramps would be renovated to meet MassDOT's standards. In addition, MPO staff recommend installing bus shelters on Essex Street at the heavily used MBTA bus stops located at the Swampscott Mall driveway and on Loring Avenue at Carol Way. The proposed roadway would have high visibility crosswalks across all town-owned streets and major driveways that intersect Essex Street and Loring Avenue. The redesigned roadway would fulfill the vision of connecting the neighborhoods to services at Vinnin Square and the Swampscott Mall.

7.6.2 Operational Features

In addition to the proposed renovation efforts discussed above, providing the following features would modernize the roadway to increase safety and make Essex Street and Loring Avenue south of Vinnin Square efficient for all users:

- New timing plans for the traffic signal at Essex Street and the Swampscott Mall driveway
- Detection for bicycles at the signalized intersections
- Signs and pavement markings to designate the bike lanes
- Street lighting

Speed limit signs

7.6.3 Level of Service

The expected performance of the signalized intersections, after implementation of the proposed improvements, is shown in Figures 37-39. Estimates of LOS and delay are provided for the weekday AM and PM and Saturday PM peak hours. The signalized intersection would operate satisfactorily during the peak hours.

7.6.4 Advantages

- Requires no land takings
- Renovates Essex Street and Loring Avenue to be a more pedestrian- and bicyclist-friendly
- Consistent with MassDOT's Healthy Transportation Compact; the sidewalks and bike lanes would attract more people who would walk and bike
- Roadway functions well and would not cause delays to motorists; traffic flow would be maintained while serving multimodal transportation
- Reduces crashes in the roadway segment, especially left-turn-related crashes and angle and rear-end collisions
- Provides a bus shelter on Essex Street at the Swampscott Mall driveway, which would make taking the bus more comfortable and attractive to residents

7.6.5 Disadvantages

There are no disadvantages associated with the proposed improvements except that construction and traffic management issues associated with improvements to Essex Street and Loring Avenue would impact traffic flow and business activities.

7.6.6 Cost

Based on the costs of similar reconstruction projects recorded in MassDOT's project information database, MPO staff estimate the improvements to cost between \$2 million and \$3 million. This estimate includes the cost of drainage improvements, sidewalks and ADA-compliant ramp construction, landscape and streetscape renovation, paving and markings, signing, signal retiming, and street lighting.

7.7 VINNIN SQUARE

Two alternatives were developed for improving traffic safety and accommodating pedestrians and bicyclists at Vinnin Square.

7.7.1 Roadway Setting

Alternative 1

Figure 44 shows the proposed improvements in Alternative 1. All of the improvements are kept within the existing right-of-way to avoid land takings at Vinnin Square. The proposed improvements would mark the travel lanes on Route 1A and Loring Avenue for shared-use with bicyclists and renovate the sidewalks and curb ramps on this section of Route 1A to meet MassDOT's standards. Additional improvements would provide a welcoming environment for pedestrians at Vinnin Square by upgrading the streetscape, landscaping with greenery, and calming traffic. MPO staff recommend the installation of bus shelters on Loring Avenue for the heavily used MBTA bus stops located near the Vinnin Street intersection. In addition, MPO staff advise the City of Salem to work with the business owners in Vinnin Square on consolidating driveways on Route 1A to reduce turn related-crashes and provide a welcoming environment for pedestrians and bicyclists. In addition, MPO staff recommend installing high visibility crosswalks at the signalized intersections and across all city- or townowned streets and major driveways that intersect Route 1A or Loring Avenue.

Alternative 2

Figure 45 shows the proposed improvements in Alternative 2. They are similar to those in Alternative 1, except that the shared-use lanes are replaced with bike lanes on Route 1A. Alternative 2 would allow and provide continuous and connected bike lanes on Route 1A from Swampscott to Salem, a distance of approximately three miles.

7.7.2 Operational Features

In addition to the proposed improvements, providing the following features would increase safety and modernize the roadways at Vinnin Square to serve all users safely and efficiently:

- New timing and coordination plans for traffic signals
- Accessible pedestrian signals at the signalized intersections
- Countdown timers to assist pedestrians at the signalized intersections
- Detection for bicycles at the signalized intersections
- Signs and pavement markings to designate the shared-use lane or bike lanes
- Street lighting
- Speed limit signs

7.7.3 Level of Service

The expected performance of the intersections, after implementation of the proposed improvements, is shown in Figures 37-39. Estimates of LOS and delay

are provided for the weekday AM and PM and Saturday PM peak hours for both Alternatives 1 and 2. Either alternative would allow the signalized intersections on Route 1A, Loring Avenue, and Vinnin Street to operate satisfactorily during the peak hours.

7.7.4 Advantages

Alternative 1

- Requires no land-takings
- Roadway functions well and would not cause significant delays to motorists
- Provides a welcoming environment for pedestrians
- Provides bus shelters at Vinnin Square, which would make taking the bus more comfortable and attractive to residents
- Defines driveway access issues more clearly so that safety for vehicles entering and exiting local businesses can be improved and a pleasant sidewalk experience can be provided for pedestrians

Alternative 2

Alternative 2 has all of the benefits of Alternative 1 in addition to the following:

- Provides continuous and connected bike lanes on Route 1A and makes the roadway more pedestrian- and bicyclist-friendly
- Remakes Vinnin Square and Route 1A into a neighborhood amenity that supports livable communities and vibrant economic activities
- Consistent with MassDOT's Healthy Transportation Compact; the
 pedestrian amenities and bike lanes would attract more people who would
 walk and bike to Vinnin Square, thereby better supporting the mixed land
 uses at the square

7.7.5 Disadvantages

Alternative 1

- Creates a gap in the bike network on Route 1A
- Does not provide safe accommodation for bicyclists, thus would not well support the mixed land uses at Vinnin Square

Alternative 2

- Requires land takings for adding bike lanes
- Construction and traffic management issues associated with the improvements would impact traffic flow and affect commuters and business activities

7.7.6 Cost

Based on the costs of similar reconstruction projects recorded in MassDOT's project information database, MPO staff estimate that Alternative 1 would cost between \$1 million and \$2 million, and Alternative 2 would cost between \$3 million and \$4 million.

The estimates exclude the cost of land takings required for the improvements. The estimates include the cost of drainage improvements, accommodation and relocation of utilities, construction of sidewalks and ADA-compliant curb ramps, streetscape and landscape renovations, paving and markings, upgrades to traffic signals and equipment, new signal-timing plans, and street lighting.

7.8 TEDESCO STREET FROM VINNIN SQUARE TO LEGGS HILL ROAD

7.8.1 Roadway Setting

Figure 46 shows the improvements developed for Tedesco Street. The proposed improvements would reconfigure the roadway to make it safer for pedestrians and bicyclists. The proposed roadway consists of two 11-foot travel lanes with five-foot shoulders, and sidewalks on both sides of Tedesco Street. Additional improvements include the renovation of sidewalks and curb ramps to meet MassDOT's standards. Construction of a new sidewalk on the north side of Tedesco Street, from West Street to Vinnin Square, will help to close the gap in the sidewalk network. MPO staff also recommend the installation of rectangular rapid flash beacons for the midblock crosswalk on Tedesco Street at the Tedesco Country Club House. Other minor adjustments include the installation of bulb outs on Brookhouse Drive to allow vehicles to approach Tedesco Street at a more perpendicular angle, shorten the crosswalk distance, and reduce the speed of right-turning vehicles onto Tedesco Street. MPO staff recommend high visibility crosswalks across all town-owned streets and major driveways that intersect Tedesco Street.

7.8.2 Traffic Signal Warrant Analysis

Traffic control signals are valuable devices for controlling vehicular and pedestrian traffic. They assign the right-of-way to various traffic movements and thereby strongly influence traffic flow. Traffic control signals that are properly designed, located, operated, and maintained will provide orderly movement of traffic, and reduce congestion and the frequency and severity of certain types of crashes, especially right-angle collisions. Traffic control signals are not solutions to all traffic problems at intersections. Poorly designed and maintained, ineffectively placed, improperly operated, or unjustified traffic control signals can result in excessive delays, a significant increase in crashes (especially the rear-

end type), and diversion of traffic to less adequate routes, as road users attempt to avoid the traffic control signals.

Investigating the need for a traffic control signal at an unsignalized intersection involves analyzing factors related to the existing traffic operations and safety conditions at the intersection, as well as the potential to improve these conditions. Such an investigation is called a traffic signal warrant analysis. The *Manual on Uniform Traffic and Control Devices* (MUTCD) lists nine traffic signal warrants that justify installing a traffic signal.²³

Using the methodology outlined in the 2009 edition of the MUTCD, staff performed detailed traffic signal warrant analyses to determine whether the installation of a traffic control signal at the intersection of Tedesco Street and Leggs Hill Road is justified and if signalizing the intersection would improve safety and traffic operations.

Table 5 presents the results of the traffic signal warrant analyses; detailed traffic signal warrant analysis worksheets are included in Appendix F. Existing conditions at the intersection of Tedesco Street and Leggs Hill Road satisfy three of the warrants. However, the intersection has a low crash rate, and traffic delays on Leggs Hill Road occur only during the two-hour AM and two-hour PM peak travel periods. In addition, a peak-hour intersection capacity analysis also indicated that a traffic signal at the intersection would create traffic queues on Tedesco Street that would extend beyond West Street and block traffic entering or exiting Brookhouse Drive. Based on the results of the signal warrant and intersection capacity analyses, MPO staff does not recommend installing a traffic signal at Tedesco Street and Leggs Hill Road at this time.

TABLE 5
Traffic Signal Warrant Analysis
Tedesco Street and Leggs Hill Road Intersection

Warrant	Results
Warrant 1, Eight-Hour Vehicular Volume	Satisfied
Warrant 2, Four-Hour Vehicular Volume	Satisfied
Warrant 3, Peak Hour	Satisfied
Warrant 4, Pedestrian Volume	Not satisfied
Warrant 5, School Crossing	Not satisfied
Warrant 6, Coordinated Signal System	Not satisfied
Warrant 7, Crash Experience	Not satisfied
Warrant 8, Roadway Network	Not satisfied
Warrant 9, Intersection Near a Grade Crossing	Not satisfied

Source: Central Transportation Planning Staff.

²³ The MUTCD lists nine traffic signal warrants that justify installing a traffic signal. The warrants are listed in Table 5.

7.8.3 Operational Features

In addition to the proposed improvements discussed above, providing the following features would modernize the roadway to increase safety and make Tedesco Street efficient for all users:

- Street lighting
- Signs and pavement markings to define shoulder areas for use by bicyclists
- Speed limit signs

7.8.4 Level of Service

The expected performance of the intersection of Tedesco Street and Leggs Hill Road, after implementation of the proposed improvements, is shown in Figures 37-39. Estimates of LOS and delay are provided for the weekday AM and PM peak hours and Saturday PM peak hour. The signalized intersection would operate satisfactorily during the peak hours but traffic on Leggs Hill Road would face longer delays.

7.8.5 Advantages

- Renovates Tedesco Street to make the roadway more pedestrian- and bicyclist-friendly
- Renovates Tedesco Street to make it safer for all users
- Closes the gap in the sidewalk network on Tedesco Street
- Requires no land takings
- Consistent with MassDOT's Healthy Transportation Compact; the sidewalks and shoulders would attract more people who would walk and bike
- Roadway functions well, and would not cause delays to motorists
- Maintains traffic flow and promotes multimodal transportation

7.8.6 Disadvantages

There are no disadvantages associated with the proposed improvements, except that construction and traffic management would have an impact on traffic flow and affect commuters and business activities.

7.8.7 Cost

Based on the costs of similar reconstruction projects recorded in MassDOT's project information database, MPO staff estimates that the improvements will cost between \$1 million and \$2 million. This estimate includes drainage improvements, construction of sidewalks and ADA-compliant curb ramps, streetscape renovation, paving and markings, and street lighting.

7.9 ROUTE 1A FROM VINNIN SQUARE TO LEGGS HILL ROAD

7.9.1 Roadway Setting

Figure 47 shows the improvements proposed for the segment of Route 1A from Vinnin Square to Leggs Hill Road. The improvements were developed taking into consideration the recommendations of a RSA conducted in collaboration with the City of Salem, the Salem State University Police Department, and MassDOT. Presently, the roadway segment is striped to include two 11-foot travel lanes and 6-to-8-foot shoulders and sidewalks on both sides from Maple Avenue to Riverview Street. The proposed improvements would reconfigure the roadway by converting the existing shoulders into bike lanes, thereby increasing safety for bicyclists. MPO staff also recommend the construction of new sidewalks and curb ramps on the west side of Loring Avenue from Riverview Street to Harrison Road to close the gap in the sidewalk network. Closing the gap in the sidewalk network on Route 1A would benefit the impending school swap that will move the Horace Mann Laboratory School to the Salem State University South Campus. In addition, MPO staff recommend installing a midblock crosswalk with pedestrian signals and advance-pedestrian-warning signs on Route 1A between Riverview Street and Leggs Hill Road. The midblock crosswalk should be located where it will be visible to approaching drivers; adequate sight distance must be considered. Other recommendations include the installation of high visibility crosswalks across all town-owned streets and major driveways that intersect Route 1A, and curve-warning signs and street lighting on the sharp horizontal curve in the vicinity of Leggs Hill Road.

7.9.2 Operational Features

In addition to the renovations discussed above, providing the following features would modernize Route 1A, increase safety, and make the roadway more efficient for all users:

- Signs and pavement markings to designate the bike lanes
- Speed limit signs

7.9.3 Advantages

- Renovates Route 1A into a more pedestrian- and bicyclist-friendly roadway
- Renovates Route 1A to better serve students of Horace Mann Laboratory School at the future location of the school on Harrison Road off of Route 1A
- Closes the gap in the sidewalk network on Route 1A
- Improves safety for all road users

- Consistent with MassDOT's Healthy Transportation Compact; the sidewalks and bike lanes would attract more people who would walk and bike
- Roadway functions well and traffic flow would be maintained while promoting multimodal transportation
- Fulfills the vision of connecting the neighborhoods to places such schools, recreational areas, and the Salem Bike Trail
- Requires no land takings

7.9.4 Disadvantages

There are no disadvantages associated with the proposed improvements.

7.9.5 Cost

Based on the costs of similar reconstruction projects recorded in MassDOT's project information database, MPO staff estimate the improvements to cost between \$1million and \$2 million. This estimate includes drainage improvements, construction of sidewalks and ADA-compliant curb ramps, streetscape renovation, paving, and street lighting.

7.10 ROUTE 1A FROM LEGGS HILL ROAD TO SUMNER ROAD

7.10.1 Roadway Setting

Figure 48 shows the improvements proposed for the segment of Route 1A from Leggs Hill Road to Sumner Road. The proposed improvements include the recommendations from the RSA conducted in collaboration with the City of Salem, Salem State University Police Department, and MassDOT. The proposed improvements reconfigure the roadway by converting the existing 6-to-8 foot shoulders into bike lanes to increase safety for bicyclists and by revamping the sidewalks and curb ramps to bring them to MassDOT's standards. The improvements include retiming the traffic signals at Harrison Road and Pickman Roads, adding countdown timers and accessible pedestrian signals to increase safety for pedestrians, and installing of high visibility crosswalks across all townowned streets and major driveways that intersect Route 1A. Additional improvements include the installation of pedestrian- and school-warning signs to alert drivers of pedestrians in crosswalks, and the installation of bus shelters at the stops on Route 1A between Harrison Road and Lincoln Road. These improvements are expected to benefit the impending school swap that will move the Horace Mann Laboratory School to the Salem State University South Campus.

Finally, MPO staff recommend that the City of Salem work with MassDOT, MassRIDES, and Walk Boston to conduct a comprehensive Safe Routes to

School study, before the school swap occurs, to identify appropriate safe routes to the new school, pedestrian and bicyclists' needs, and safety improvements that would encourage students to walk and bike to the relocated school.

7.10.2 Operational Features

In addition to the renovation efforts discussed above, providing the following features would modernize Route 1A, increase safety, and make the roadway more efficient for all users:

- Street lighting
- Signs and pavement markings to designate the bike lanes
- Speed limit signs

7.10.3 Level of Service

The expected performance of the Route 1A intersection at Harrison Road and Pickman Road, after implementation of the proposed improvements, is shown in Figures 37-39. Estimates of LOS and delay are provided for the weekday AM and PM and Saturday PM peak hours. Analyses show that the two signalized intersections would operate satisfactorily during the peak hours.

7.10.4 Advantages

- Renovates Route 1A into a more pedestrian- and bicyclist-friendly roadway
- Renovates Route 1A to better serve students of the Horace Mann Laboratory School at the future location of the school on Harrison Road off of Route 1A
- Improves safety for all road users
- Consistent with MassDOT's Healthy Transportation Compact; the sidewalks and bike lanes would attract more people who would walk and bike
- Roadway functions well, and traffic flow would be maintained while promoting multimodal transportation
- Enhances safety for people who would walk or bike to school or work, or for recreation
- The bike lanes would provide better protection and visibility for bicyclists
- Fulfills the vision of connecting the neighborhoods to schools, recreational areas, and the Salem Bike Trail
- Requires no land takings

7.10.5 Disadvantages

There are no disadvantages associated with the proposed improvements.

7.10.6 Cost

Based on the costs of similar reconstruction projects recorded in MassDOT's project information database, MPO staff estimate the project to cost between \$2 million and \$3 million. This estimate includes drainage improvements, construction of sidewalks and ADA-compliant curb ramps, streetscape renovation, paving and markings, signing, and street lighting.

7.11 EXAMPLES OF MODEL ROADWAYS AND FEATURES

The improvements proposed in this study would increase safety, balance the needs of all transportation users, expand mobility, improve public health, support economic activities, reduce emissions for a cleaner environment, and create stronger communities. Figure 49 shows examples of accessible curb ramps that comply with MassDOT's standards and high visibility crosswalks that increase safety for pedestrians. Figure 50 shows examples of sidewalk designs that provide welcoming sidewalk experiences and median cuts that offer pedestrians refuge areas. Figure 51 shows examples of MBTA bus shelters proposed for the heavily used MBTA bus stops and examples of pedestrian signals that can be used to increase safety for pedestrians at midblock crosswalks on high-traffic-volume roadways. Finally, Figure 52 shows photographs of other roadways in the Boston region that received the type of treatments described in this report; they include Route 109 in Westwood, Route 135 in Natick, and Route 109 in Medway.

Chapter 8—Conclusion and Next Steps

8.1 CONCLUSIONS

The above analyses and evaluations indicate that Route 1A and the ancillary streets surrounding it in Marblehead, Salem, and Swampscott need renovations to improve safety and mobility for motorists, pedestrians, and bicyclists. This study identified the transportation issues related to safety, operations, and mobility, and proposed solutions to address needs in those areas. This study aligns with the Boston Region MPO's goals of modernizing roadways to reduce congestion, increasing safety on the region's highway system, expanding the quantity and quality of walking and bicycling infrastructure, and making transit service more efficient and modern. The proposed improvements offered in this report, if implemented, would increase traffic safety, make traffic operations more efficient, and modernize the roadway to accommodate all users.

8.1.1 Problems

There are several reasons why pedestrians and bicyclists find navigating the roadways in the study area challenging and why the roadways are considered unfriendly for those users: wide roadways create inequity by placing too much emphasis on vehicular use and by encouraging higher vehicle speeds; a lack of shoulders or bike lanes makes the roadways uncomfortable for bicyclists; and sidewalks are too close to the travel lanes, which causes discomfort for pedestrians. Furthermore, obstructions in crosswalks, curb ramps that are not ADA compliant, broken sidewalks, and sidewalk connectivity problems (gaps) create an unfriendly environment for pedestrians, especially for people with disabilities. Also, a lack of bus shelters at the heavily used stops creates problems for bus riders, especially during inclement weather.

Among the traffic operations problems facing motorists are high vehicular speeds at the sharp horizontal curve on Route 1A at Leggs Hill Road in Salem, where many crashes have occurred. Also, outdated signal-timing plans make the flow of traffic inefficient through the Vinnin Square District. Additionally, high volumes of traffic and a lack of left-turn lanes on Route 1A causes a high number of crashes, traffic queues, and congestion near Swampscott Mall, in Vinnin Square, and on Route 1A between Harrison Road and Sumner Road.

8.1.2 Solutions

MPO staff, working with the study's advisory task force, developed recommendations for improvements that could transform the study area into a pedestrian- and bicyclist-friendly transportation corridor that serves all modes of transportation and maintains regional travel capacity. MPO staff evaluated

different roadway cross sections to determine how best to accommodate all road users safely and fulfill the vision of connecting the neighborhoods to the Vinnin Square District, educational institutions, and recreational areas, thereby fostering cohesive land uses, connecting people and their destinations, and promoting economic activity. This study provides the City of Salem, Towns of Marblehead and Swampscott, MassDOT, and other stakeholders with an assessment of the transportation needs at Vinnin Square and the adjoining neighborhoods in light of past, recent, and future developments, and allows them to start planning projects to implement the recommended improvements. When selecting preferred improvements to advance through the planning process, planners should consider the advantages and disadvantages of each alternative, cost, effectiveness, and impacts on their goals and objectives.

8.1.3 Project Implementation

Because there are multiple jurisdictions in the study area, successful implementation of the proposed improvements would require cooperation between MassDOT Highway Division and the municipalities to ensure that signal equipment can communicate and share data; sidewalks, shoulders, and bike lanes are continuous and connected; and MassDOT's standards guide the design of roadway elements such as curb ramps, bike lanes, sidewalks. It is important for stakeholders to examine the design concepts with all road users in mind.

MassDOT owns Route 1A and would be responsible for any renovations to the following sections of Route 1A:

- Route 1A from Ellis Street to Longwood Drive in Swampscott
 The proposed improvements on this segment are estimated to cost
 between \$2 million and \$3 million. This estimate includes sidewalk
 renovation, installation of ADA-compliant curb ramps, the addition of new
 bike lanes, and pavement resurfacing and marking, as well as signage,
 street lighting, and drainage improvements. The Town of Swampscott has
 expressed support for the improvements.
- Route 1A from Longwood Drive to Vinnin Street in Swampscott
 Four alternatives were developed for this segment. The Town of
 Swampscott has expressed strong support for Alternative 1— a two-lane,
 two-way roadway with a two-way left-turn lane, and bike lanes and
 sidewalks on both sides. Additional recommendations include
 improvements and upgrades to the streetscape, bus shelters, traffic
 signals and equipment, signage, and drainage. Alternative 1 is estimated
 to cost between \$4 million and \$6 million.

Route 1A from Vinnin Square to Sumner Road

The proposed improvements would reconfigure the roadway by converting the existing shoulders into bike lanes, renovating existing sidewalks and curb ramps, and constructing new sidewalks and crosswalks to close the gap in the sidewalk network. Additional improvements include traffic signal equipment upgrades; safety improvements to support the impending school swap that will move the Horace Mann Laboratory School to the Salem State University South Campus; the addition of bus shelters at the heavily used MBTA bus stops; and signing, drainage, and streetscape improvements. MPO staff estimate the improvements to cost between \$2 million and \$4 million.

The City of Salem and the Towns of Marblehead and Swampscott own the arterials and the local collectors that connect to Route 1A. These municipalities would be responsible for any renovation to the roadways under their jurisdiction:

Essex Street and Loring Avenue South of Vinnin Square

The Town of Swampscott has jurisdiction of Essex Street and the City of Salem has jurisdiction of Loring Avenue. The proposed improvements would reconfigure the roadway to make it safer and easier for pedestrians and bicyclists to traverse. The reconfigured roadway would include two 11-foot travel lanes and bike lanes and sidewalks on both sides of the roadway. Additional improvements include the installation of a bus shelter at the stop located near the Swampscott Mall entrance on Essex Street. MPO staff estimate the improvements to cost between \$2 million and \$3 million.

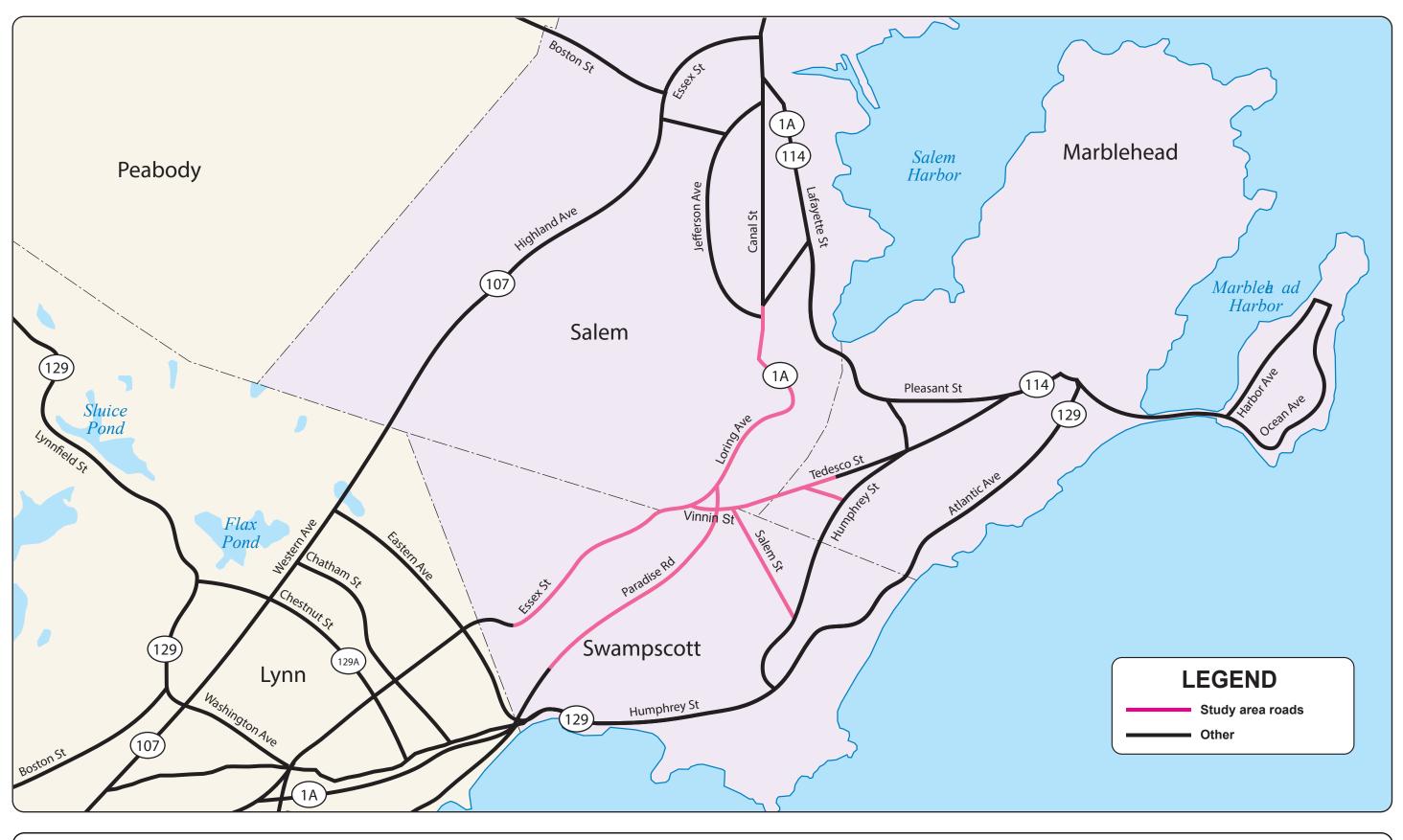
Tedesco Street from Vinnin Square to Leggs Hill Road

The Town of Marblehead has jurisdiction over Tedesco Street. The proposed improvements would convert the roadway into a two-way, two-lane facility with sidewalks and shoulders on both sides of the street. Other improvements would close the gap in the sidewalk network, renovate curb ramps to MassDOT's standards, and install the following: rectangular rapid flash beacons in the midblock crosswalks; minor adjustments (bulb outs) on the Brookhouse Drive approach, which would shorten the crosswalk distance and reduce the vehicle speeds; and high visibility crosswalks. MPO staff estimate the improvements to cost between \$1 million and \$2 million.

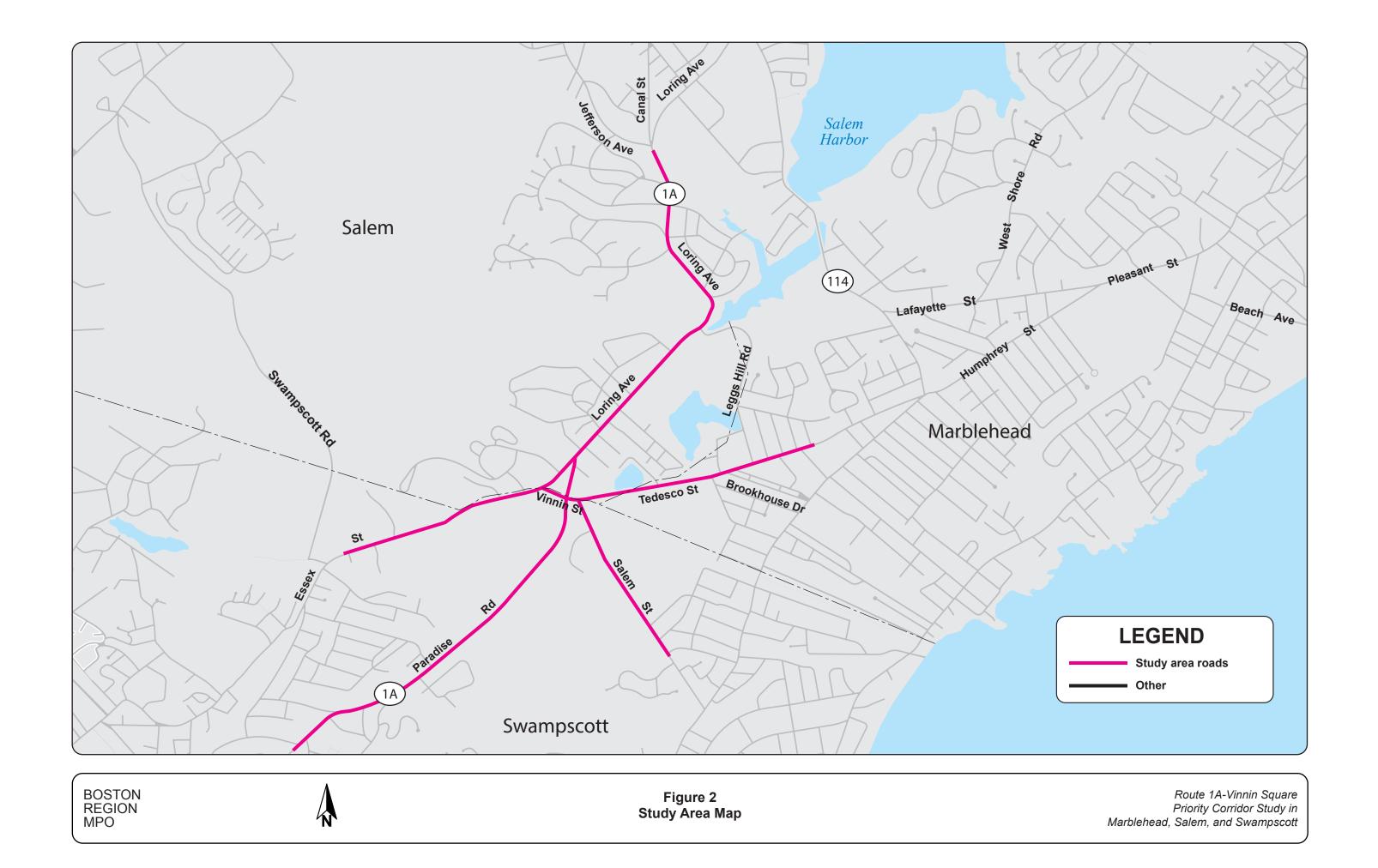
8.1.4 Project Development

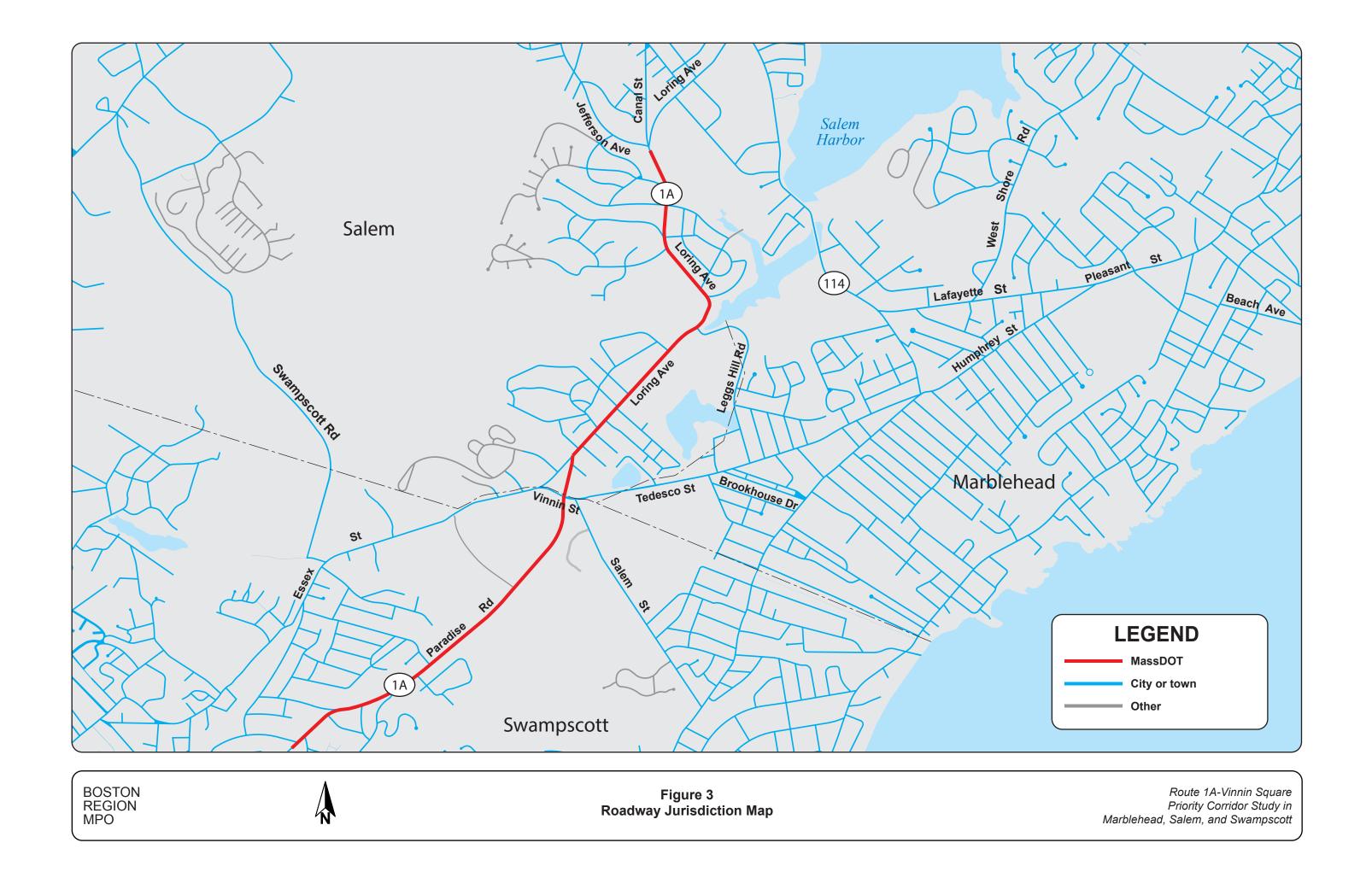
Transportation decision making is complex and is influenced by factors such as financial limitations and agencies' programmatic commitments. Project development is the process that takes transportation improvements from concept to construction. This process will depend upon cooperation between MassDOT, Marblehead, Salem, Swampscott, and the MPO. This planning study and the two RSAs provide the necessary information for the project proponents to initiate the project notification and review process. After completing the initial steps, the proponents can start preliminary design and engineering and place the project on the TIP. An overview of the project development process is included in Appendix G.

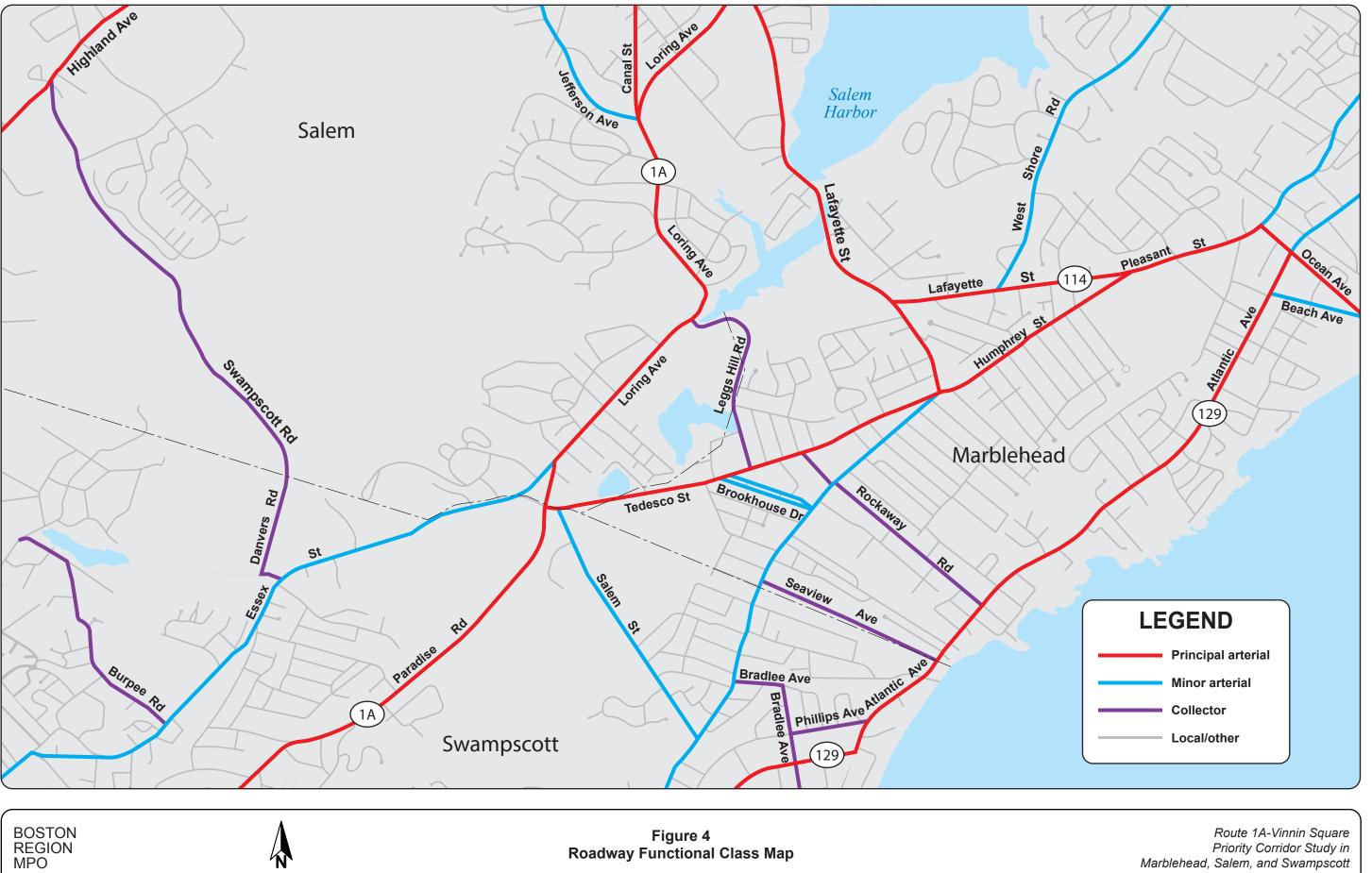
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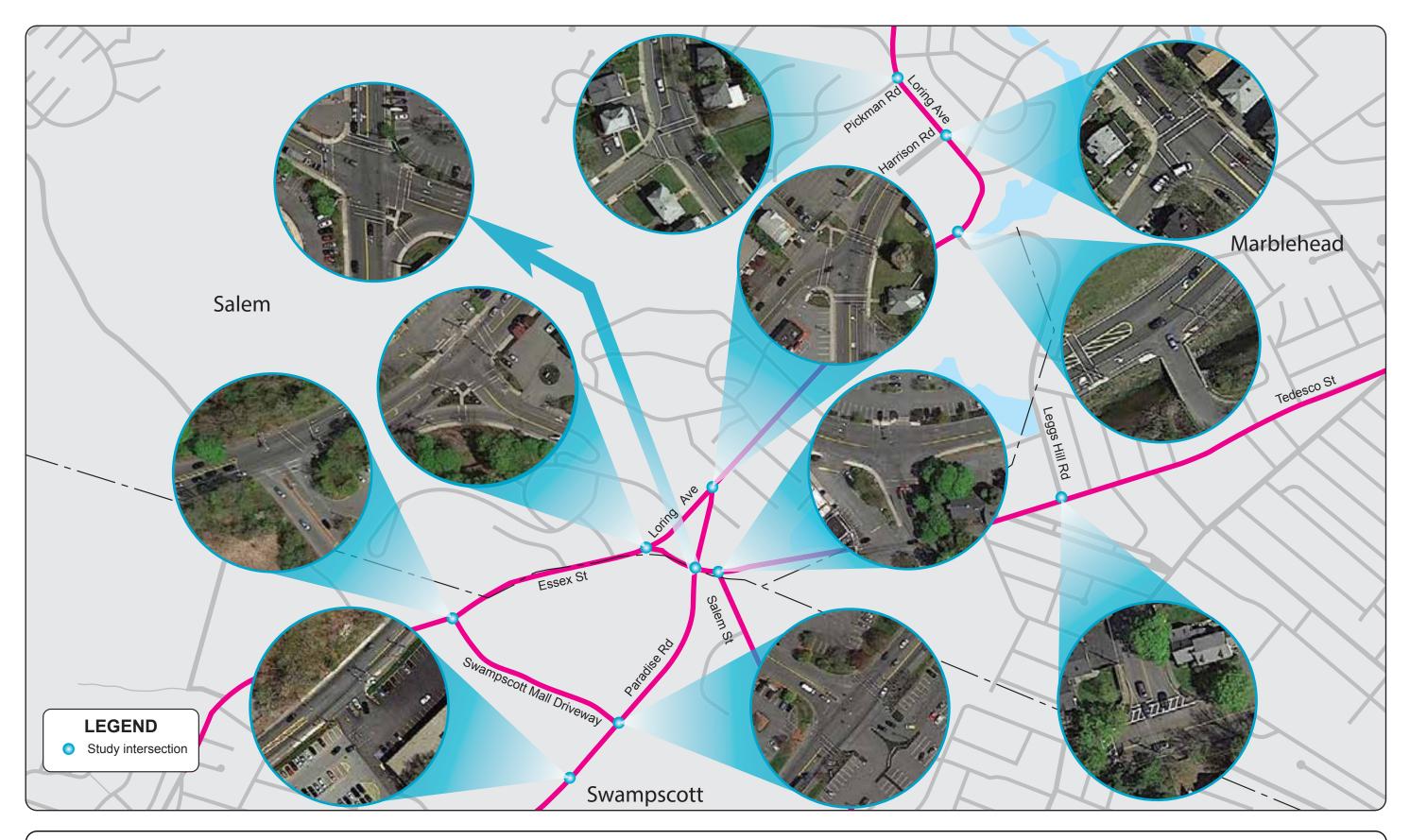


Route 1A-Vinnin Square Priority Corridor Study in Marblehead, Salem, and Swampscott

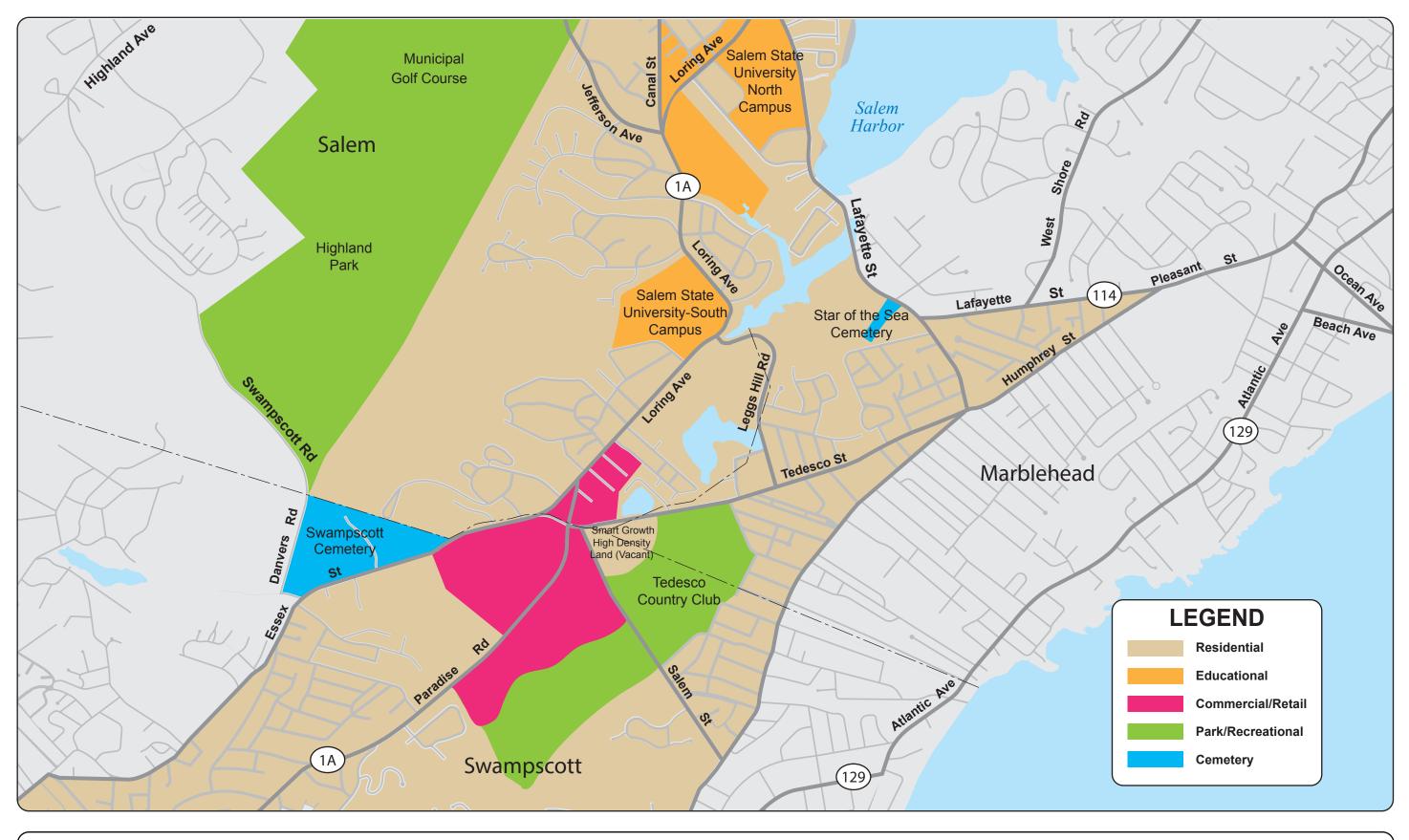




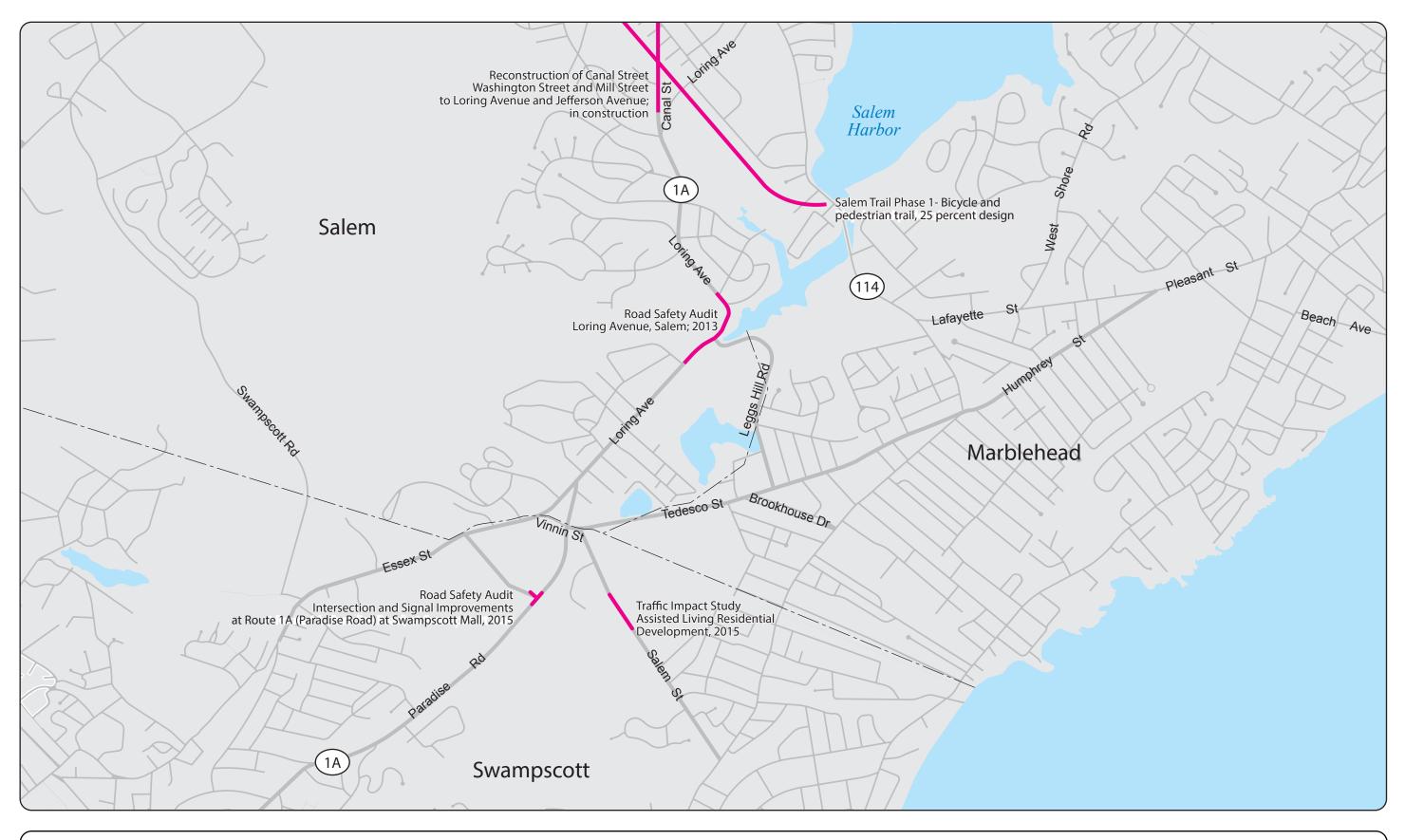




BOSTON REGION MPO Route 1A-Vinnin Square Priority Corridor Study in Marblehead, Salem, and Swampscott

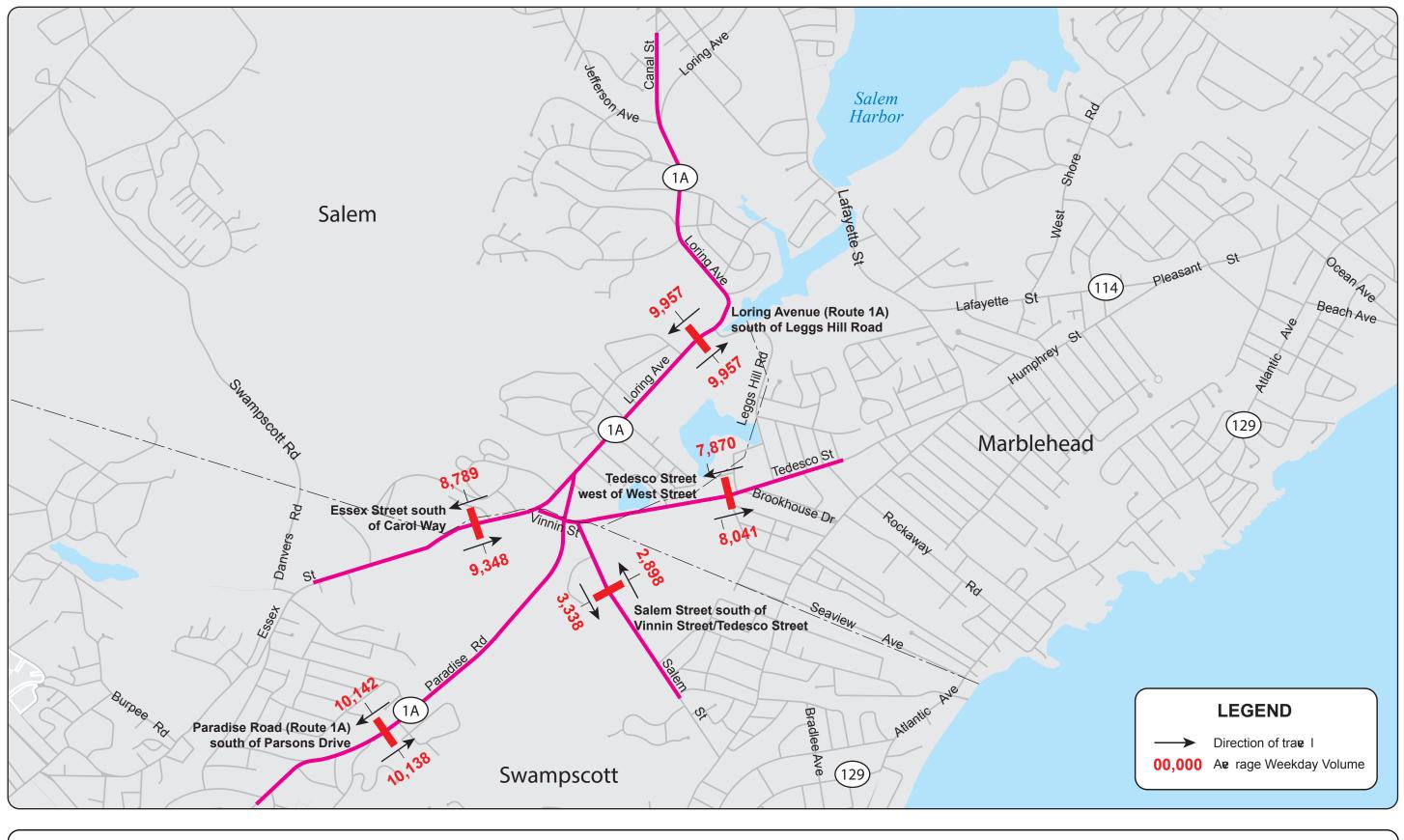


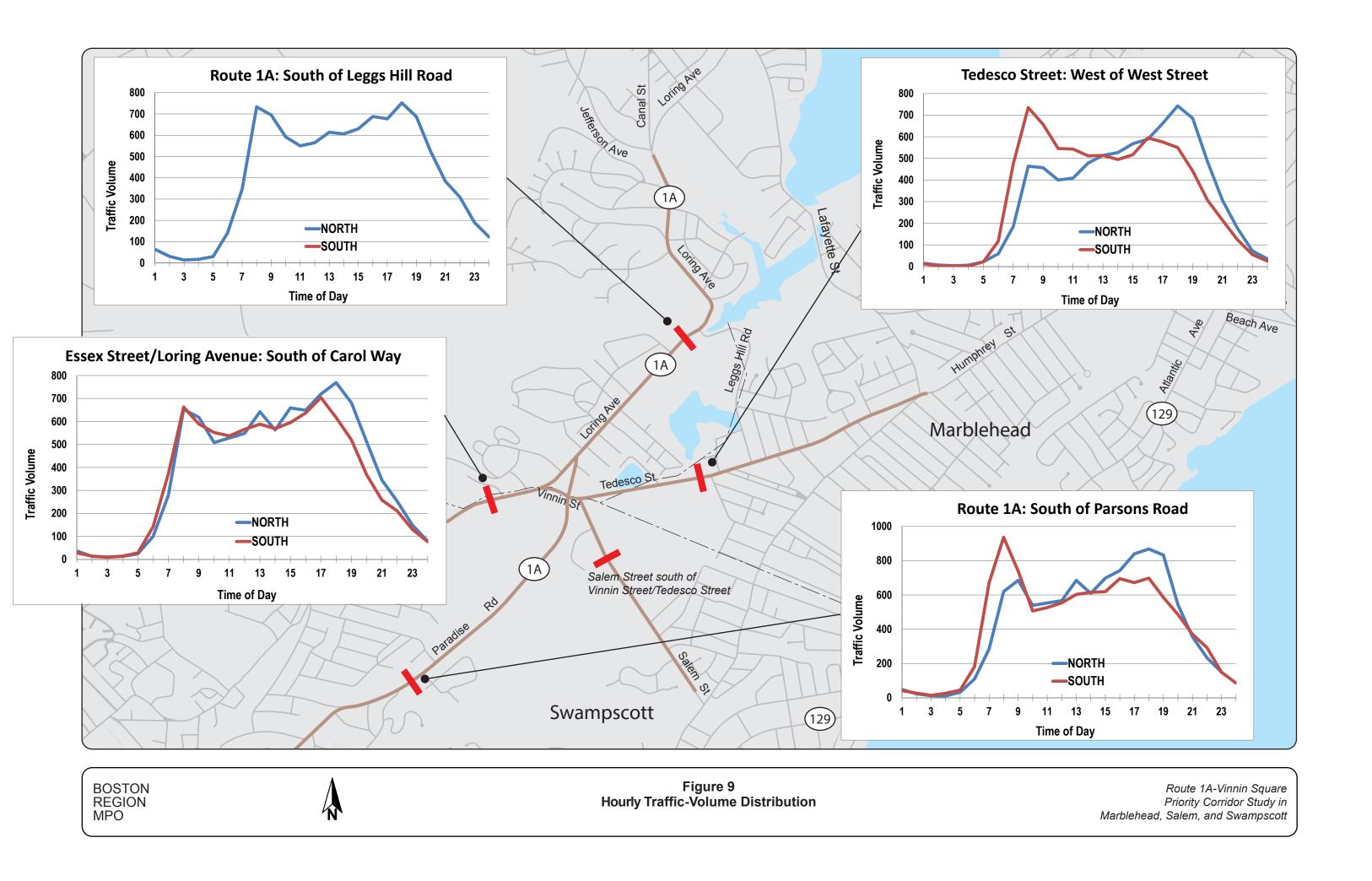
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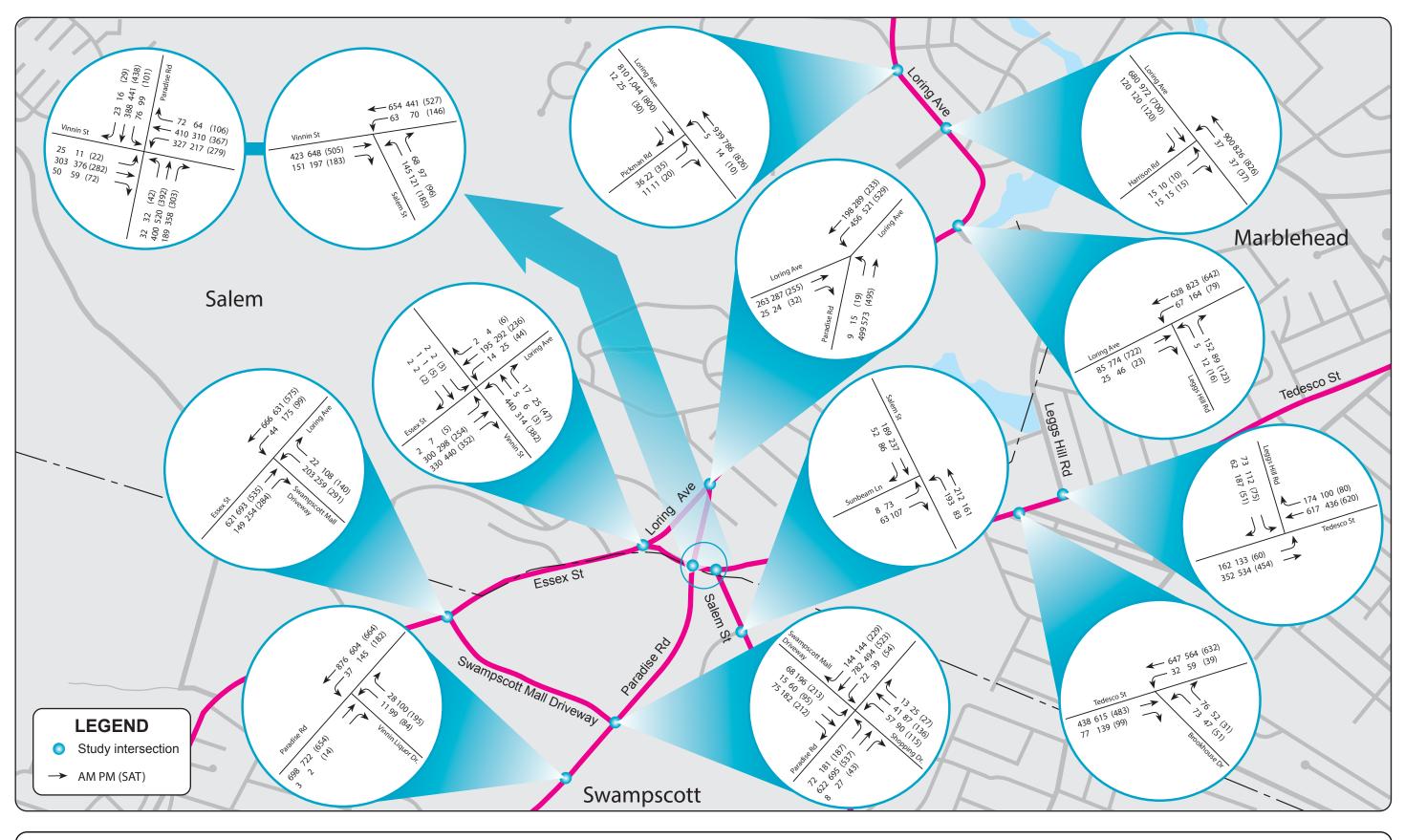


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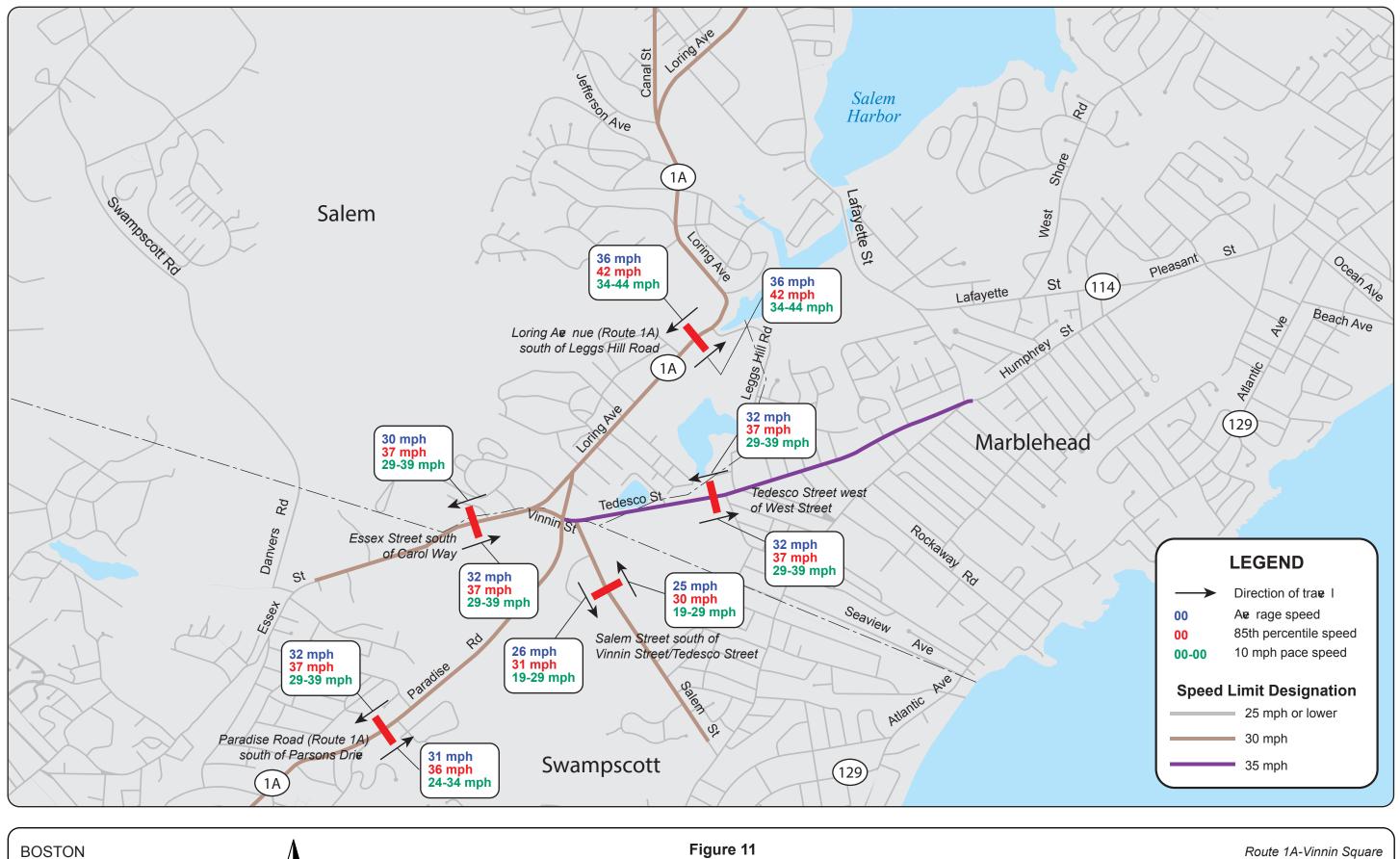






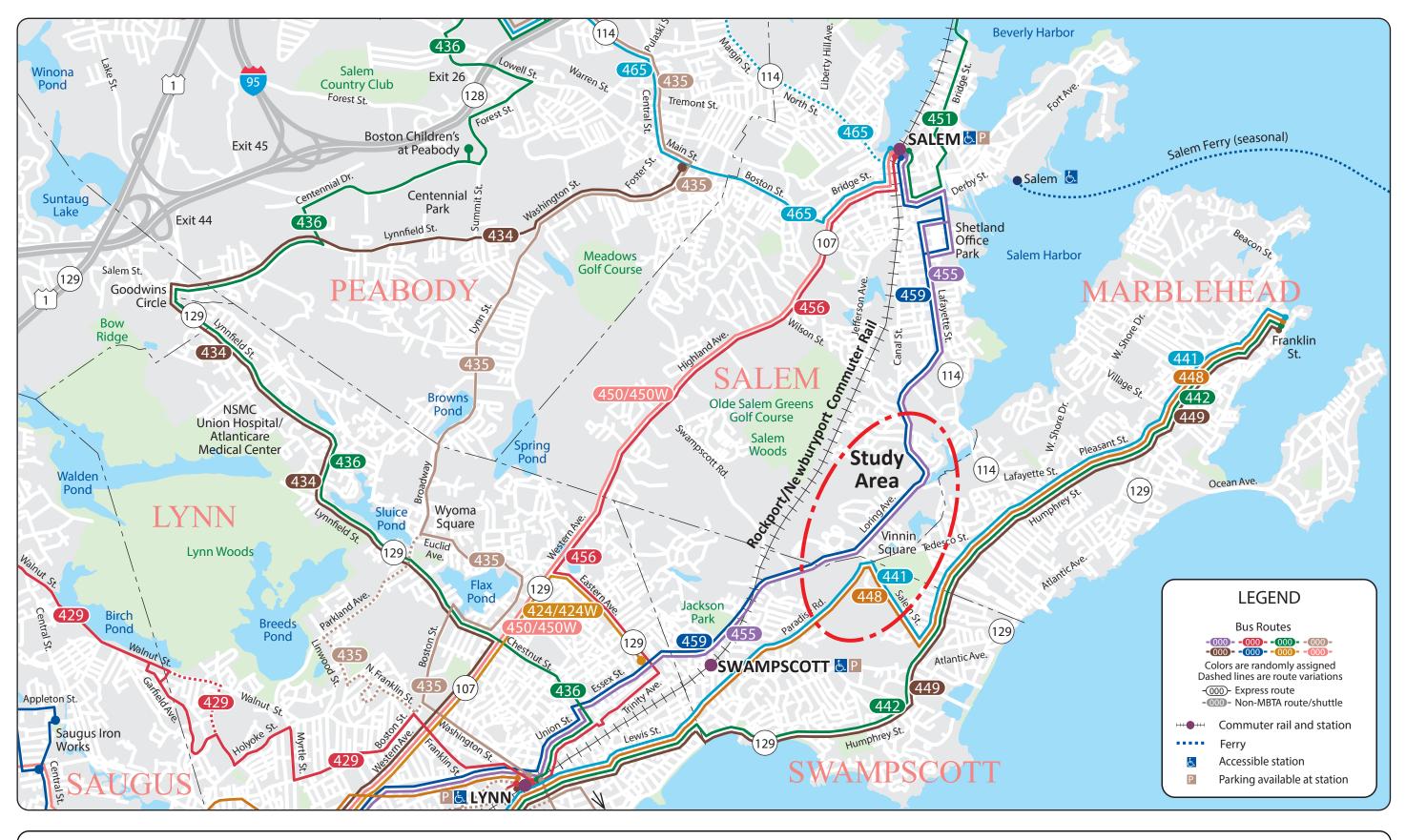
BOSTON REGION MPO

Route 1A-Vinnin Square Priority Corridor Study in Marblehead, Salem, and Swampscott



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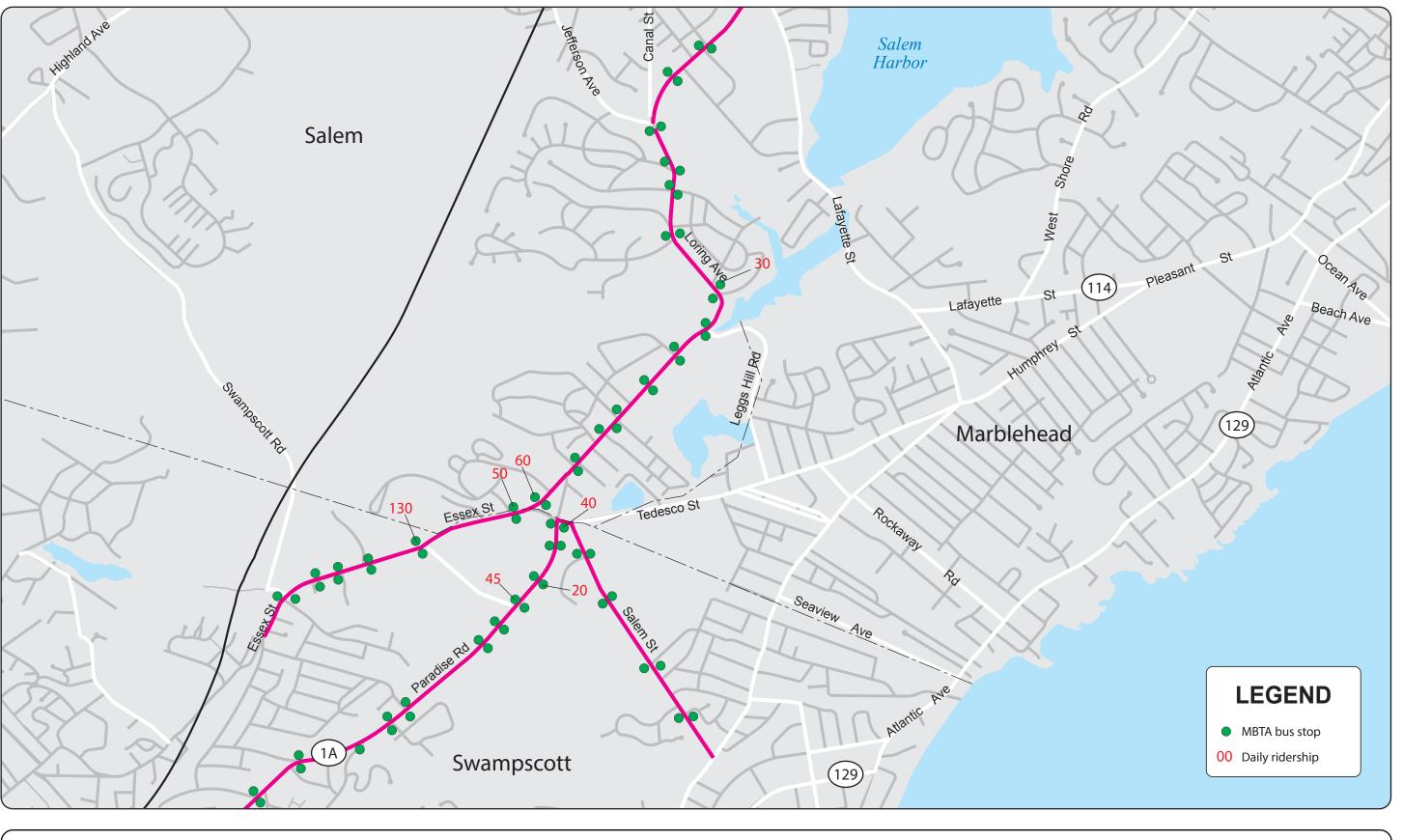




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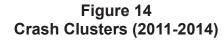


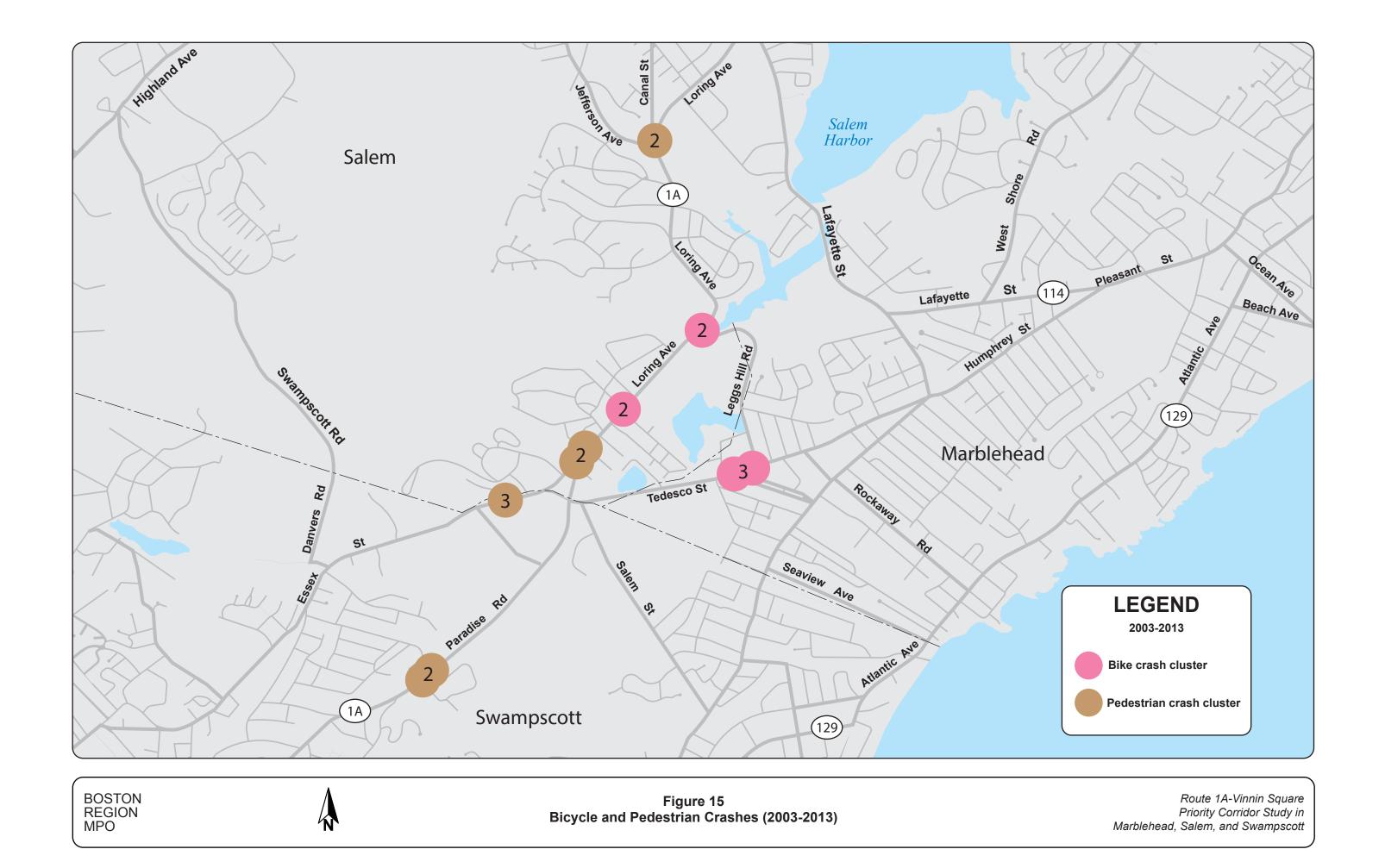


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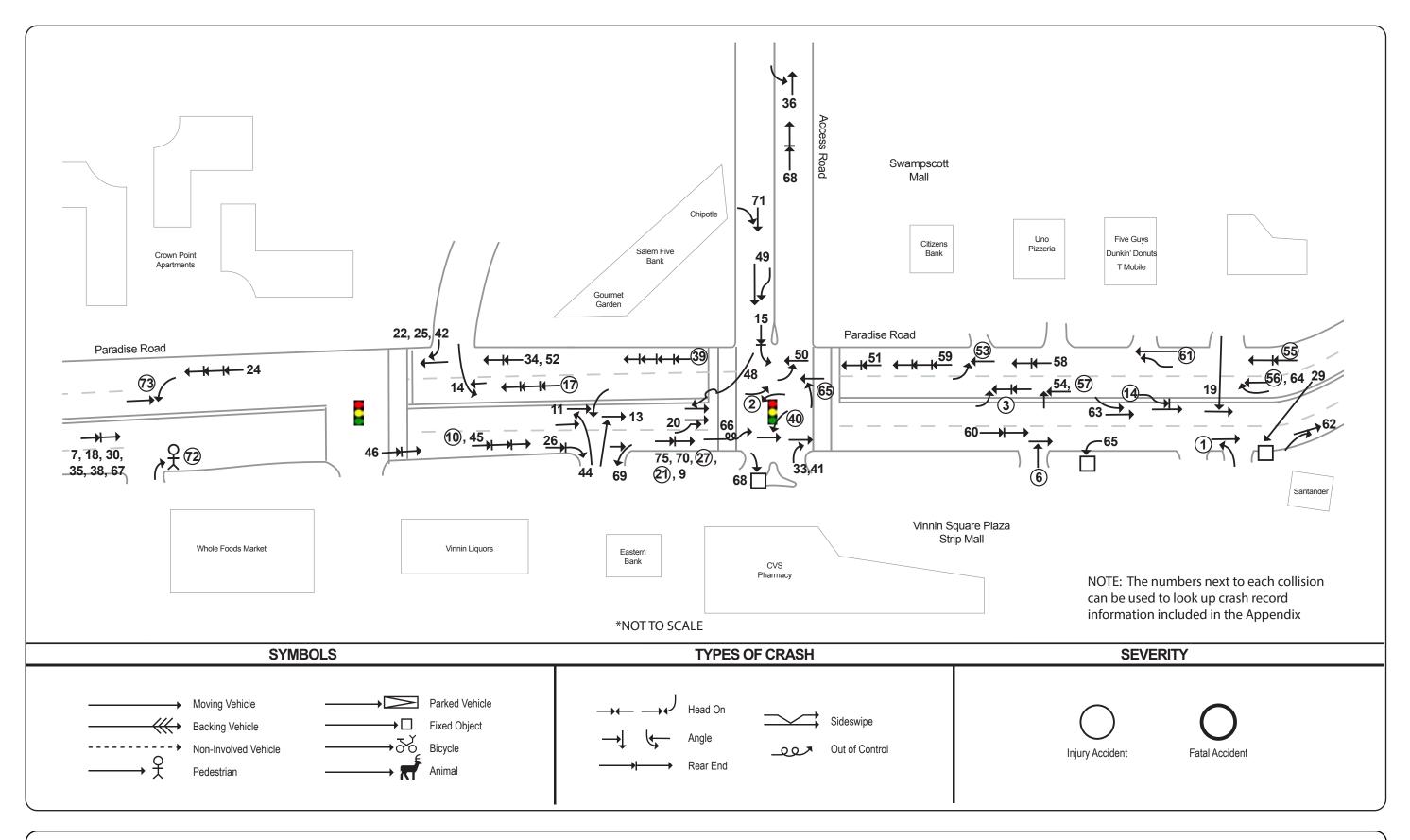
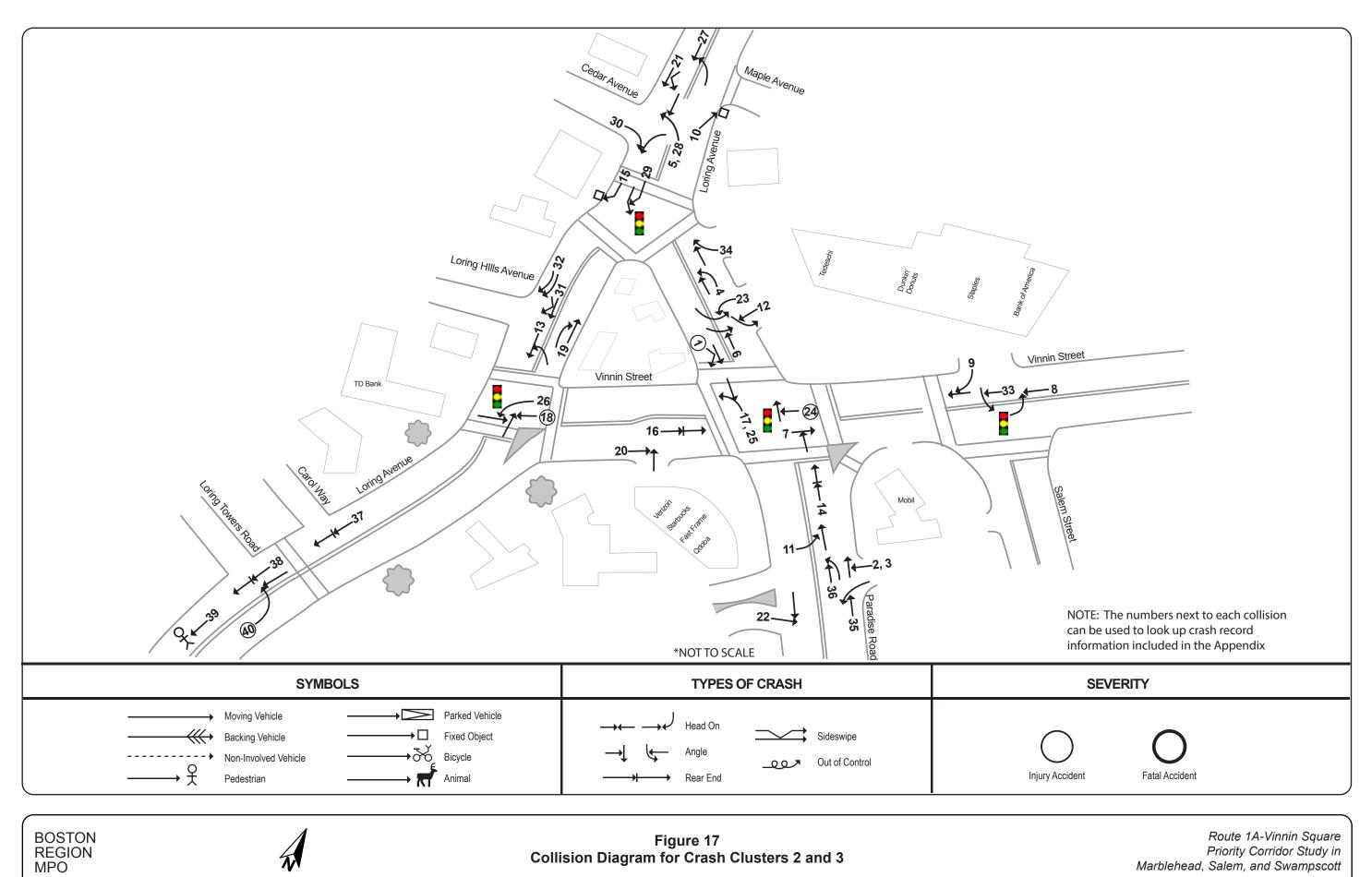




Figure 16
Collision Diagram for Crash Cluster 1





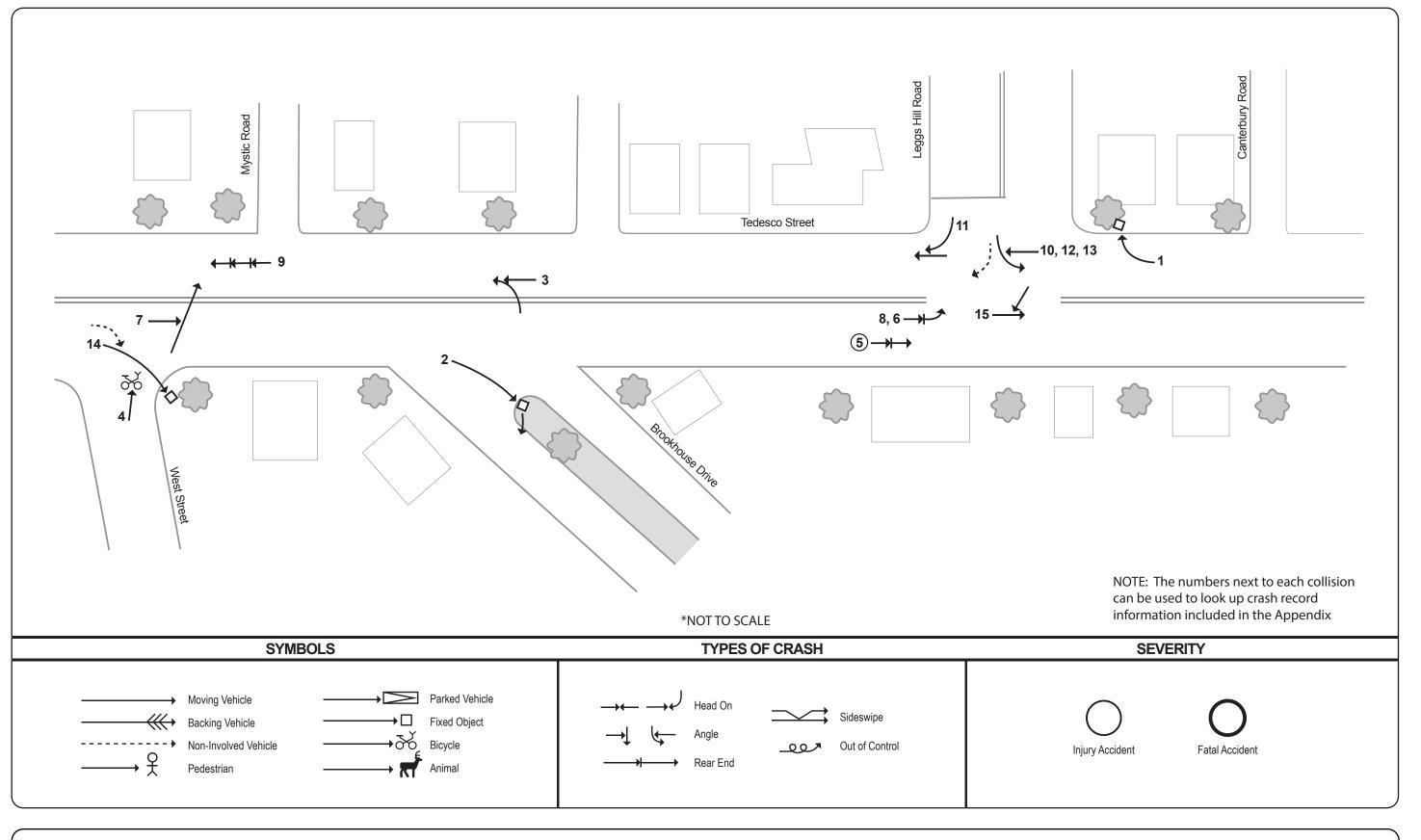
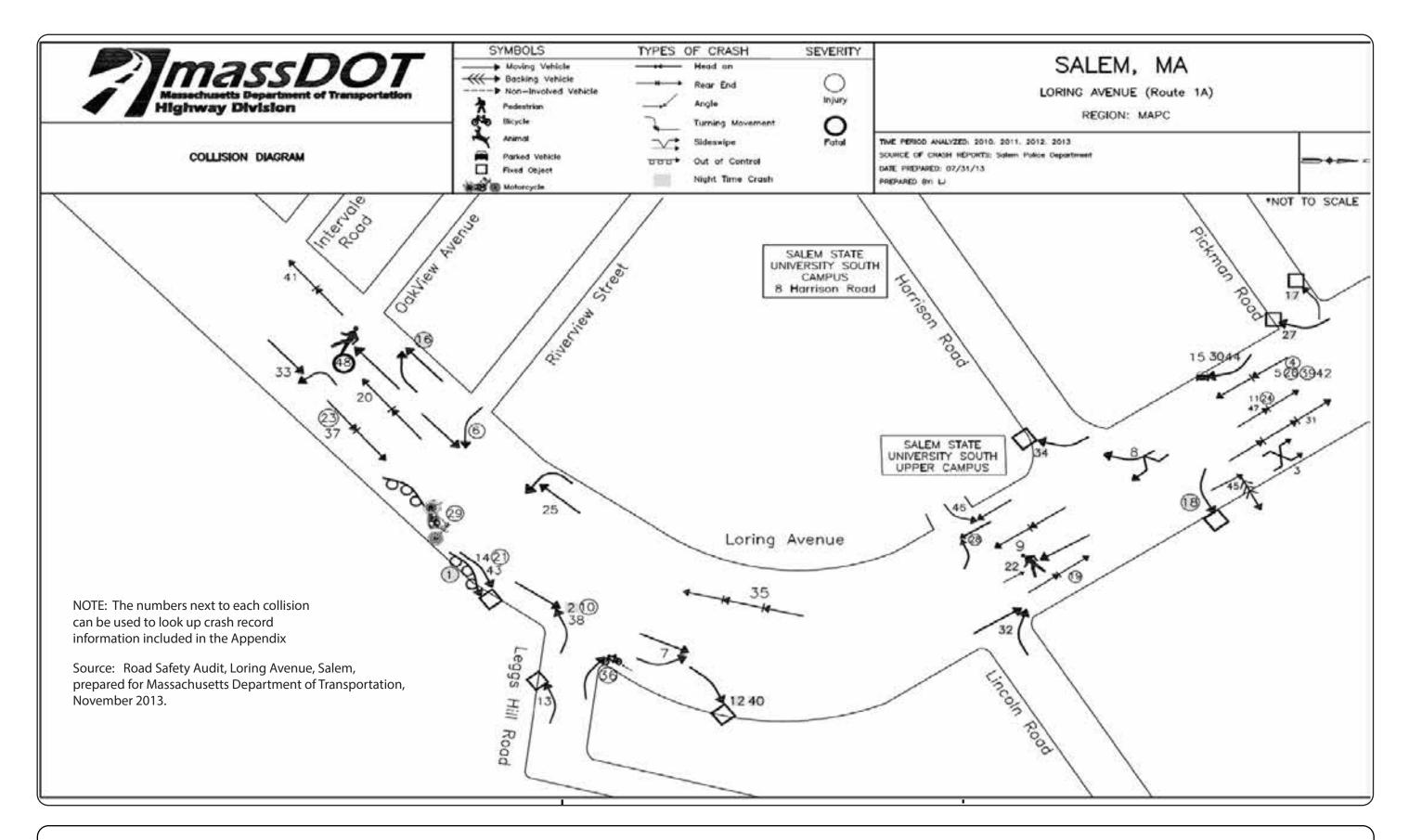
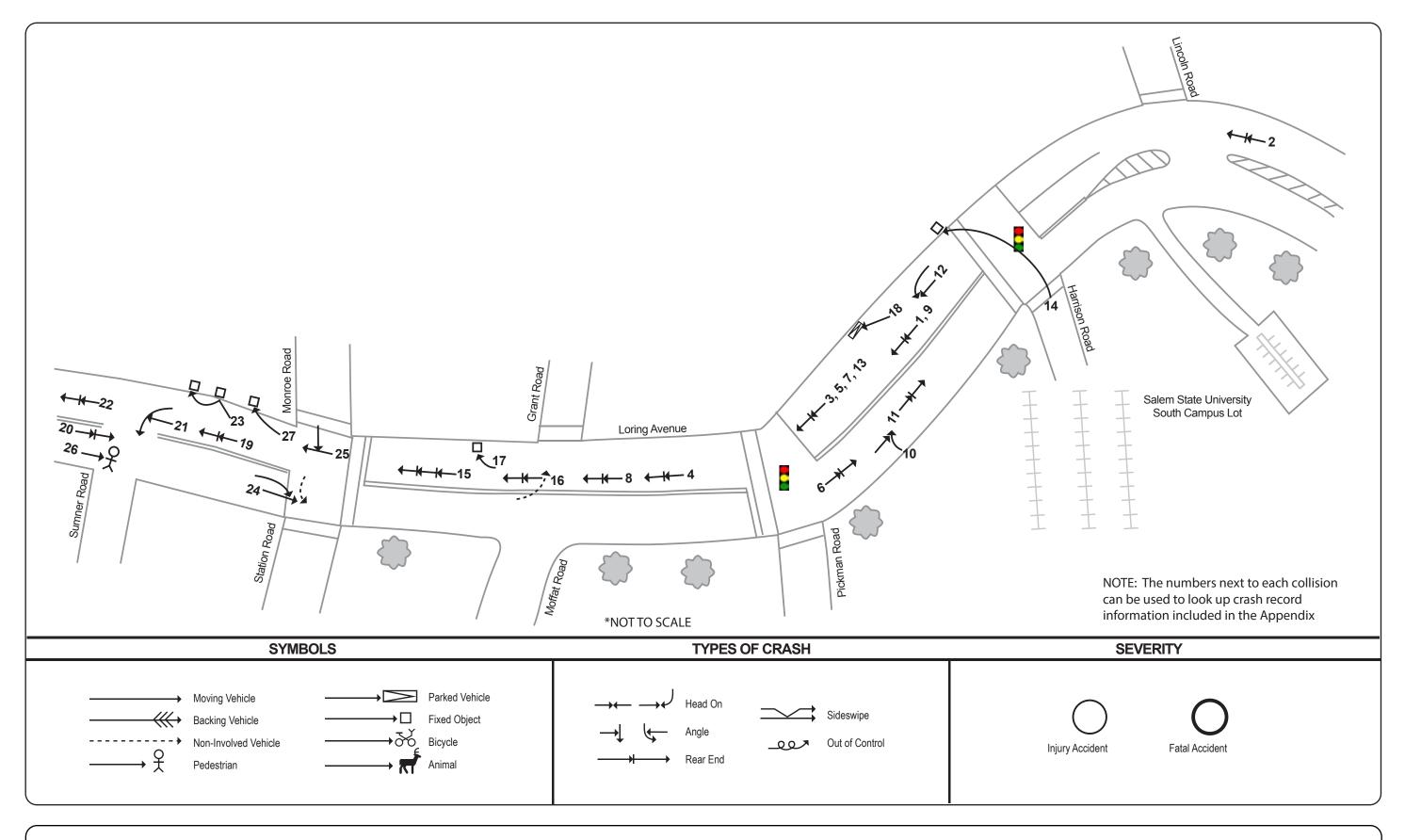


Figure 18
Collision Diagram for Crash Cluster 4







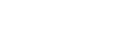


Figure 20
Collision Diagram for Crash Cluster 6

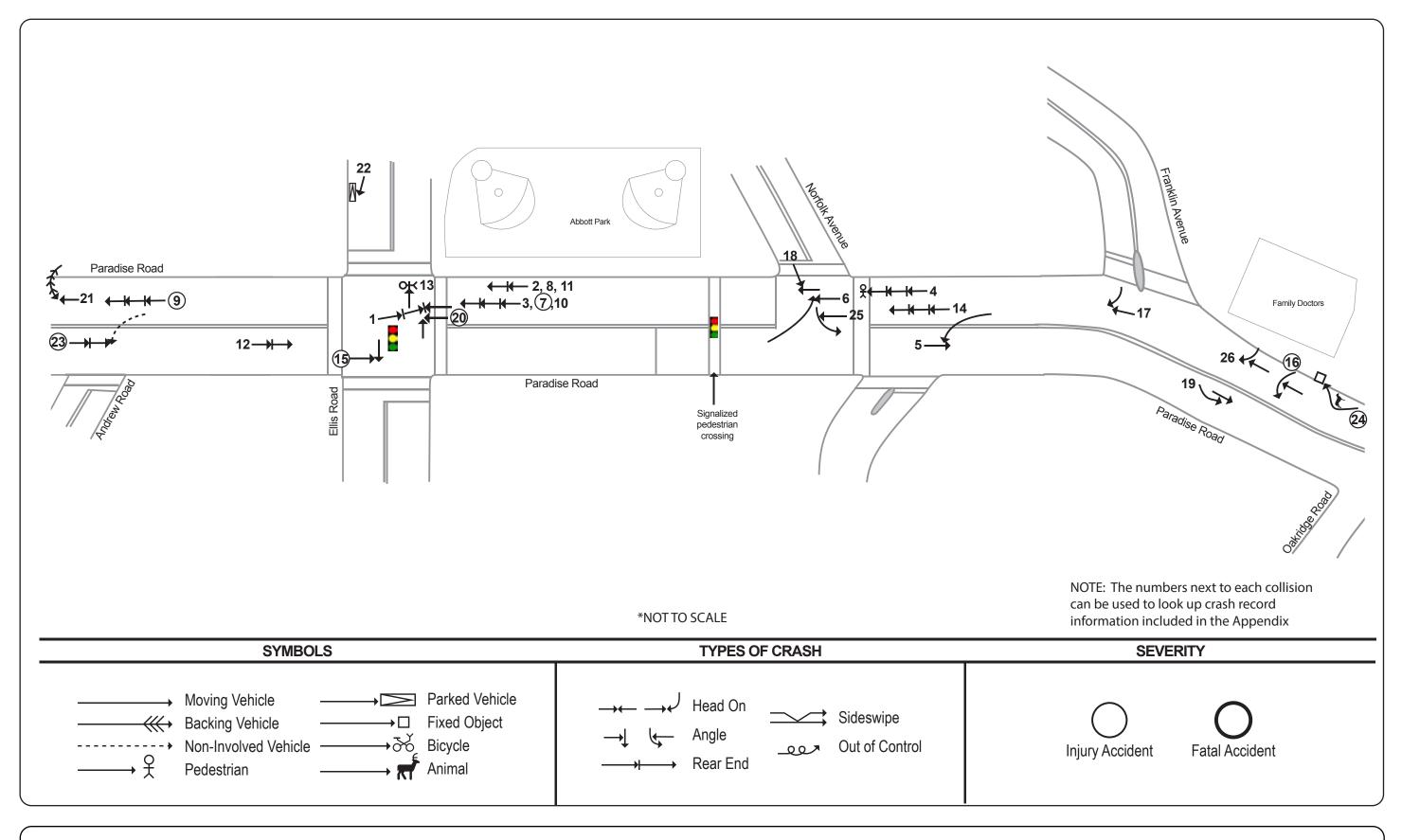
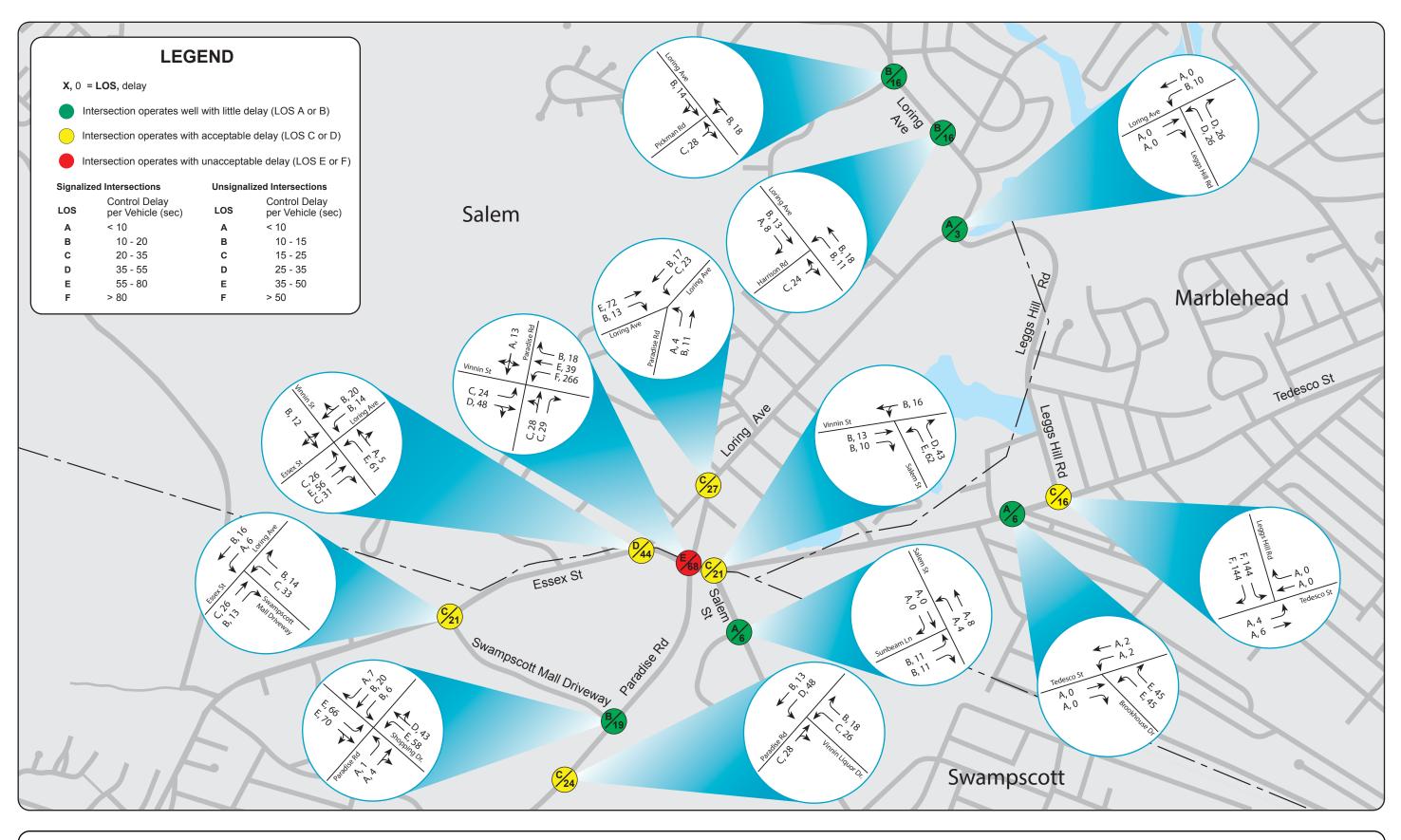
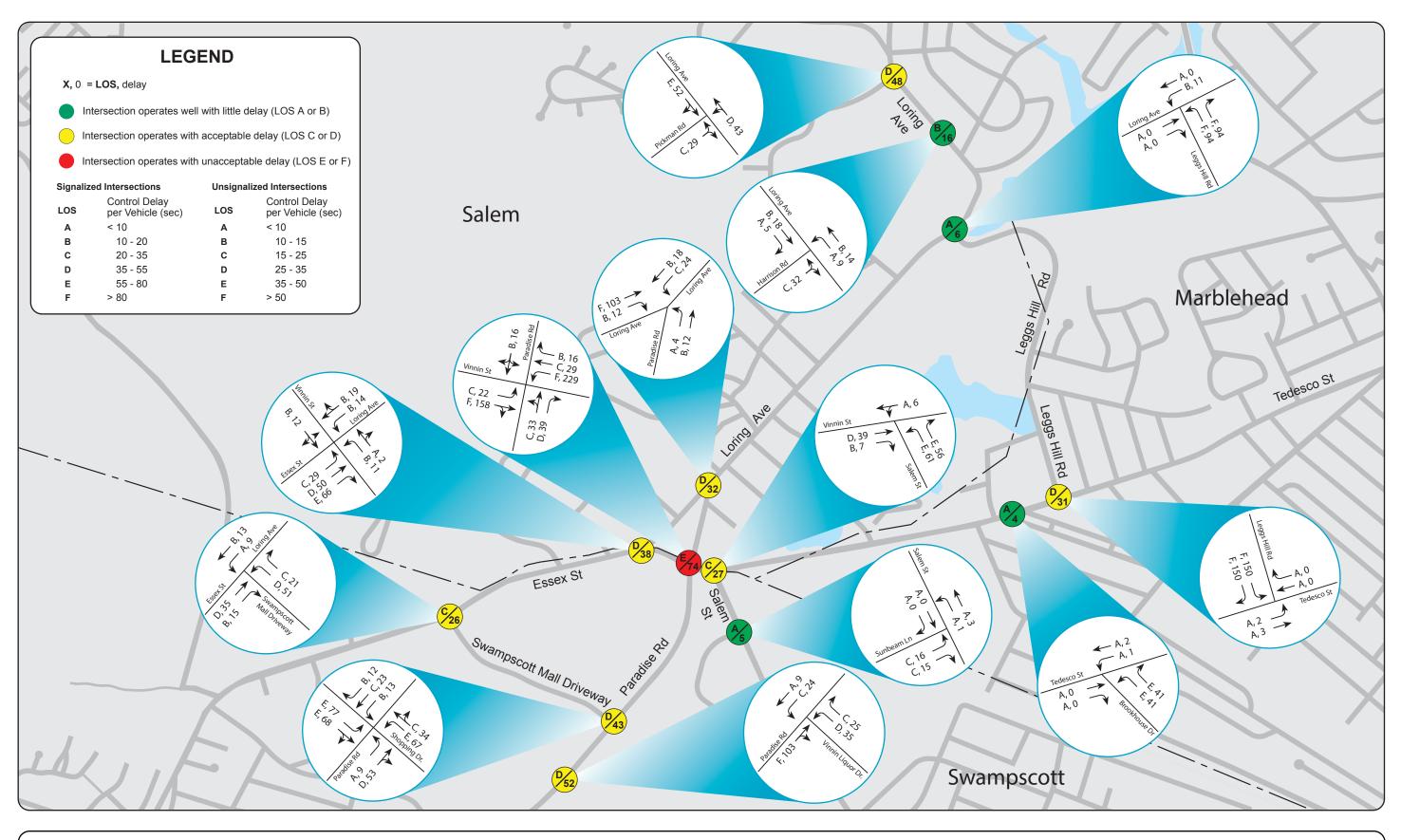




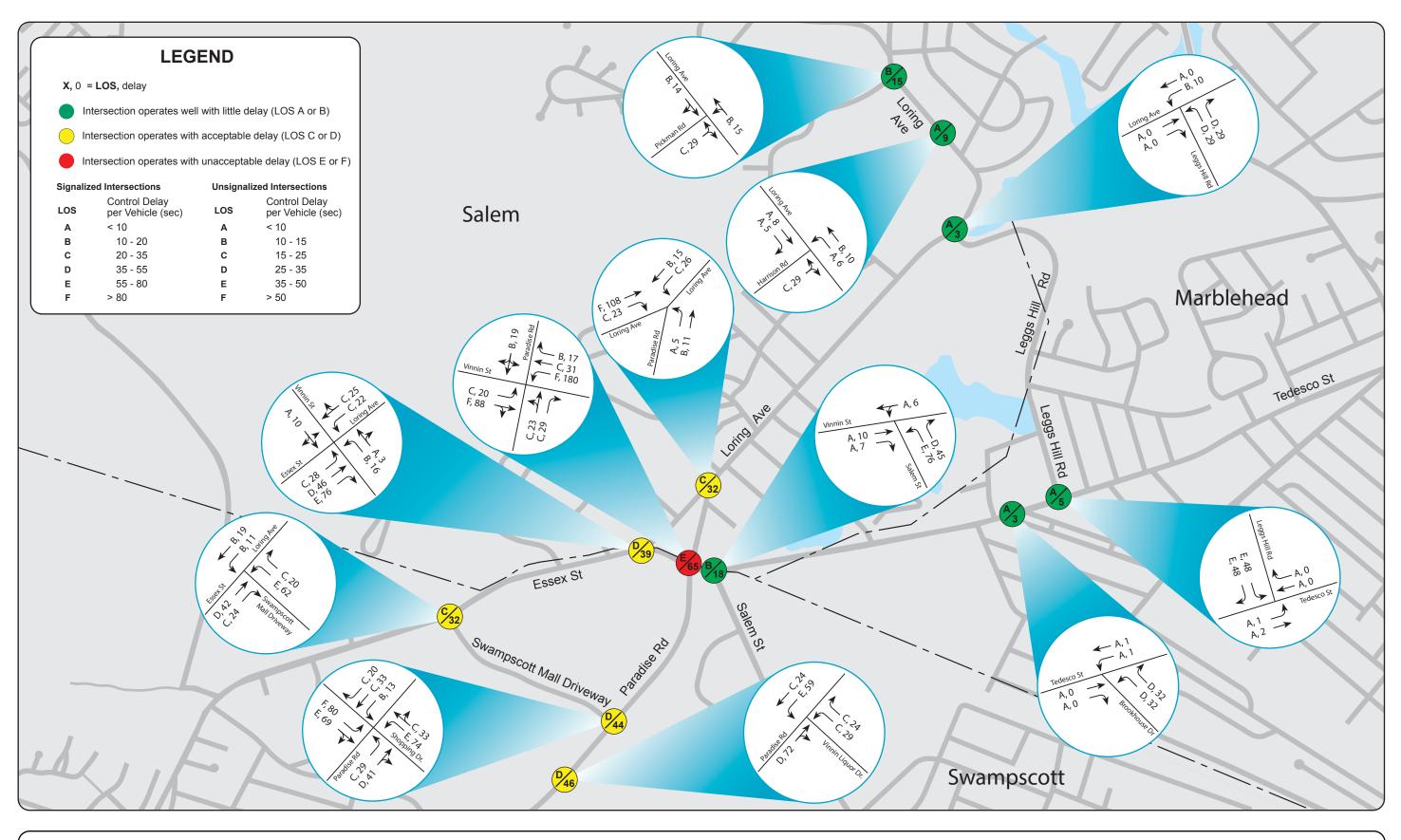
Figure 21
Collision Diagram for Crash Cluster 7



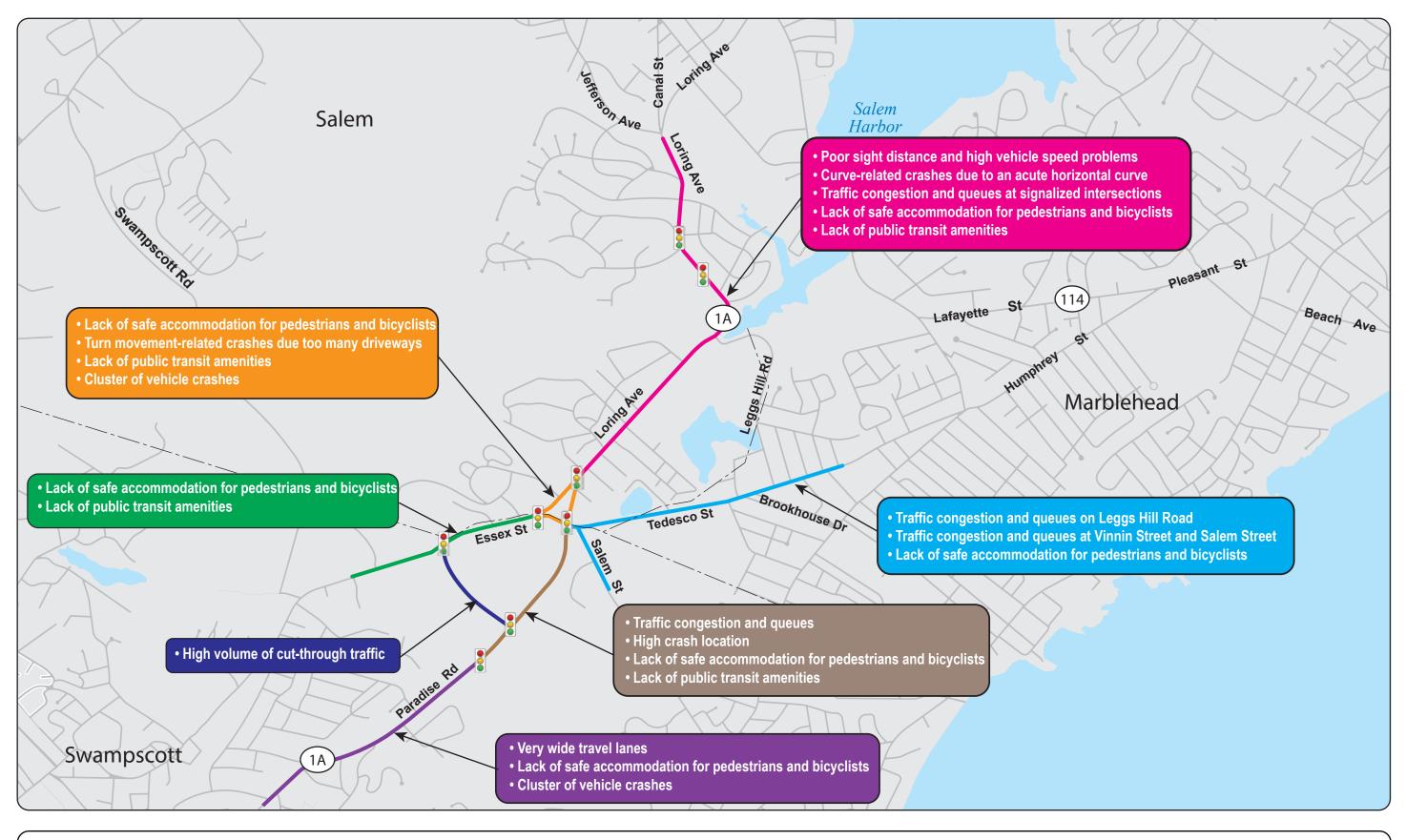


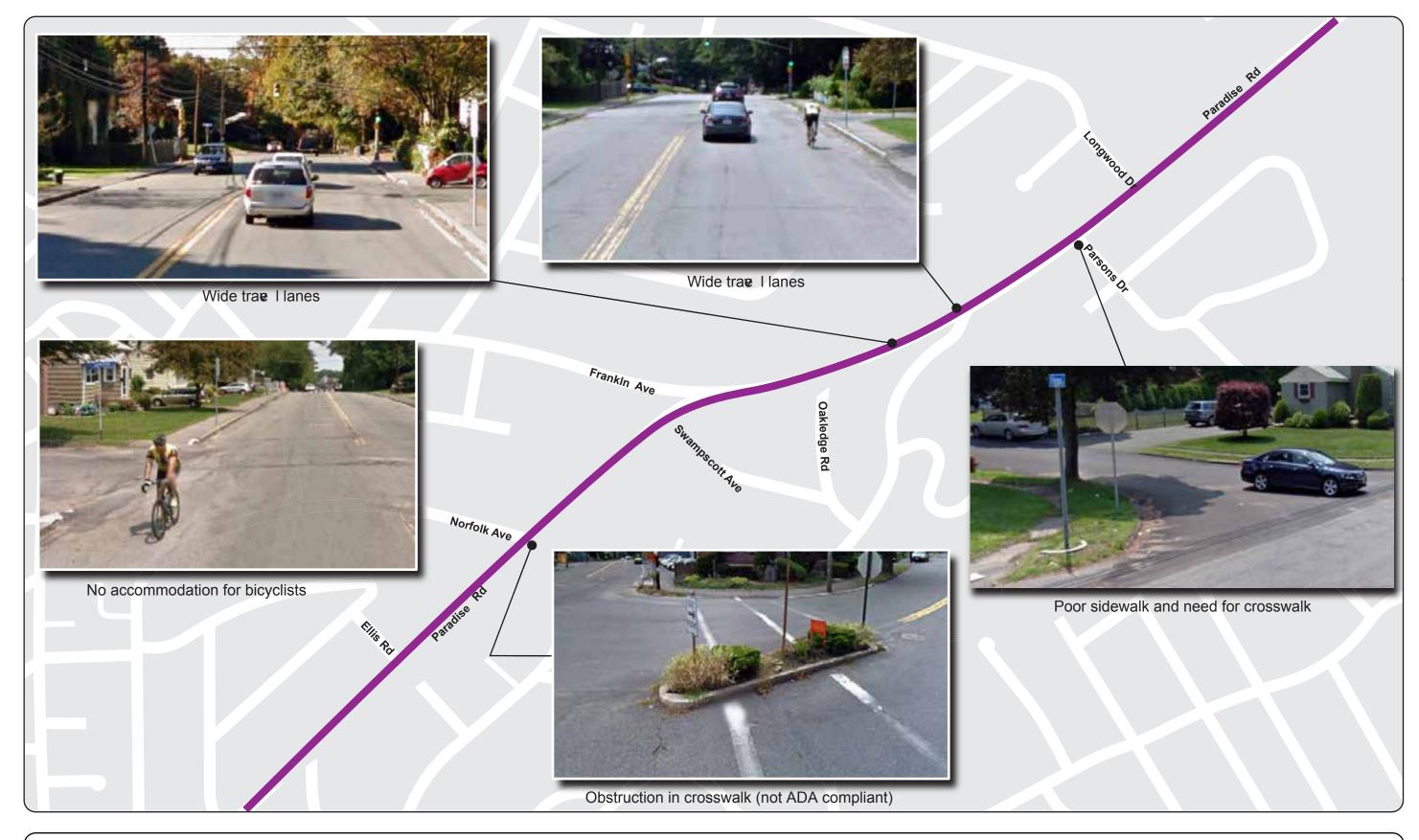






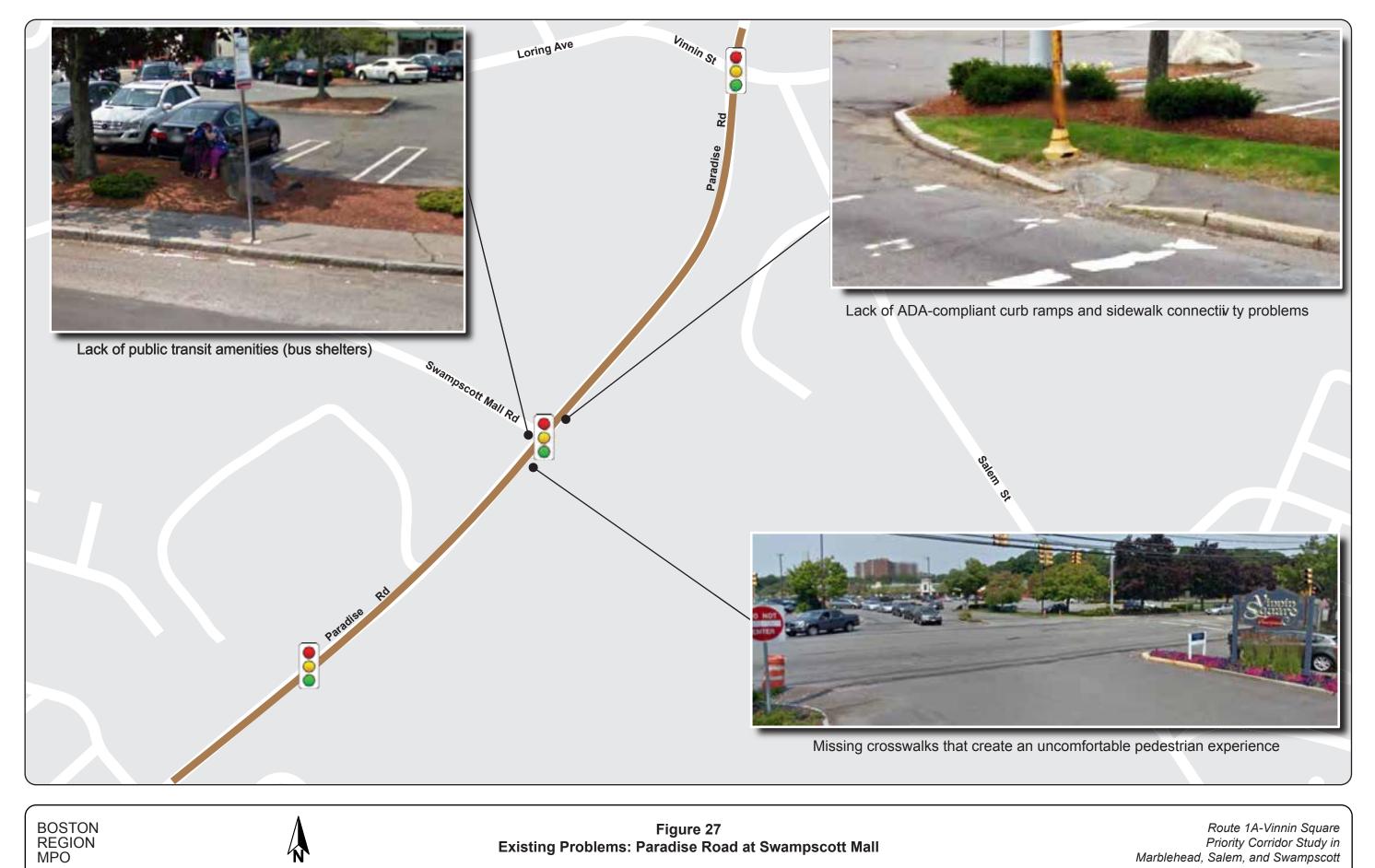




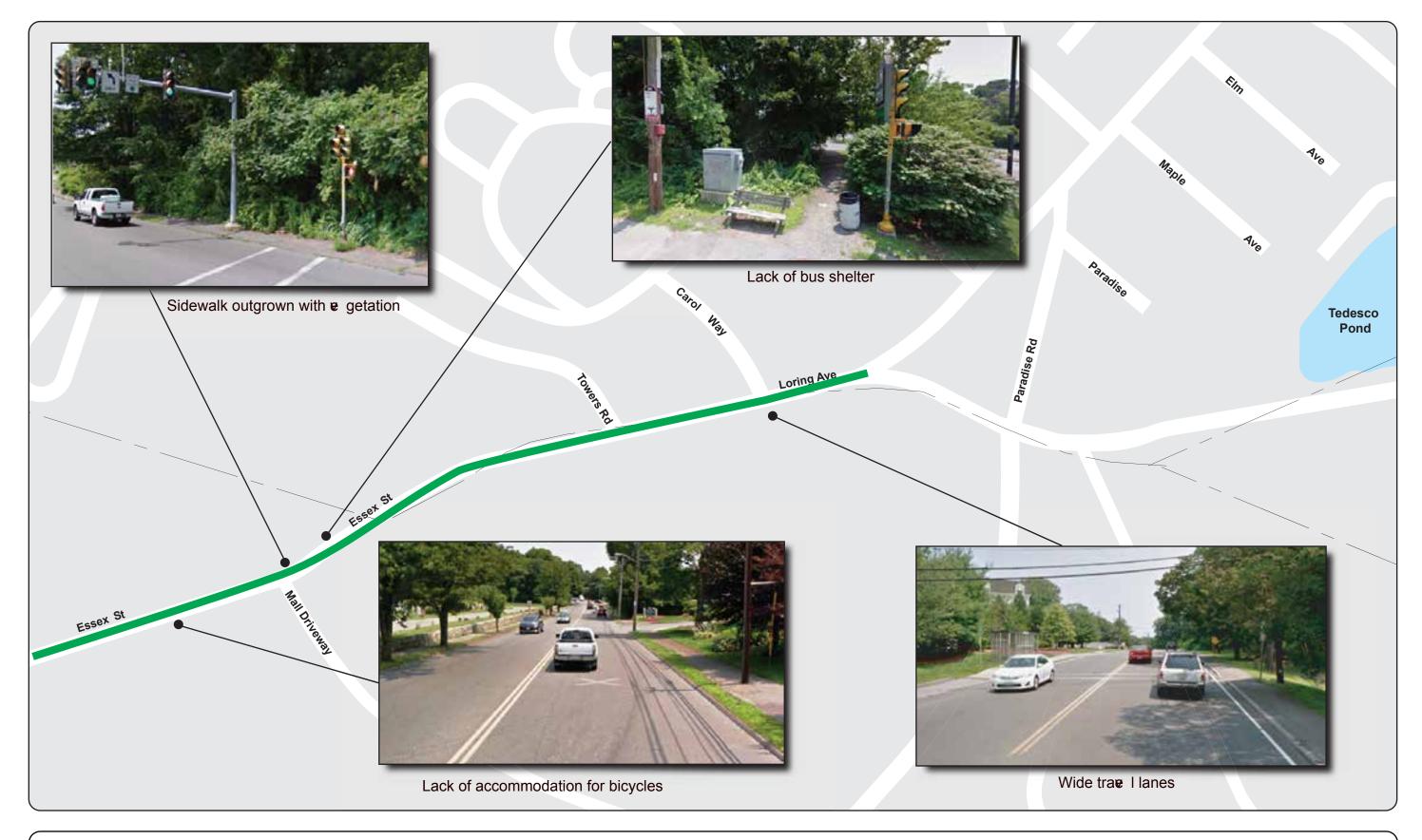




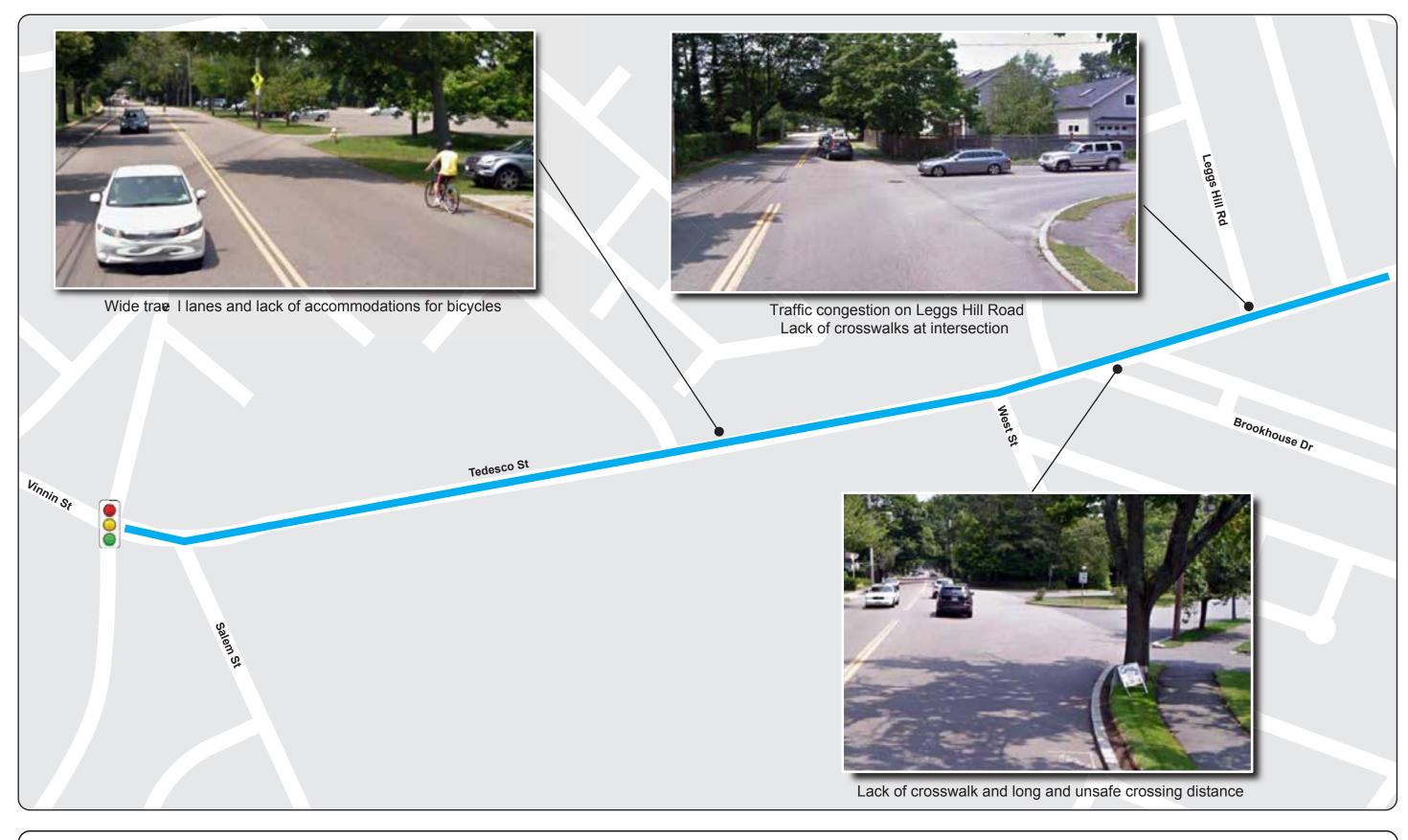






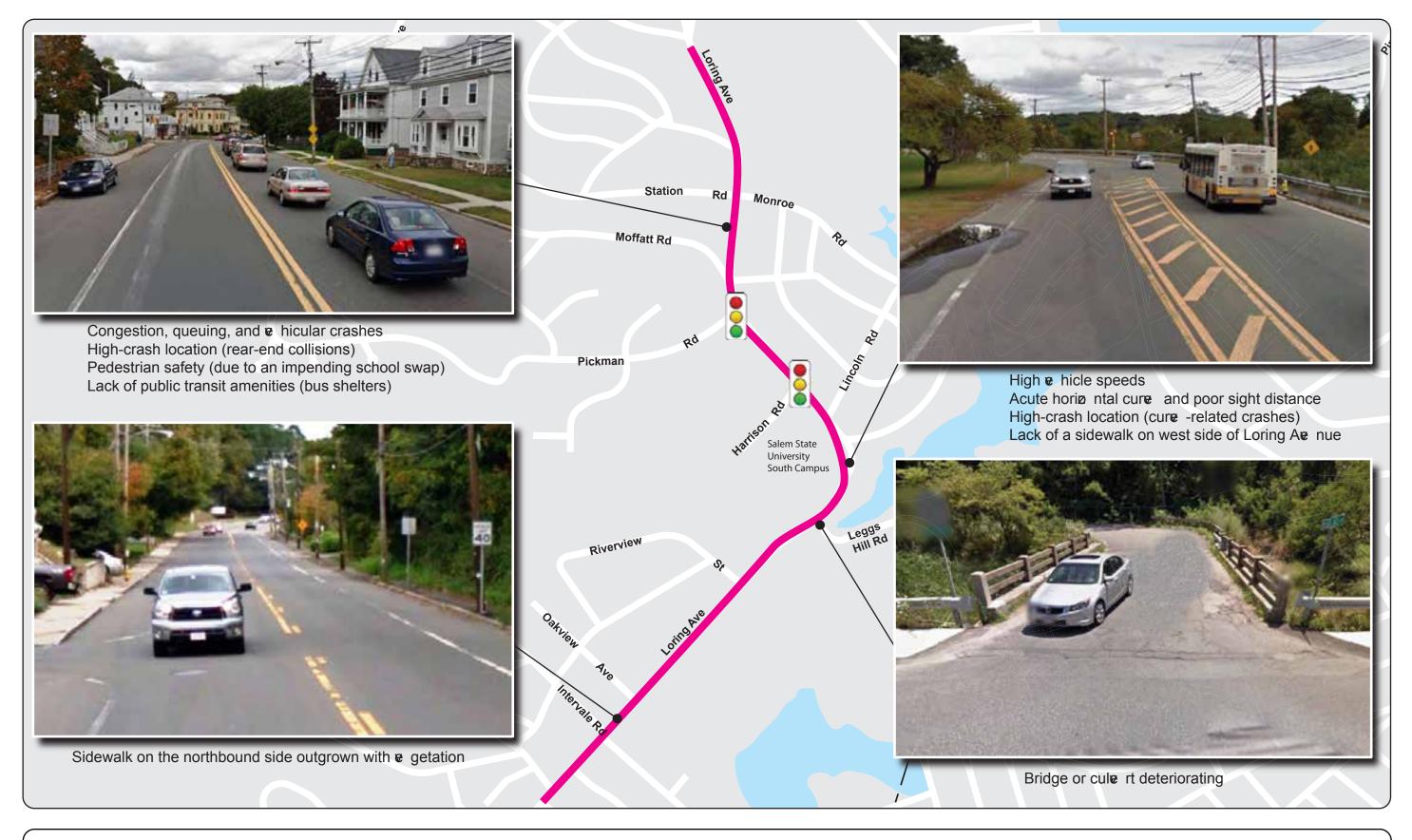












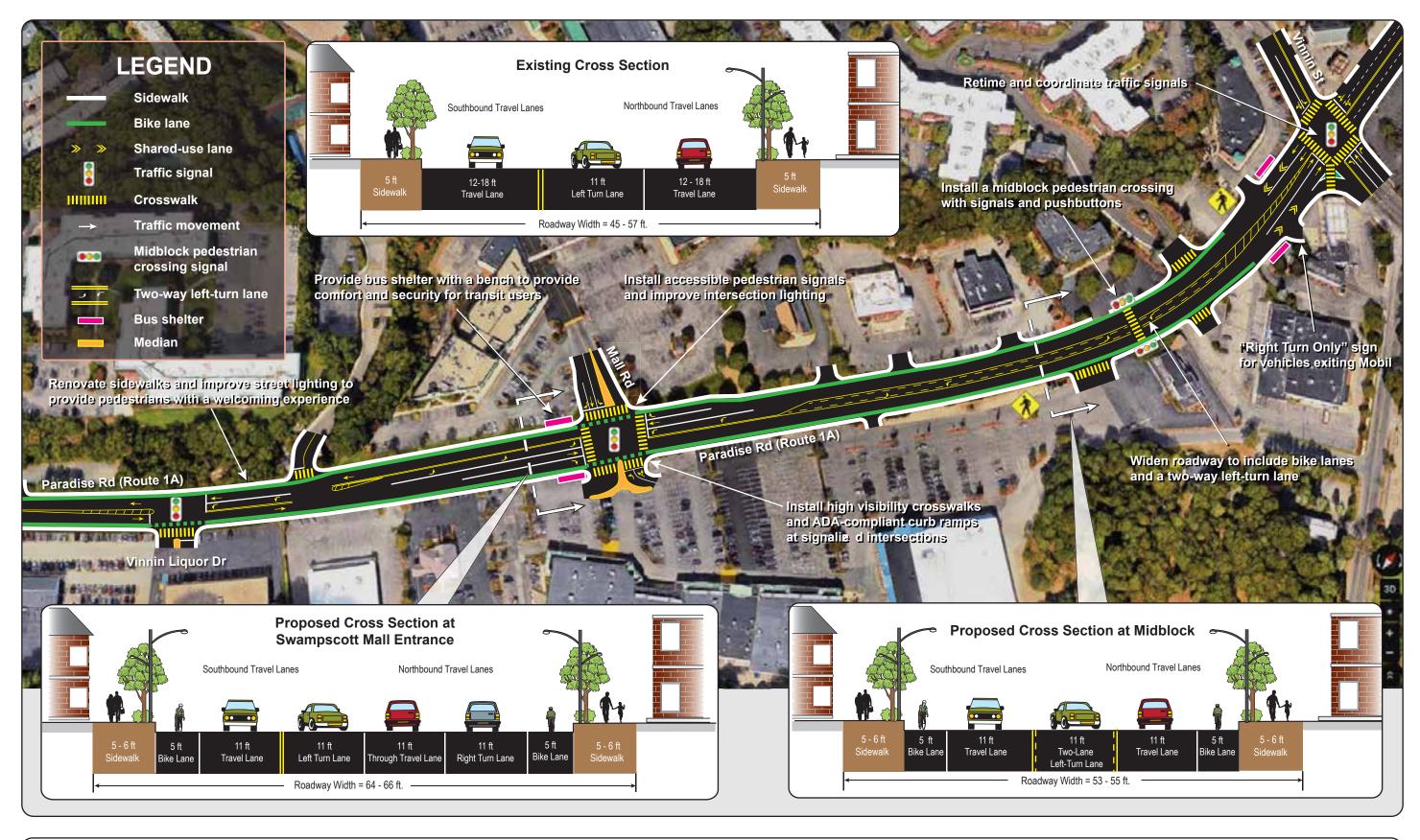


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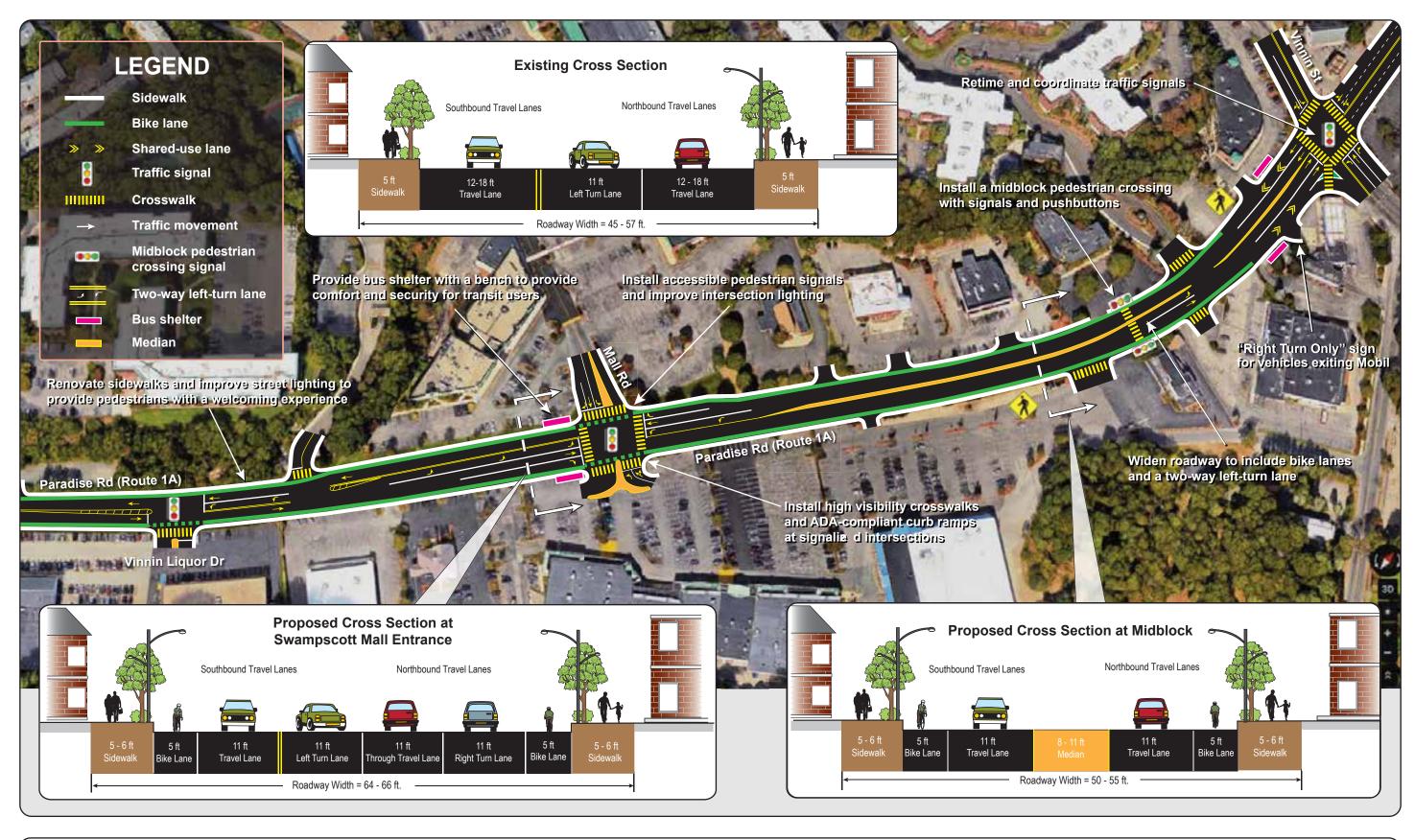






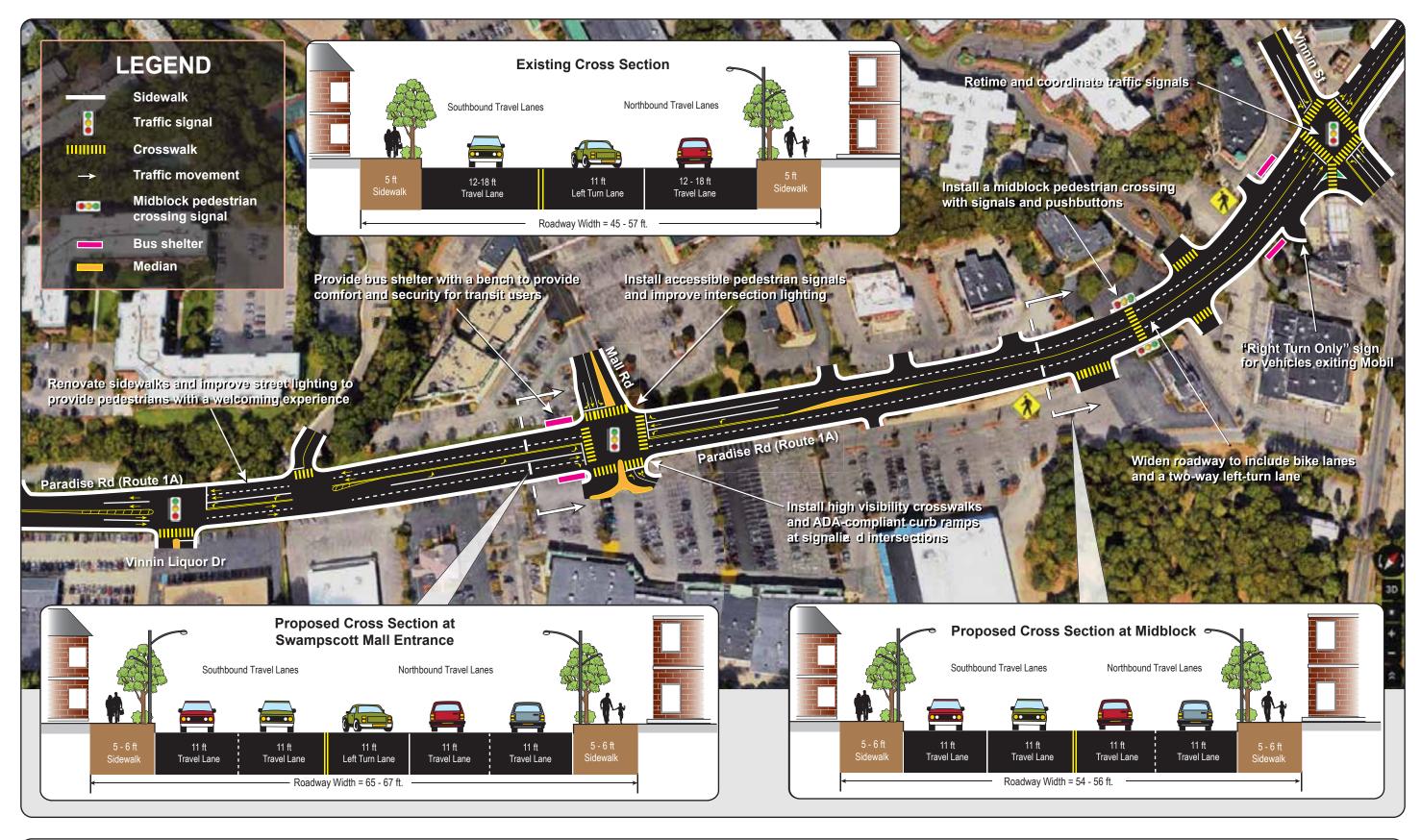






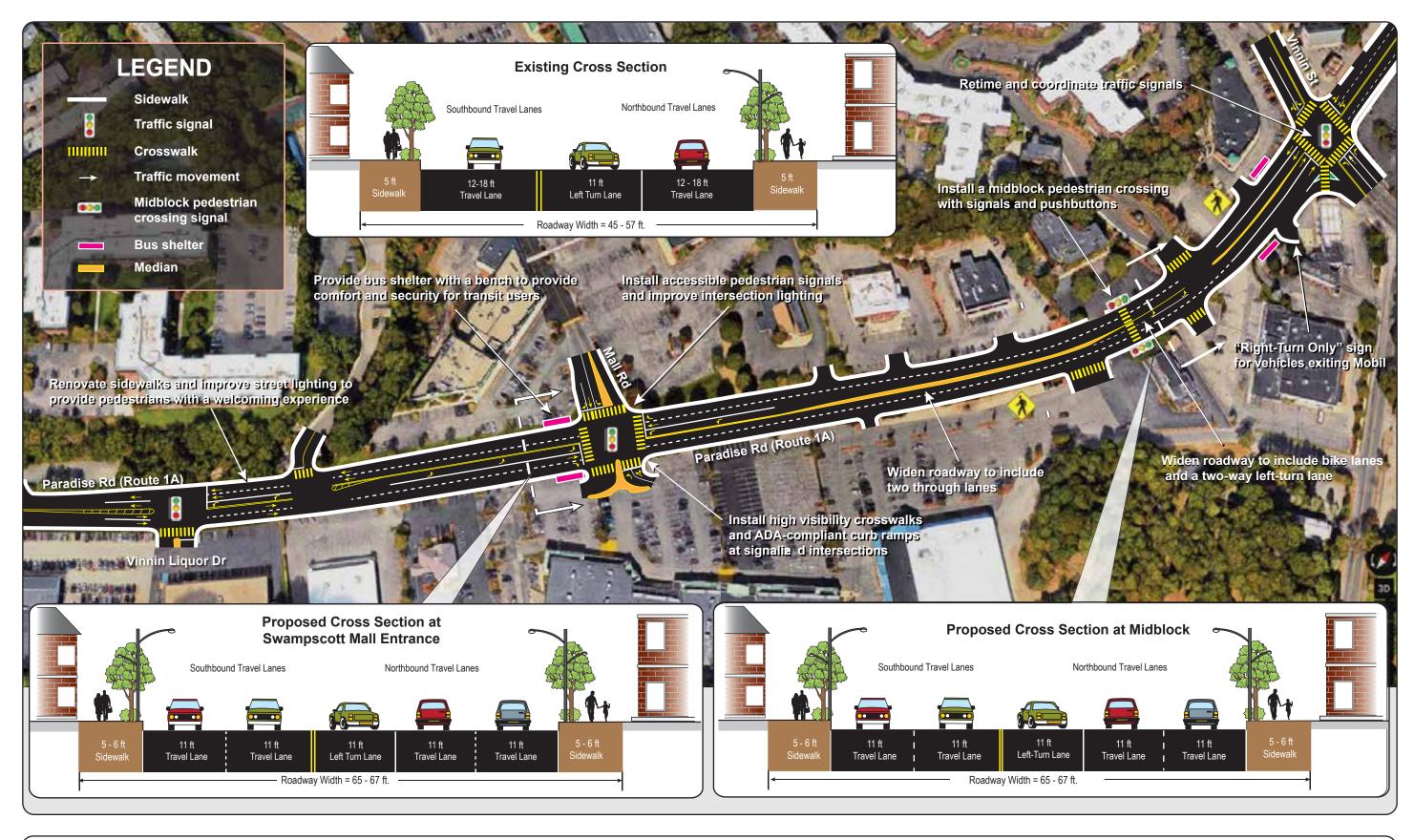






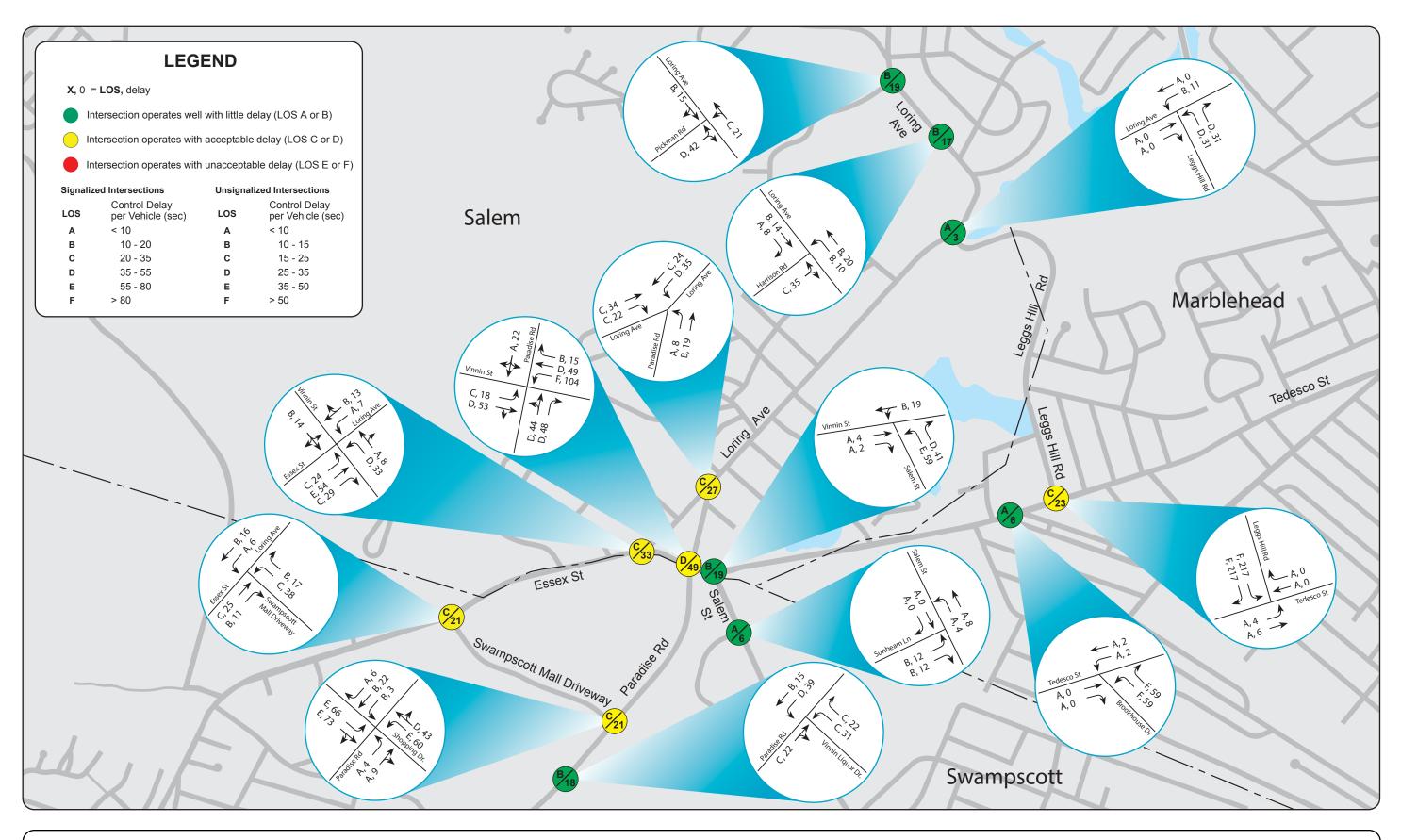




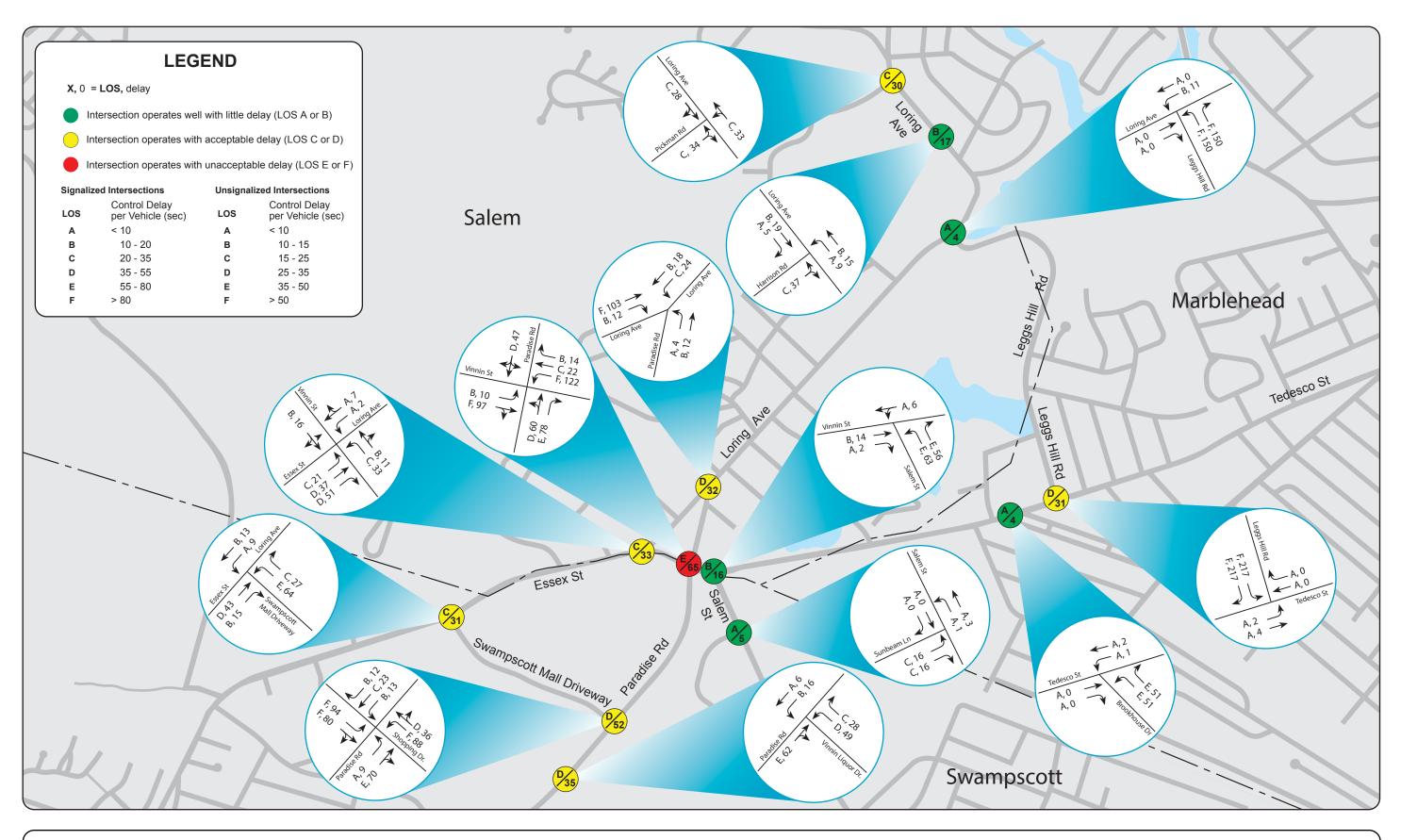




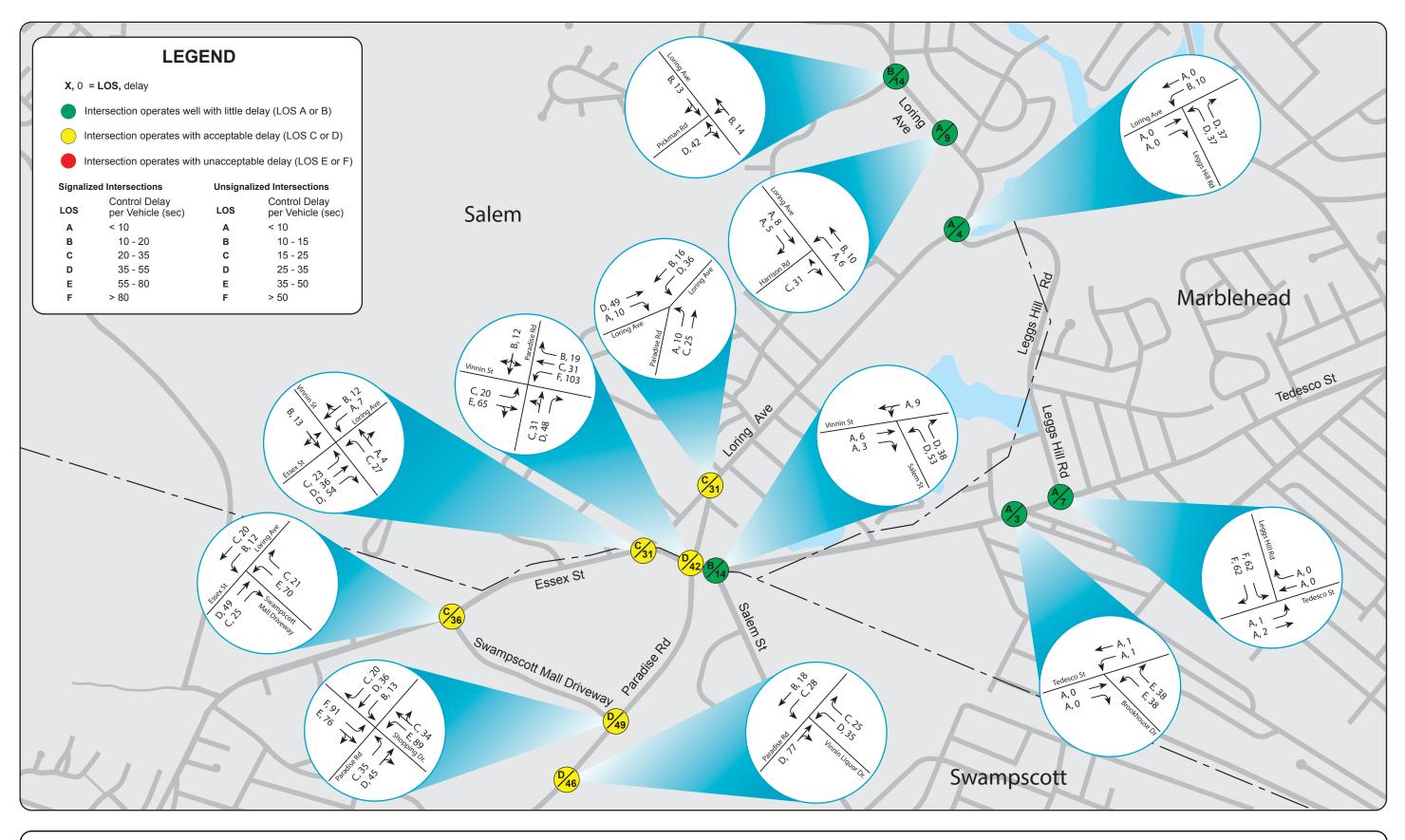




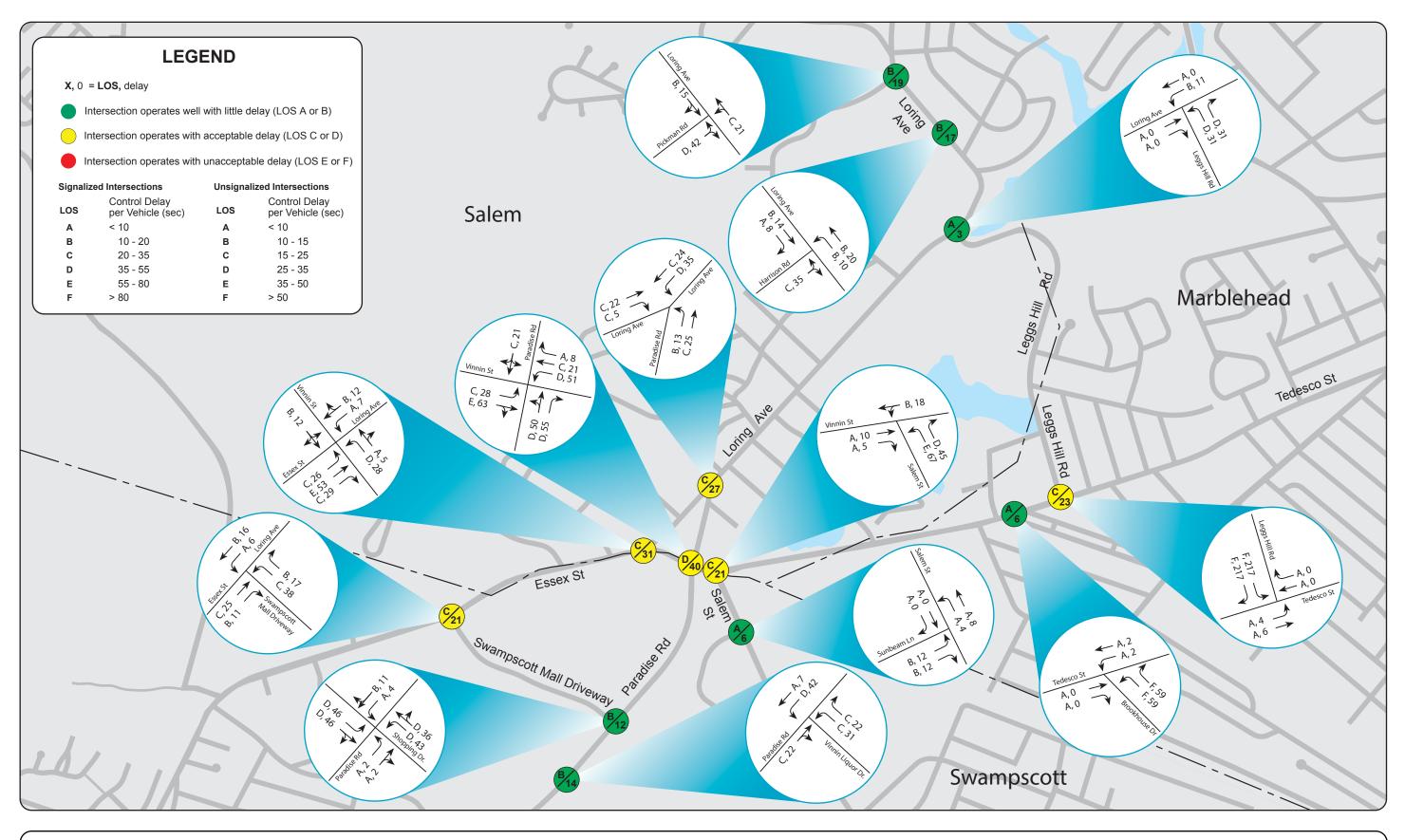






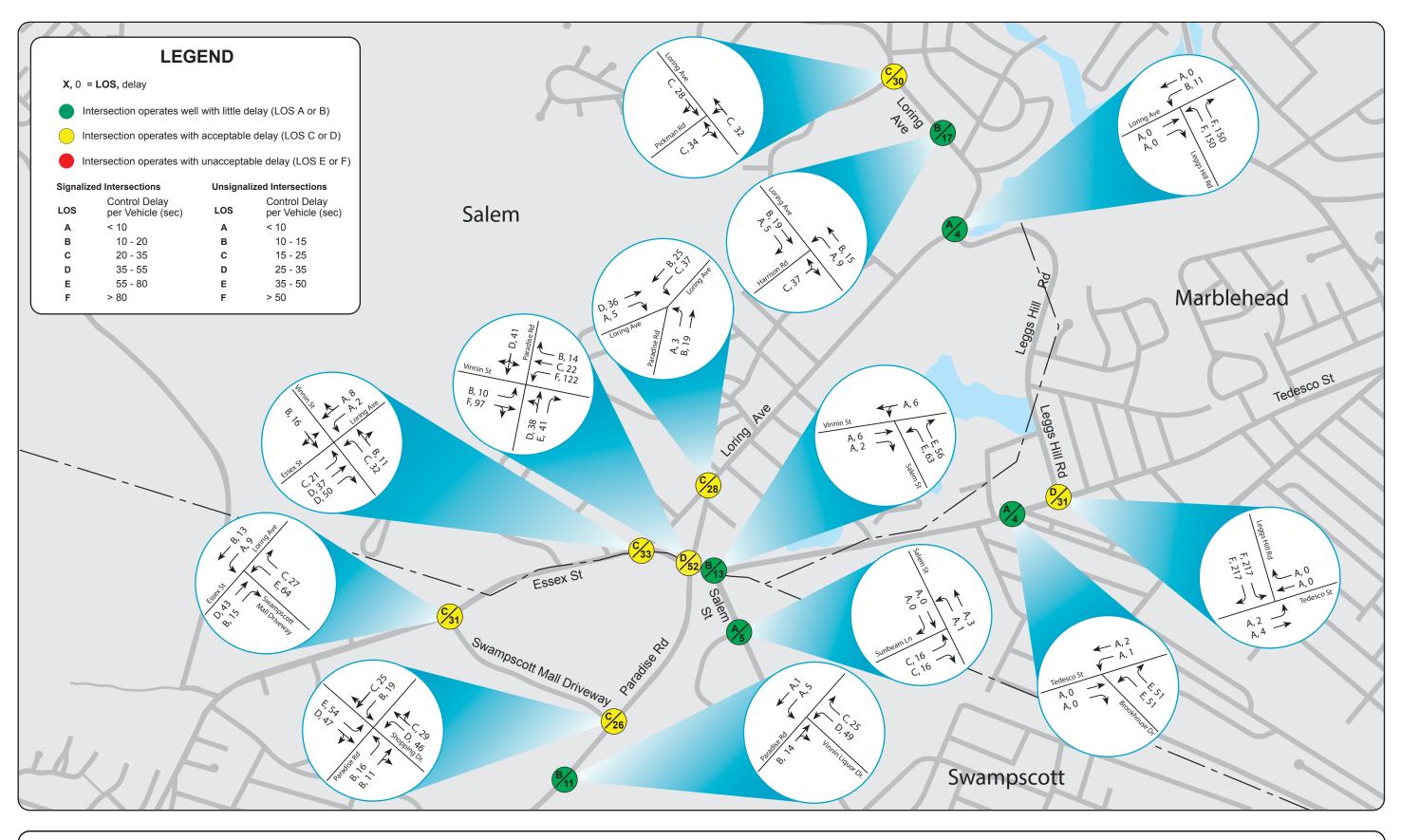




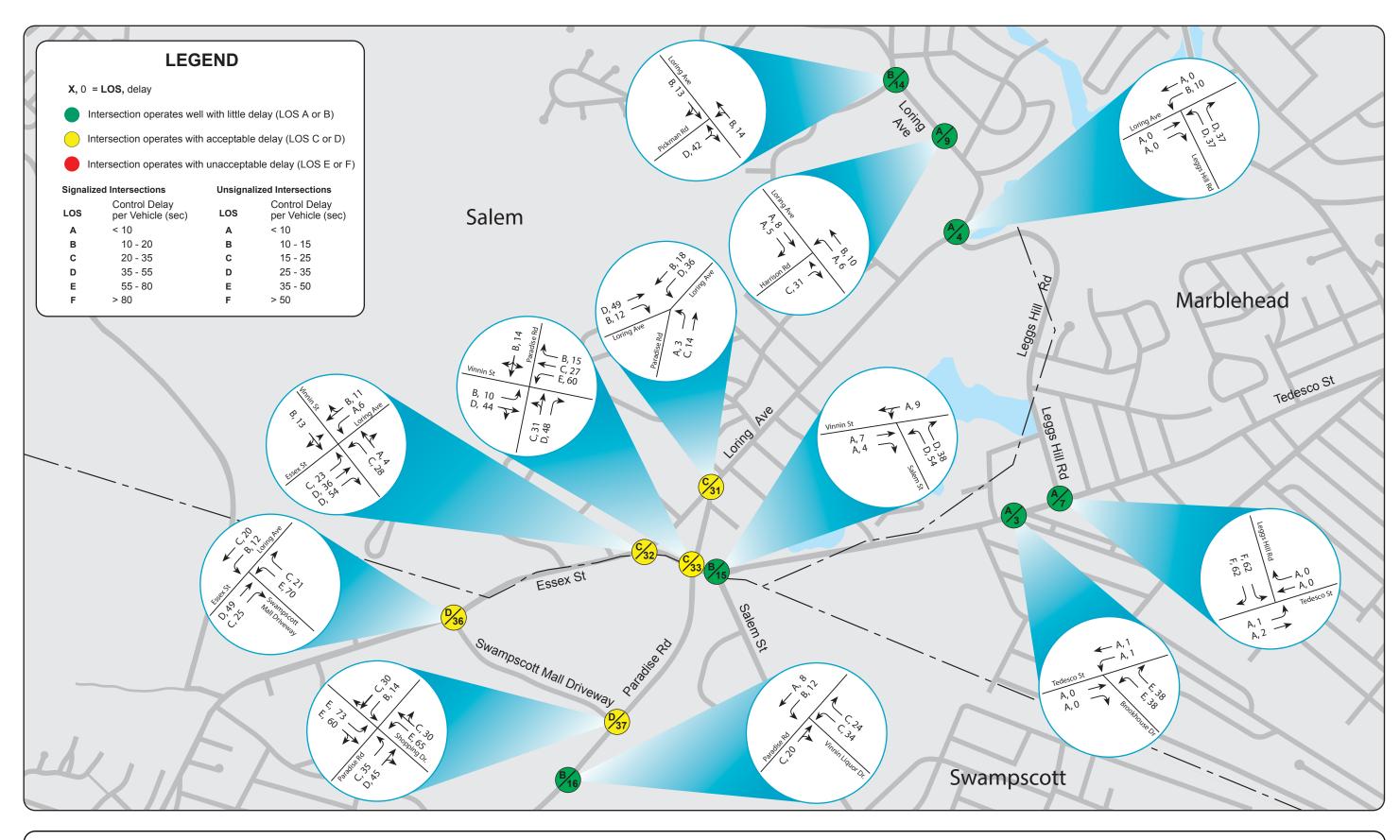




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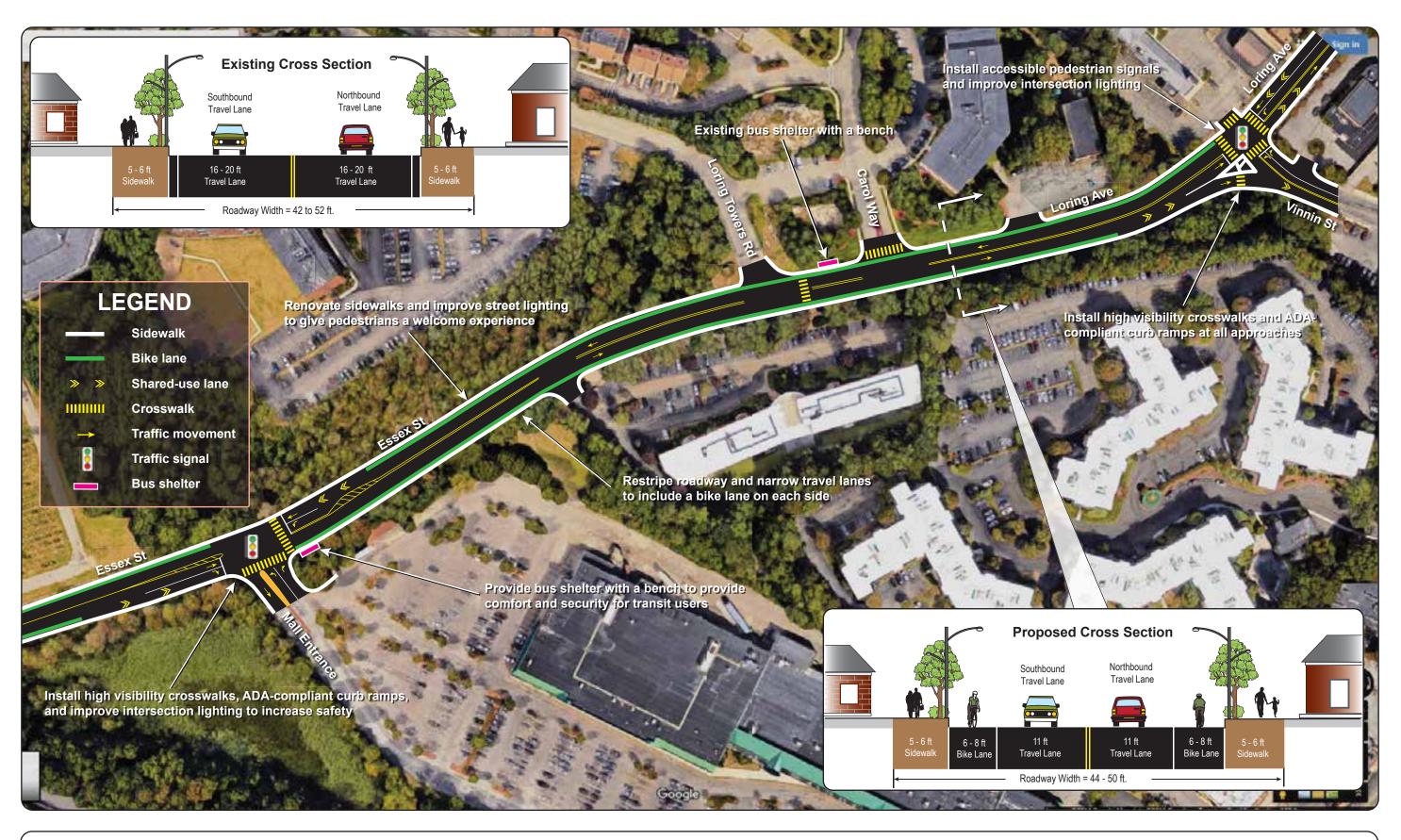








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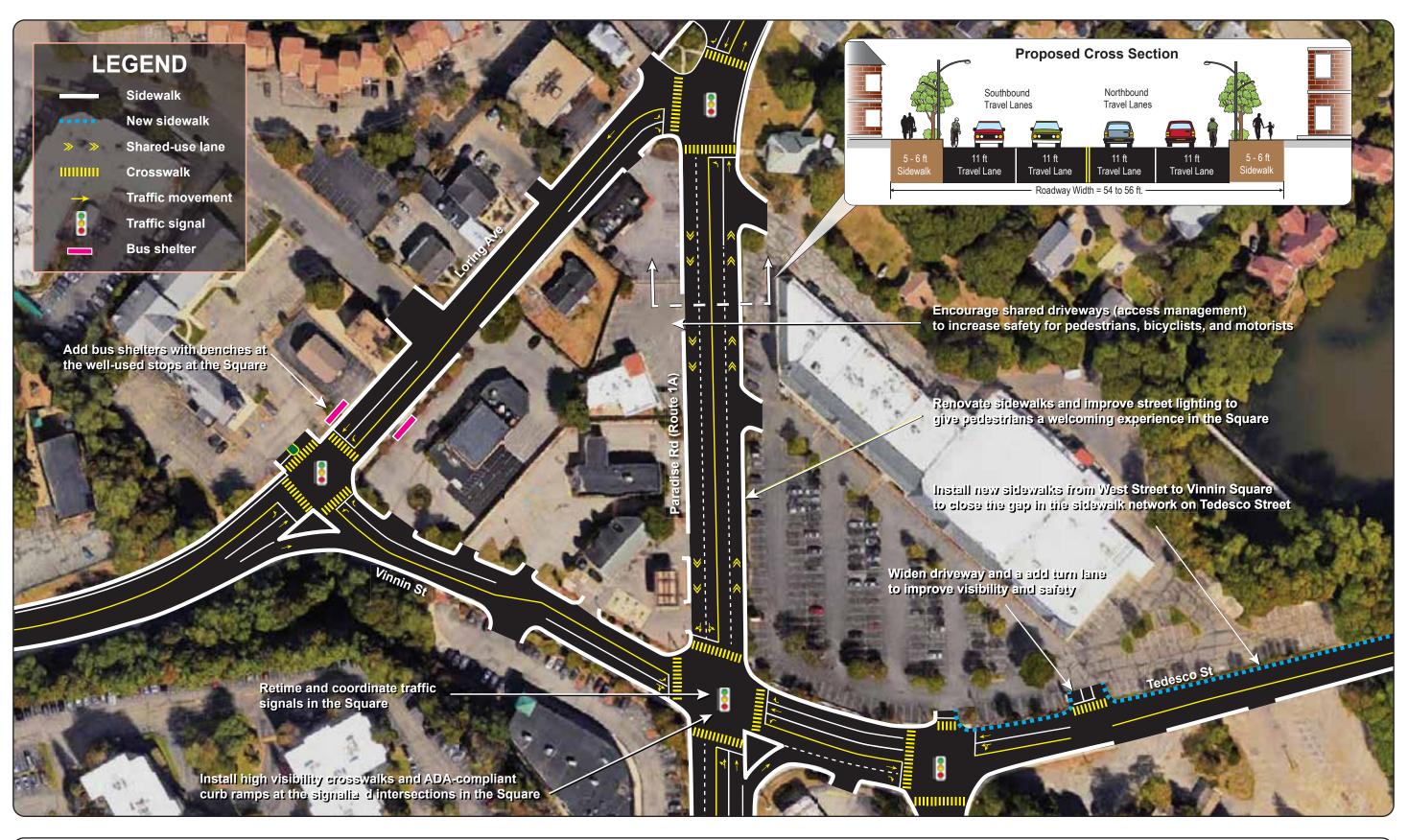




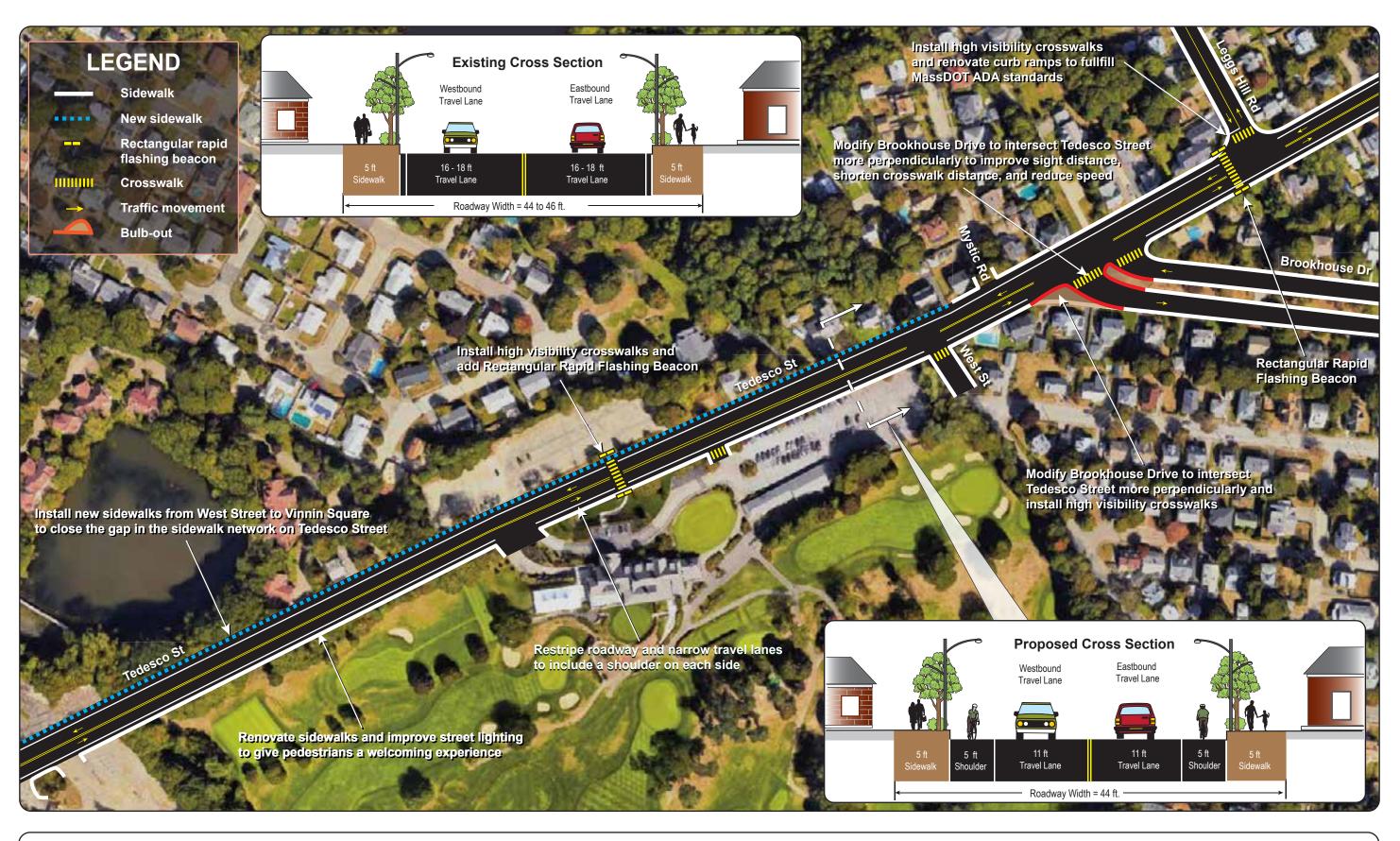


Figure 44
Proposed Improvements:
Vinnin Square
Alternative 1: Shared-Use Lanes





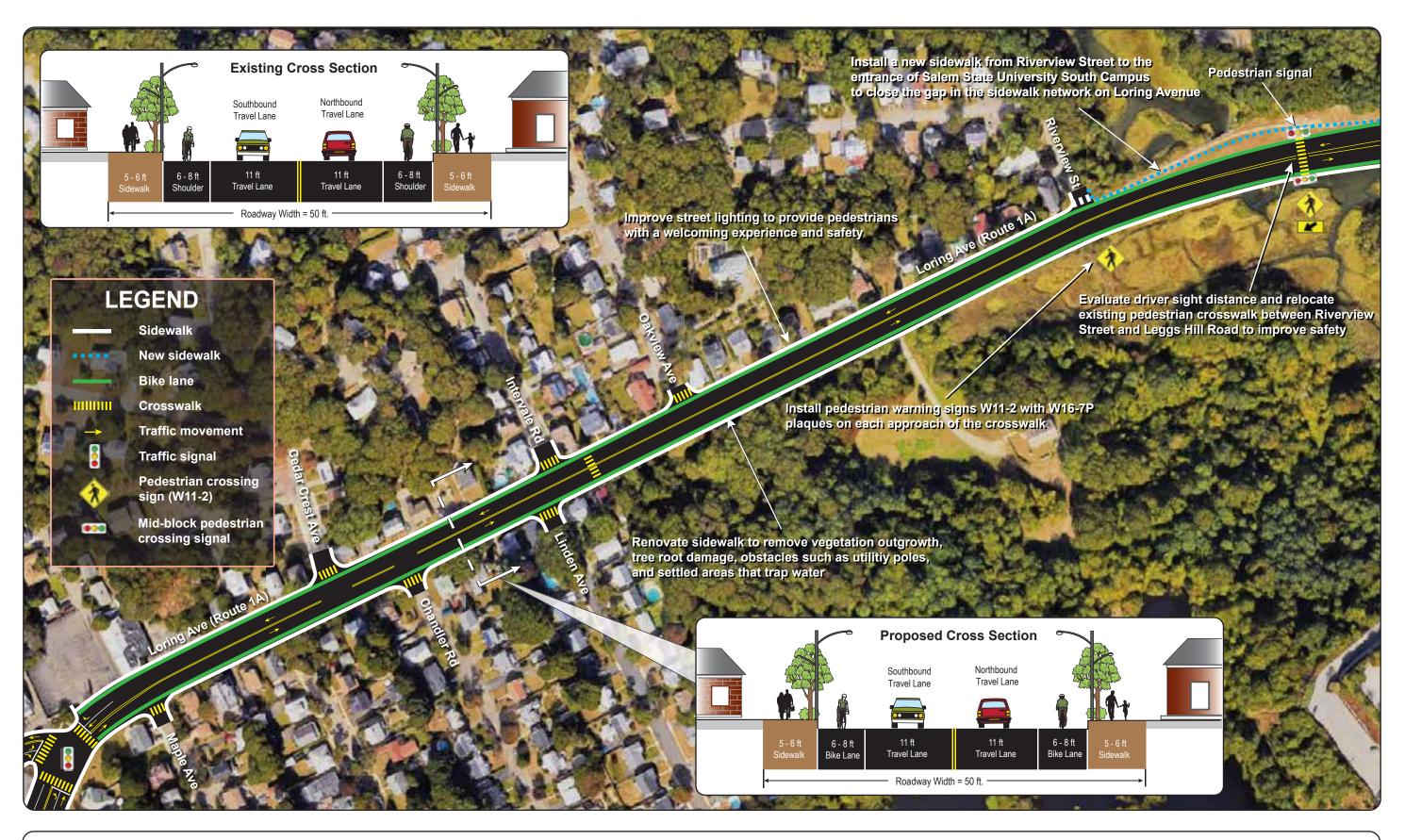
Figure 45
Proposed Improvements:
Vinnin Square
Alternative 2: Bike Lanes



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Examples of ADA-accessible curb ramps







Examples of high visibility crosswalks







Examples of sidewalk designs















Examples of MBTA bus shelters





Examples of pedestrian crossing signals



Route 135 in Natick Two-lane, two-way roadway with shoulders and sidewalks with buffers



Route 109 in Medway
Two-lane, two-way roadway with a two-way left-turn lane and sidewalks



Route 109 in Westwood
Two-lane, two-way roadway with shoulders and sidewalks



Route 114 in Dane rs
Four-lane, two-way roadway with a two-way left-turn lane and sidewalks

Appendices

""APPENDIX A

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BOSTON REGION METROPOLITAN PLANNING ORGANIZATION

Stephanie Pollack, MassDOT Secretary and CEO and MPO Chair Karl H. Quackenbush, Executive Director, MPO Staff

TECHNICAL MEMORANDUM

DATE: March 17, 2016

TO: Boston Region Metropolitan Planning Organization (MPO)

FROM: Seth Asante and Katrina Crocker

RE: Federal Fiscal Year (FFY) 2016 Priority Corridors

for Long-Range Transportation Plan (LRTP)

Needs Assessment: Selection of Study Locations

1 BACKGROUND

This memorandum presents the results of Task 2 of the work program for Priority Corridors for LRTP Needs Assessment: FFY 2016. Task 2 of that work program—to select study locations—includes presenting the results to the MPO for discussion.

The existing needs for all transportation modes in the MPO region were identified as part of the Needs Assessment of the LRTP.² The LRTP Needs Assessment guides the process of deciding which projects to fund in future Transportation Improvement Programs (TIPs). Some of the current mobility requirements of the MPO region listed in the current LRTP Needs Assessment are:

- Maintaining and modernizing roadways with high levels of congestion and safety problems
- Increasing the quantity and quality of walking and bicycling
- Improving the efficiency of transit service and adherence to schedules

Based on previous and ongoing transportation-planning work—including the MPO's Congestion Management Process (CMP) and MPO planning studies—MPO staff identified several priority arterial roadway segments that require maintenance, modernization, safety improvements, and mobility improvements, and listed them in the LRTP Needs Assessment. To address problems of some

¹ Karl H. Quackenbush, CTPS Executive Director, memorandum of a work program to the Boston Region Metropolitan Organization, "Priority Corridors for LRTP Needs Assessment: FFY 2016," October 15, 2015.

² Region Metropolitan Planning Organization, *Charting Progress to 2040: The New Long-Range Transportation Plan of the Boston Region Metropolitan Planning Organization*, endorsed by the Boston Region MPO on July 30, 2015.

of these arterial segments, a study was included in the federal fiscal year (FFY) 2016 Unified Planning Work Program (UPWP).³

By focusing on arterial segments rather than intersections, planners can evaluate multimodal transportation needs comprehensively (with the goal of creating "complete streets"). A holistic approach to analyzing problems and forming recommendations ensures that the needs of all public transportation users—including pedestrians, bicyclists, and motorists—are considered. Ultimately, this will result in roadways where it is safe to cross the street and walk or cycle to shops, schools, train stations, and recreational facilities, and where buses can run on time. Typically, the recommended improvements are within a roadway's right-of-way. They take into account the needs of abutters and users, and the interests and support of stakeholders.

2 SELECTION PROCEDURE

The study selection process consisted of three steps. First, MPO staff assembled data about the arterial segments that are identified in the current LRTP and used them to screen the roadway segments. Next, MPO staff examined the arterial segments more closely by applying specific criteria. Finally, staff scored each arterial segment and assigned a priority of low, medium, or high to each segment.

2.1 Gathering Data

MPO staff identified 54 arterial segments in 39 municipalities in the MPO region. The assembled data are:

- The Massachusetts Department of Transportation (MassDOT) 2014 Road Inventory File and 2009–13 crash database – used to assemble the following information for each arterial segment in each municipality: roadway jurisdiction, National Highway System (NHS) status, average daily traffic (ADT), high-crash locations, and crash rates
- MPO Congestion Management Process data on arterial congestion used to determine average travel speeds, travel time index (travel time in the peak period divided by travel time at free-flow conditions), and speed index (average travel speed divided by the speed limit) on each arterial segment
- MPO data on gaps in the bike network and MassDOT bike facilities used to identify bicycle needs, including connectivity, and accommodations

³ Boston Region Metropolitan Planning Organization, Unified Planning Work Program, Federal Fiscal Year 2016, endorsed by the Boston Region Metropolitan Planning Organization on July 30, 2015.

- Data on MBTA bus service performance and passenger load used to determine the percentage of bus trips that do not adhere to the schedule (in other words, that provide late service) or do not adhere to passenger load standards (resulting in crowding)
- Data on MBTA bus routes, subway lines, and commuter rail lines used to identify which segments serve MBTA buses or stations
- Data on the Boston Region MPO's Environmental Justice (EJ) transportation analysis zones – used to identify EJ areas
- Data selected from MassDOT's project-information database; the MPO's FFY 2016–20 TIP projects; MPO planning studies and other studies; and municipal websites – used to obtain data on projects, studies, and TIP projects that are planned or programmed for each arterial segment

Table 1 (at the end of this memorandum) presents, for each arterial segment, the data and information gathered for this study, including the municipality, MAPC subregion, jurisdiction, MassDOT district office, crash rate per million vehiclemiles traveled, number of top-200 high-crash locations, number of crash clusters that are eligible for Highway Safety Improvement Program (HSIP) funding, travel time index, transit service performance, whether the segment is located in, or within a half mile of, an EJ transportation analysis zone, and a list of relevant studies or projects. It also includes the score and priority rating that were determined by applying the selection criteria. The processes for scoring and assigning priority ratings to segments are described below.

2.2 Applying Criteria

MPO staff examined the arterial segments more closely by applying the following six criteria:

- Safety Conditions, 0–4 points (each of the four criteria is worth 1 point)
 - Location has a higher-than-average crash rate for its functional class
 - Location contains an HSIP-eligible crash cluster
 - Location is on the list of the Massachusetts top-200 high-crash locations
 - Location has a significant number of pedestrian and bicycle crashes per year (two or more per mile) or contains one or more HSIP-eligible bike-pedestrian clusters

- Congested Conditions, 0–2 points (each of the two criteria is worth 1 point)
 - Travel time index is at least 1.3
 - Travel time index is at least 2.0
- Multimodal Significance, 0–3 points (each of the three criteria is worth 1 point)
 - Location currently supports transit, bicycle, or pedestrian activities
 - Location needs to have improved transit, bicycle, or pedestrian facilities
 - Location has a high volume of truck traffic serving regional commerce
- Regional Significance, 0–4 points (each of the four criteria is worth 1 point)
 - Location is in the National Highway System
 - Location carries a significant portion of regional traffic (ADT is greater than 20,000)
 - Location lies within 0.5 miles of an EJ transportation analysis zone
 - Location is essential for the region's economic, cultural, or recreational development
- Regional equity, 0–2 points (each of the two criteria is worth 1 point)
 - Location is in an MPO subregion for which there has not been a priority-corridor study
 - Location is in an MPO subregion for which there has been a priority-corridor study in the previous three years.
- Implementation Potential, 0–3 points (each of the three criteria is worth 1 point)
 - Location is proposed or endorsed by its roadway administrative agency
 - Location is proposed or endorsed by its MPO subregion and is a priority for that subregion
 - Location has strong support for improvements from other stakeholders

2.3 Scoring and Rating

Arterial segments that have a total score of 10 or fewer points were rated low priority; those with a score of 11 to 12 points were rated medium priority; and those with a total score 13 or more points were rated high priority. Eleven arterial segments were given a high-priority rating by MPO staff based on safety, operations, multimodal and regional significance, regional equity, and support from agencies and municipalities. The availability of funding determined the number of segments selected.

The high-priority segments were then examined more closely, and arterials that had projects meeting any of the following criteria were excluded from further consideration for this cycle of the priority-corridors study: recently completed, in construction, in design, under study, or programmed in the TIP. Figure 1 shows the general locations of previous priority-corridor studies, and also shows that the arterial segment selected for study is located in a subregion in which there has never been a priority-corridor study. Based on this evaluation, the segment described below was selected for study.

3 ARTERIAL SEGMENT SELECTED FOR STUDY: ROUTE 1A AT VINNIN SQUARE AREA IN SWAMPSCOTT, SALEM, AND MARBLEHEAD

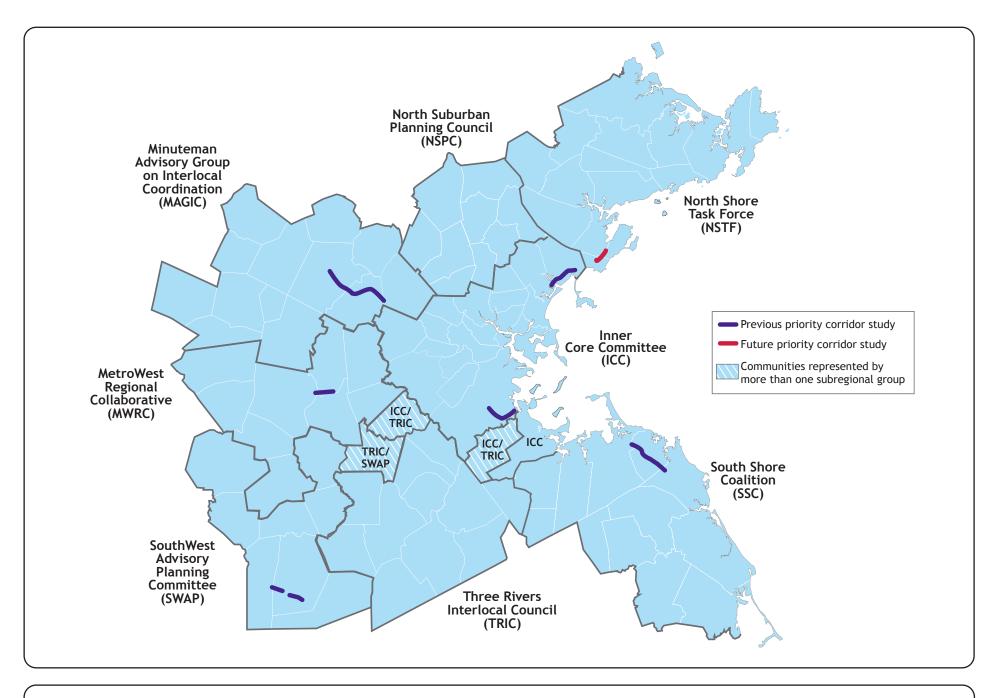
MPO staff recommend that the corridor that includes Route 1A and ancillary streets in the Vinnin Square area of Swampscott, Salem, and Marblehead be selected for study. The Towns of Swampscott and Marblehead and the City of Salem requested this study via MPO outreach for the UPWP. Those municipalities asked the MPO staff to perform this study to identify problems related to recent and future developments expected in the area, and then to identify solutions that could be implemented in tandem with MassDOT. The MassDOT Highway Division District 4 Office and the North Shore Task Force expressed their support for and willingness to participate in a study of the selected arterial segment.

4 SUMMARY

The recommended arterial segment and its ancillary streets meet the selection criteria of this study, especially by supporting the transportation improvement priorities of the MPO's LRTP. While the work scope for this study assumed that "as many as two" arterial segments would be selected, the MPO staff does not propose studying a second arterial segment because this study will include ancillary streets (Loring Avenue, Essex Street, Vinnin Street, and Salem Street) in three municipalities, which would require considerable resources for evaluating alternatives (possible improvements).

Staff will present this recommendation to the MPO for discussion and approval. If the MPO approves this corridor selection, staff will meet with officials from Swampscott, Salem, Marblehead, MassDOT, and MAPC to discuss the study specifics, conduct field visits, collect data, and perform various analyses.

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BOSTON REGION MPO



FIGURE 1 Previous Priority Corridor Studies

FFY (2016) Priority Corridors for LRTP Needs Assessment

TABLE 1 Arterial Segments Considered for Study: Priority Corridors for Long-Range Transportation Plan Needs Assessment Study (Arterial Segment Selected for Study Is Highlighted in Green)

Arterial Segment	Community	MAPC Subregion	MassDOT District	Jurisdiction		Functional Class*	Distance (Miles)	Crash Rate (MVMT)	Number of Top-200 High-Crash Locations 2011–13	Number of HSIP- Eligible Crash Clusters 2011–13**	Time Index	Transit Service			Study, Project, or TIP Project	Safety Conditions		Multimodal Significance	Regional Significance	Regional Equity	Implementation Potential	Score	Priority Rating	Summary of Comments
Route 1A	Swampscott	NSTF	4	MassDOT and Town	d Yes	2	1.7	3.9	1	1	1.3	27 MBTA bus stops MBTA bus Routes 441 and 448 MBTA Commuter Rail at Swampscott and Lynn/Central Square	Yes	Yes	MassDOT Project #607761, Intersection and Signal Improvement at Route 1A (Paradise Road) at Swampscott Mall; in preliminary design	4	1	2	4	2	3	16	High	The Towns of Swampscott and Marblehead and the City of Salem requested this study to identify problems and solutions that can be implemented in tandem with MassDOT and the communities. The communities wanted a study of Route 1A at Vinnin Square area including other streets, such as Loring Avenue/Essex Street, Vinnin Street, and Salem Street, because of recent and future development in the area. Location was suggested in 2016 UPWP and TIP outreach. MassDOT Highway Division District 4 has jurisdiction of Route 1A and supports this study.
Route 114	Salem	NSTF	4	MassDOT an City	d Yes	2, 3	1.7	11.9	1	3	1.35	18 MBTA bus stops MBTA bus Routes 450, 451, 455, 456, 459, and 465 MBTA Commuter Rail at Salem and Beverly Ferry service	Yes	Yes Half the segment abuts EJ zones.	Transportation Improvement Study for Routes 1A, 114, and 107 and Other Roadways in Downtown Salem, 2005 CTPS study MassDOT Project #605332, Bridge Replacement (Route 114) North Street over North River; in preliminary design	4	1	2	4	2	2	15	High	Location suggested in 2012 UPWP outreach via an NSTF letter, which suggested that a study [on Routes 114/1A and Route 127 from Swampscott to Gloucester] would include suggestions about how to improve bike facilities and bike-to-rail connections in this heavily traveled tourist region. This builds on NSTF's primary recommendation for that year and the anticipated popularity of the Essex Coastal Scenic Byway in the region.
Route 9	Framingham	MWRC	3	MassDOT	Yes	2	2.4	3.6	2	8	2.23	MWRTA bus Routes 1, 2, 3, 7, and 9	None	Yes Over half the route lies within or adjacent to an EJ zone.	MAPC Land Use/Route 9 Corridor Study (fall 2013). MassDOT Project #603865 is located in Framingham at the intersection of Route 9 and Temple Street; in preliminary design MassDOT Project #608006 Pedestrian Hybrid Beacon Installation at Route 9 and Maynard Road; 25% design stage MassDOT Project #604991, Resurfacing and Related Work on Route 9, includes wheelchair ramp upgrades, additional sidewalks/repairs, and signal improvements; completed in 2011	4	2	3	4	0	1	14	High	This arterial segment was not selected because according to MassDOT District 3, most of the intersections on this corridor have already been studied. In addition, MPO staff studied Route 30 in Framingham and Natick under the FFY 2013 Priority Corridors for LRTP Needs Assessment.
Route 18	Weymouth	SSC	6	MassDOT	Yes	3	4.2	6.8	4	13	1.44	Nine MBTA bus stops MBTA bus Route 225 MBTA Commuter Rail at South Weymouth	Yes	Yes EJ zones lie adjacent to the segment.	Programmed TIP (2017) and MassDOT Project #601630, Reconstruction and Widening on Route 18 (Main Street), from Highland Place to Route 139; construction ends spring 2016 MassDOT Project #603161, Signalization and Improvements on Route 18 (Three Locations) at West Street, Park Avenue, and Columbian Street; completed in spring 2009 MassDOT Project #603738, Traffic Signal Improvements on Route 18 at Pond Street and Pleasant Street; completed in summer 2006	4	1	3	4	1	1	14	High	This arterial segment was not selected because according to MassDOT District 6, a MassDOT project is underway, and no project is needed at this time.
Route 16 (Revere Beach Parkway and Mystic Valley Parkway)		ICC	4	MassDOT and DCR	d Yes	2, 3	1.3	4.8	0	2	2.59	MBTA bus Routes 90, 97, 99, 100, 106, 108, 110, 112, and 134 MBTA Rapid Transit on the Orange Line at Wellington and on the Red Line at Porter Square MBTA Commuter Rail at West Medford and Porter Square	Yes		DCR announced a \$500,000 comprehensive study of the parkway system for bike lanes in FFY 2015. The goals of the study include updated traffic information, assessment of parkway conditions, and assessment and understanding of deficiencies along the heavily cycled parkways.	3	2	3	4	0	1	13	High	This arterial segment was not selected because it is part of the Mystic River Working Group Study. In addition, the Wynn Everett DEIR (2015) includes intersection improvements and mitigated traffic operations for Revere Beach Parkway and Mystic Valley Parkway.
Route 1	Norwood	TRIC	5	MassDOT	Yes	3	4.8	1.2	0	6	2.69	MBTA Commuter Rail at Islington, Dedham Corp Center, Endicott, Norwood Depot, Norwood Central, Windsor Gardens, and Plimptonville		lies adjacent to the southern end	MassDOT's I-95 South Corridor Study, provided a comprehensive evaluation of the I-95 and Route 1 corridors south of Route 128 that included a recommended plan of short-term and long-term improvements (June 2010) MassDOT Project #608052, Route 1 at Morse Street (approved by PRC Nov. 2014); in preliminary design MassDOT Project #605857, Route 1 at University Avenue and Everett Street; Town design is at pre-25% MassDOT Project #605321, Bridge Preservation, Route 1 over the Neponset River; in design stage	1	2	3	4	2	1	13	High	The location has MassDOT projects and studies and it is not recommended for study.
Route 114	Peabody	NSTF	4	MassDOT and Town	d Yes	2	3.5	2.8	2	7	1.3	Three MBTA bus stops MBTA bus Routes 435, 465	Yes	Yes Half the segment abuts an EJ zone.	No projects	3	1	2	3	2	2	13	High	Route 114 in Peabody was listed as a potential corridor in need of signal progression and improvements to accommodate pedestrians and bicyclists.

Arterial Segment Route 3A	Community Quincy	MAPC Subregion ICC	MassDOT District 6	Jurisdiction MassDOT, DCR, and City	System Yes	Functional Class* 3	Distance (Miles) 2.3	Crash Rate (MVMT) 5.6	Number of Top-200 Migh-Crash Locations 2011–13	Number of HSIP- Eligible Crash Clusters 2011–13** 7	Time Index	Transit Service	Late Bus Yes	In or Near Environmental Justice Zone Yes The entire segment lies within or near EJ zones.	Study, Project, or TIP Project MassDOT Project #605729, Intersection and signal improvements at Hancock Street and East/West Squantum streets. The project consists of widening and improvements to the intersection of Hancock Street with East and West Squantum Streets and improvements along Hancock Street to the MBTA access drive; completed in fall 2015. MassDOT Project #606518. As part of the Quincy Redevelopment project, the city plans to construct a new bridge over the existing MBTA tracks that will connect the downtown area at Market Square and Hancock Street. The main goal of the new bridge will be improved pedestrian conditions along Hancock Street. An FFY 2012 CTPS safety and operations study addressed problems at Route 3A and Coddington Street intersection.	Safety Conditions 4	Congested Conditions	Multimodal Significance 2	Regional Significance 4	Regional Equity 0	Implementation Potential 2	Score 13		Summary of Comments Route 3A (Hancock Street) is part of the Quincy Redevelopment project; study completed in April 2011
Route 28	Randolph	TRIC	6	MassDOT and Town	Yes	3	3.2	5.7	0	3	1.46	50 MBTA bus stops MBTA bus Routes 240 and 238 MBTA Commuter Rail at Holbrook/Randolph BAT Route 12	Yes	Yes The entire segment lies within EJ Zones.	MassDOT Project #603716, Resurfacing and Related Work on a Section of Route 28; completed 2007/2008 Conceptual TIP #1002, Route 28 (N. Main Street) Bridge Conceptual TIP #1010, Route 28 (N. Main Street) and Liberty Street intersections Conceptual TIP #1011, Route 28 (N. Main Street) and West Street intersection FFY 2008 Safety and Operations Analyses at Intersections study Arterial Coordination Study, CTPS study (2010)	3	1	2	4	2	1	13	High	The location has several MassDOT projects and CTPS studies and it is not recommended for study.
Route 1A	Salem	NSTF	4	Town	Yes	2	0.8	14.7	0	1	1.32	16 MBTA bus stops MBTA bus Routes 455 and 459 MBTA Commuter Rail at Salem Ferry service	Yes	Yes The entire segment lies within EJ zones.	CTPS Lower North Shore Transportation Improvement Study proposed improvements for Route 1A in Revere in October 2000; an update may be necessary.	3	1	2	4	2	1	13	High	The southern end of this arterial segment is included in the study of Route 1A study at the Vinnin Square area in Marblehead and in Swampscott, selected for study under the FFY 2016 Priority Corridors Study.
Route 129	Wilmington	NSPC	4	MassDOT and Town	Yes	3	2.9	6.8	1	8	1.3	MBTA Commuter Rail at Wilmington, North Wilmington, Anderson/Woburn, and Reading	N/A	None	MassDOT Project #601732, Rehabilitation, Route 129 (Lowell Street) from Route 38 (Main Street) to Woburn Street. The project includes full-depth reconstruction and widening, accessible (ADA-compliant) sidewalks, new tree plantings, and bicycle accommodation within the newly paved shoulders. The intersection of Route 129 and 38 was realigned with new traffic signals and the bridge over Maple Meadow Brook was replaced; completed in 2009. MassDOT Project #608051 will reconstruct Route 38 from Route 62 to the Woburn city line and will add bike lanes, sidewalks, turn lanes, and signal upgrades; in preliminary design.	4	1	2	3	2	1	13	High	N/A
Route 60	Arlington	ICC	4	Town	Yes	3	0.9	6.0	1	2	1.34	Eight MBTA bus stops MBTA bus Routes 67, 62, 76, 77, 78, 79, 80, 84, and 350	Yes	Yes	CTPS and MAPC Community Transportation Technical Assistance Program evaluated the high-crash location at the intersection at Massachusetts Avenue, March 2010. MassDOT Project #606885, will connect the two legs of the Minuteman Bikeway and improve traffic operations and safety and pedestrian safety in the Arlington Center area. The critical segment in the Arlington Center area has a project programmed in the FFY 2014 TIP.	4	1	3	3	0	1	12	Medium	N/A
Alewife Brook Parkway	Cambridge	icc	6	MassDOT and DCR	Yes	2	0.8	7.2	0	2	2.41	MBTA bus Routes 79, 350, 62, 67, 74, 76, 78, 84, and 351 MBTA Rapid Transit on the Red Line MBTA Commuter Rail at Porter Square		Yes Most of the segment lies within or adjacent to EJ zones.	Alewife Studies, Phase II, CTPS study (2009). DCR announced a comprehensive study of the parkway system for bike lanes. MassDOT Project #605637, Improvements at Route 2 and Route 16. The purpose of this project is to perform minor widening, eliminate a merge condition, and improve throughput capacity and vehicle queue storage at the intersection of Route 2 and Route 16 (Alewife Brook Parkway); under construction.	3	2	2	4	0	1	12	Medium	The Fresh Pond Residents Alliance identified Fresh Pond Parkway and Alewife Brook Parkway as locations in need of transportation improvements. Concerns include pedestrian safety of young students walking to Shady Hill School because of high traffic volumes, environmental issues, and livability.
Route 138	Canton	TRIC	6	MassDOT	No	3, 2	2.6	4.7	0	3	2.26	MBTA Commuter Rail at Route 128, Canton Junction, and Canton Center	N/A	None	MassDOT Project #603883, Reconstruction on Route 138, from I-93 to Dan Road; in preliminary design MassDOT Project #605807, Improvements on Route 138 from Randolph Street to Washington Street; completed in 2011 MassDOT Project #602745, Improvements and Signalization, Route 138 at Washington Street and at Randolph Street; completed in spring 2009 Route 138 Corridor Study, CTPS study (July 2001)	3	2	2	2	2	1	12	Medium	Many locations in this segment have MassDOT projects or studies MassDOT District 6 says that the area around I-93, which has congestion issues, was evaluated by a consultant for a private company.

Arterial Segment Route 16 (Revere		MAPC Subregion	MassDOT District	Jurisdiction MassDOT and	National Highway System Yes	Functional Class*	Distance (Miles)	Crash Rate (MVMT)	Number of Top-200 High-Crash Locations 2011–13	Number of HSIP- Eligible Crash Clusters 2011–13**			Late Bus	In or Near Environmental Justice Zone Yes	Study, Project, or TIP Project The Lower North Shore Transportation Improvement Study,	Safety Conditions	Congested Conditions	Multimodal Significance	Regional Significance	Regional Equity	Implementation Potential 0		Priority Rating	Summary of Comments This arterial segment was not selected because it is part of
Beach Parkway)				DCR								and 111 MBTA Commuter Rail at Chelsea		The entire segment lies within EJ zone.	CTPS study (2000) DCR announced a comprehensive study of the parkway system for bike lanes.									the Mystic River Working Group Study. In addition, the Wynn Everett DEIR (2015) includes intersection improvements and mitigated traffic operations for Revere Beach Parkway and Mystic Valley Parkway.
Route 16 (Revere Beach Parkway)	Everett	ICC	4	MassDOT and DCR	Yes	2	1.7	2.8	1	7	1.38	3 MBTA bus Routes 97, 99, 106, 110, 112, 104, 105, and 109 MBTA Orange Line Rapid Transit at Wellington and MBTA Commuter Rail at Chelsea	Yes	Yes The entire segment lies within EJ zones.	DCR announced a \$500,000 comprehensive study of the parkway system for bike lanes in FFY 2015. The goals of the study include updated traffic information, assessment of parkway conditions, and assessment and understanding of deficiencies along the heavily cycled parkways.	3	1	3	4	0	1	12	Medium	This arterial segment was not selected because it is part of the Mystic River Working Group Study. In addition, the Wynn Everett DEIR (2015) includes intersection improvements and mitigated traffic operations for Revere Beach Parkway and Mystic Valley Parkway.
Routes 4 and 225	Lexington	MAGIC	4	MassDOT	Yes (part)	3, 5	0.7	6.3	0	2	1.3	3 Nine MBTA bus stops MBTA Route 62	Yes	None	MassDOT section from I-95 to Hartwell Ave, was the subject of a Town study (Hartwell Avenue Traffic Mitigation Plan Bedford Street Concept Plan), and a road safety audit was performed for this segment in November 2011 CTPS FFY 2008 Safety and Operations at Intersections Study, Massachusetts Avenue at Maple Street	3	1	2	3	1	2	12	Medium	The MAGIC subregion and the Towns of Lexington and Bedford requested that this corridor be included in the FFY 2012 UPWP for study. The MassDOT section from I-95 to Hartwell Avenue was the subject of a Town study.
Route 1A (Lynnway)	Lynn	ICC	4	MassDOT and DCR	Yes	2, 3, and 5	3.1	3.2	0	5	1.36	MBTA bus stops MBTA bus Routes 426, 439, 441, 442, 448, 449 MBTA Commuter Rail at River Works, Lynn/ Central Square, and Swampscott Ferry service	Yes	Yes The entire segment lies within EJ zones.	TIP Project #1321, Route 1A Lynnway at Blossom Street; conceptual TIP Project #1322, Route 1A Lynnway intersection at Market Street; conceptual	2	1	2	4	0	3	12	Medium	This arterial segment was selected for MPO study under FFY 2015 Priority Corridors Study for LRTP Needs Assessment.
Route 107	Lynn	icc	4	MassDOT and Town	Yes	3	1.3	12.6	4	14	1.19	MBTA bus Routes 424,426, 436, 441, 442, 450, 455, 456, 459, 429, and 435 MBTA Commuter Rail at River Works, Lynn/Central Square, and Swampscott Ferry service	Yes	Yes The entire segment lies within EJ zones.	MassDOT Project #604952, Bridge Replacement, Route 107 over the Saugus River MassDOT Project #26710, Bridge Replacement, Route 107 over the Saugus River (Fox Hill Bridge); completed spring 2013 MassDOT Project #603938, Western Avenue Bridge over Saugus River (Fox Hill Bridge) TIP Project #374, Lynn Garage (transit)	4	0	3	4	0	1	12	Medium	This arterial segment was not selected for study because there is an ongoing Route 107 Corridor Study in Lynn and Salem, which is being conducted by MassDOT in conjunction with Lynn and Salem.
Route 28	Milton	ICC and TRIC	6	MassDOT, DCR, and Town	Yes	3	3.8	3.3	1	4	1.3	MBTA bus stops MBTA bus Routes 240, 245, 24, 28, 26, 30, 31, and 33 MBTA Red Line Rapid Transit at Mattapan/Ashmont Station BAT Route 12	Yes	Yes EJ zones are located at the northern end.	MassDOT Project #607342, Intersection and Signal Improvements at Route 28 (Randolph Avenue) and Chickatawbut Road; in preliminary design MassDOT Project #106901, Roadway Reconstruction on Route 28 (Randolph Avenue) from Reedsdale Road to Milton/Quincy town line; completed 2008 Conceptual TIP #1008, Reconstruct the Intersection of Blue Hills Parkway and Brook Road	3	1	2	3	1	2	12	Medium	This arterial segment was not selected because there have been several improvements in this segment in recent years.
Route 9	Natick	MWRC	3	MassDOT	Yes	2	3.5	4.9	2	9	2.32	2 MWRTA bus Routes 1, 4, 9, and 10	None		MAPC Land Use/Route 9 Corridor Study (fall 2013) MassDOT Project #601586 is currently reconstructing the Route 9/Oak Street intersection and should address some of the congestion and safety issues at the intersection. MassDOT Project #605313 will reconstruct the Route 9/Route 27 interchange; 25% project design stage. MassDOT Project #604991, Resurfacing and Related Work on Route 9, includes wheelchair ramp upgrades, additional sidewalks/repairs, and signal improvements; completed in 2011	4	2	1	4	0	1	12	Medium	According to MassDOT District 3, the Route 9 and Oak Street intersection is currently under construction. The Route 9 and Route 27 interchange is currently in design.
Route 16	Newton	ICC	6	MassDOT and City	Yes	3	4.3	4.1	0	4	1.52	MBTA Routes 59, 170, 505, 553, 554, and 556 MBTA Green Line Rapid Transit MBTA Commuter Rail at West Newton		Yes An EJ zone lies adjacent to the segment.	MassDOT Project #606780, Bridge Rehabilitation, Route 16 (Washington Street) over I-90, MBTA/CSX Corporation and Access Road Conceptual TIP #1067, Washington Street (Phase 2), from Commonwealth Avenue to Perkins Street	3	1	2	4	0	2	12	Medium	In FFY 2014, a subregional study was conducted on Washington Street in Newton. The location was suggested in 2014 LRTP outreach through verbal comments at a 495/MetroWest Partnership meeting.
Route 1	Walpole	TRIC	5	MassDOT	Yes	3	3.3	2.1	0	2	1.38	B MBTA Commuter Rail at Sharon and Walpole	N/A		MassDOT's I-95 South Corridor Study presented a comprehensive evaluation of the I-95 and Route 1 corridors south of Route 128 and included a recommended plan of short-term and long-term improvements (June 2010)	1	1	3	4	2	1	12	Medium	The location has MassDOT projects and studies.

Arterial Segment	Community	MAPC Subregion	MassDOT District	Jurisdiction	National Highway System	Functional Class*	Distance (Miles)	Crash Rate (MVMT)	Number of Top-200 N High-Crash Locations 2011–13	umber of HSIP- Eligible Crash Clusters 2011–13**	Travel Time Index		Crowded or Late Bus	In or Near Environmental Justice Zone	Study, Project, or TIP Project	Safety Conditions	Congested Conditions	Multimodal Significance	Regional Significance	Regional Equity	Implementation Potential	Score	Priority Rating	Summary of Comments
Route 16	Wellesley	MWRC	6	MassDOT and Town	Yes	4	4.5	7.3	1	3		MBTA Commuter Rail at Wellesley Square, Wellesley Hills, and Wellesley Farms MWRTA Route 8	N/A	Yes The southern end of the segment lies in an EJ zone.	MassDOT Project #94762, Bridge Rehabilitation, Route 16 (Washington Street) over Route 9, including relocation of retaining wall; completed summer 2010.	4	1	2	3	0	2		Medium	The location was suggested in 2014 LRTP outreach through verbal comments at a 495/MetroWest Partnership meeting.
Route 2	Acton	MAGIC	3	MassDOT	Yes	2	2.1	1.4	0	1	3.35	MBTA Commuter Rail at South Acton and West Concord	N/A	Yes	MassDOT Project #604472, Resurfacing and Related Work on Route 2 (includes all of Acton); completed in spring 2014 MassDOT Project #607748, Intersection and Signal Improvements on Route 2 and Route 111 at Piper Road and Taylor Road; in preliminary design MassDOT Project #604609, Traffic Sign Replacement and Safety Improvements on Route 2; completed in summer 2009 TIP Project #606223, Bruce Freeman Rail Trail Construction (Phase II-B) in Acton and Concord to connect the trail across Route 2, programmed in TTY 2018 TIP	1	2	2	4	1	1	11	Medium	Location has MassDOT projects. A MassDOT road safety audit is scheduled for the Piper Road/Taylor Road intersection; the project is in the preliminary design phase. The MAGIC subregion expressed interest in a Route 2 study.
Route 62	Bedford	MAGIC	4	MassDOT and Town	No	5	0.9	6.9	0	1		Three MBTA bus stops MBTA bus Route 62	Yes	None	Great Road Project: Master Plan and Conceptual Design, prepared by Vanasse Hagen Brustlin Inc. (VHB) for the Town of Bedford in 2011. The plan was to improve pedestrian and bicycle access, recommend streetscape improvements that will highlight the "Center" of Bedford while taking into consideration traffic flow through the area, crosswalk locations, intersection and traffic control improvements, property access, and parking.	3	1	2	2	2	1	11	Medium	Forms part of Routes 4 and 225 arterial segment.
Route 2 (Fresh Pond Parkway)	Cambridge	ICC	6	DCR	Yes	2	1.3	3.5	0	2	1.51	MBTA bus Routes 75, 71, 72, 73, 74, and 78 MBTA Red Line Rapid Transit MBTA Commuter Rail at Porter Square	Yes	Two EJ zones are located within 0.5 miles of the segment.	DCR announced that the agency will conduct a traffic study of several intersections along Mount Auburn Street and Fresh Pond Parkway, in partnership with the City of Cambridge and the MBTA. The study will focus on safety measures, bus prioritization, and accessibility. Conceptual TIP project #987 would acquire Minuteman Path right-of-way in Watertown to connect Minuteman Bikeway from Arlington, Cambridge, and Watertown to Dr. Paul Dudley White Bike Path in Boston.	3	1	2	4	0	1	11	Medium	The Fresh Pond Residents Alliance identified Fresh Pond Parkway and Alewife Brook Parkway as locations in need of transportation improvements. Concerns include pedestrian safety of young students walking to Shady Hill School because of high traffic volumes, environmental issues, and livability.
Route 2	Concord	MAGIC	4	MassDOT	Yes	2	5.2	1.1	1	6	2.68	MBTA Commuter Rail at West Concord, Concord, and Lincoln	N/A	Yes. One EJ zone is adjacent to the segment.	MassDOT Project #602894, Crosby's Corner (Route 2 at Route 2A) Improvements; under construction MassDOT Project #602091, Concord Rotary; in preliminary design MassDOT Project #604069, Bridge Replacement over Sudbury River; in preliminary design MassDOT Project #604630, Resurfacing and Related Work on Route 2; completed in 2010 MassDOT Project #604472, Resurfacing and Related Work on Route 2; completed in 2014 Programmed (March 2014) TIP Project #606223: Bruce Freeman Rail Trail Construction (Phase II-B) in Acton and Concord, will connect the trail across Route 2, in preliminary design	2	2	2	4	1	0	11	Medium	FFY 2013 Priority Corridors for LRTP Needs Assessment Study (Concord and Lincoln) Route 2 was suggested during MPO outreach as a route experiencing congestion that affects MAGIC communities as well as Cambridge. There are many projects and studies conducted for this corridor, including the Route 2 (Crosby's Corner) improvements and Concord Rotary upgrade and improvements.
Route 99	Everett	ICC	4	MassDOT, DCR, and City	Yes	3	2.4	1.4	0	2	2.4	40 MBTA bus stops MBTA bus Routes 97, 104, 105, 109, 110, 112, 99, and 106	Yes	Yes The entire segment lies within EJ zones.	MassDOT Project #602383 reconstructed Route 99 with a traffic signal upgrade, from Second Street to the Malden city line in 2008. MassDOT Project #601580 reconstructed Route 99 from Sweetser Circle to Second Street in 2004. MassDOT Project #602382 reconstructed Route 99 from Sweetser Circle to the Alford Street Bridge in 2013.	2	2	2	4	0	1	11	Medium	Not recommended for study because the MassDOT projects listed completely reconstructed Route 99 with signal improvements from Alford Street Bridge to the Malden city line.
Route 30 between I- 90 and Route 9	-Framingham	MWRC	3	Town	Yes (part)	3	1.1	4.5	0		1.3	MWRTA bus Routes 10 and 11 MBTA Commuter Rail at Natick and West Natick	None		FFY 2013 Priority Corridors for LRTP Needs Assessment Study MassDOT Project #86450, Roadway Reconstruction and Related Work on sections of Route 126 and Route 30 (includes traffic signal improvements at their intersection); construction ended in summer 2005.	3	1	2	3	1	1	11	Medium	This location is not recommended for study because of an FFY 2013 Priority Corridors for LRTP Needs Assessment Study that was performed for the corridor. Framingham and Natick have advanced some of the recommendations into projects.

Arterial Segment			MassDOT District	Jurisdiction	System	Functional Class*	Distance (Miles)	Crash Rate (MVMT)	Number of Top-200 High-Crash Locations 2011–13	Number of HSIP- Eligible Crash Clusters 2011–13**	Time Index	Transit Service	Crowded or Late Bus	In or Near Environmental Justice Zone	Study, Project, or TIP Project	Safety Conditions	Congested Conditions	Multimodal Significance	Regional Significance	Regional Equity	Implementation Potential			Summary of Comments
Route 3A	Hingham	SSC	5	MassDOT	Yes	3	4.7	1.9	0	1	1.3	3 MBTA Commuter Rail at Cohasset, Nantasket Junction, West Hingham, and East Weymouth Ferry service	N/A	None	There are two approved projects that are not advancing in design: MassDOT Project #603137, Intersection Improvements on Route 3A at Kirby Street. There has been local interest in installing a traffic signal at this intersection; in preliminary design. MassDOT Project #605168, Intersection Improvements at Route 3A/Summer Street Rotary. The Town 's consultant prepared preliminary concepts for proposals at this location. Design is pre-25%.	2	1	2	3	0	3	11	Medium	In FFY 2015, a subregional priority roadway study was conducted for Route 3A in Hingham and Hull. The location received strong support from the Towns of Hingham and Hull, as well as the South Shore Coalition and MassDOT Highway Division District 5 Office
Route 138	Milton	ICC and TRIC	6	MassDOT	Yes	2	3.6	4.2	0	3	1.58	MBTA bus Route 245 MBTA Commuter Rail at Route 128 Station MBTA Red Line Rapid Transit at Mattapan Station	Yes	Yes Half of the segment is contained within EJ zones.	MassDOT Project #607763, Intersection and Signal Improvements at Two Locations: Route 138 (Blue Hill Avenue) at Atherton Street and Bradlee Road and Route 138 (Blue Hill Avenue) at Milton Street and Dollar Lane, programmed in FFY 2019 TIP	3	1	2	3	1	1	11	Medium	Congestion issues have been identified on this route, from the I-93 interchange to Mattapan Square.
Route 9	Newton	ICC	6	MassDOT	Yes	2	3.1	1.7	0	2	1.73	3 Six MBTA bus stops MBTA bus Routes 60, 52, and 59 MBTA Green Line	Yes	Yes An EJ zone in Brookline is 0.3 mi from the segment.	MassDOT Project #604327, Resurfacing and Related Work on Route 9 (Boylston Street) from the Wellesley/Newton city line to Newton/Brookline city line; completed in 2012 MassDOT Project #601704, Reconstruction and Signal Improvements on Walnut Street, from Homer Street to Route 9; in design MassDOT Project #606635, Reconstruction of Highland Avenue, Needham Street, and Charles River Bridge, from Webster Street to Route 9; 25% design stage MassDOT Project #604327, resurfaced this segment, including updates to guardrails and improvements to the existing drainage structures; construction was completed in 2012.	2	1	3	4	0	1	11	Medium	According to MassDOT District 6, improvements were recently made to accommodate new developments. An analysis of the new existing conditions would be helpful to compare with the future projected conditions.
Route 16 (Revere Beach Parkway)	Revere	ICC	4	MassDOT and DCR	d Yes	2	1.5	2.9	0	3	1.43	MBTA bus Routes 110, 116, 117, 119, 424, 426, 428, 448, 449, 450, 455, and 459 MBTA Rapid Transit on Blue Line MBTA Commuter Rail at Chelsea	Yes	Yes The entire segment lies within EJ Zones.	DCR announced a \$500,000 comprehensive study of the parkway system for bike lanes in FFY 2015. The goals of the study include updated traffic information, assessment of parkway conditions, and assessment and understanding of deficiencies along the heavily cycled parkways. The Wynn Everett DEIR (2015) includes intersection improvements and mitigated traffic operations for Revere Beach Parkway and Mystic Valley Parkway.	2	1	3	4	0	1	11	Medium	This arterial segment was not selected because it is part of the Mystic River Working Group Study. In addition, the Wynn Everett DEIR (2015) includes intersection improvements and mitigated traffic operations for Revere Beach Parkway and Mystic Valley Parkway.
Route 1A	Revere	ICC	4	MassDOT	Yes	2	1.5	2.9	0	1	3.17	7 15 MBTA bus stops MBTA bus Routes 110, 116, 117, 411, 424, 426, 439, 441, 442, 448, 449, 450, and 455 MBTA Rapid Transit on Blue Line MBTA Commuter Rail at Chelsea and River Works	Yes	Yes The entire segment lies within EJ zones.	CTPS Lower North Shore Transportation Improvement Study proposed improvements for Route 1A in Revere in October 2000; an update may be necessary. Conceptual TIP Project #982, Mahoney Circle (Bell Circle) Grade Separation	2	2	2	4	0	1	11	Medium	This arterial segment was not selected because it is part of the Mystic River Working Group Study. In addition, the Wynn Everett DEIR (2015) includes intersection improvements and mitigated traffic operations for Revere Beach Parkway and Mystic Valley Parkway.
Route 1	Sharon	TRIC	5	MassDOT	Yes	3	1.8	0.8	0	2	1.38	MBTA Commuter Rail at Sharon and Walpole	N/A	None	MassDOT's I-95 South Corridor Study, provided a comprehensive evaluation of the I-95 and Route 1 corridors south of Route 128 that included a recommended plan of short-term and long-term improvements (June 2010) MassDOT Project #603622, Bridge Rehabilitations, Route 1/Route I-95; completed in 2010	2	1	3	2	2	1	11	Medium	Segment has MassDOT projects and studies.
Route 3A	Weymouth	SSC	6	MassDOT	Yes	3	1.9	3.6	0	2	1.14	30 MBTA bus stops MBTA bus Routes 220, 221, and 222 MBTA Commuter Rail at Quincy Center, Weymouth Landing/ East Braintree, and West Hingham Ferry service	Yes	Yes An EJ zone in Quincy is 0.2 miles from the segment.	Advertised (2008) TIP and MassDOT Project #604382, Route 3A (Washington Street) Bridge; construction ends winter 2016/2017 MassDOT Project #602703, Bridge Rehabilitation, Route 3A (Lincoln Street) over the Weymouth Back River; completed in autumn 2006	3	0	2	4	2	0	11	Medium	MassDOT District 6 and Town of Weymouth interest are critical for implementation.

Arterial Segment	Community	MAPC Subregion	MassDOT District	Jurisdiction	National Highway System	Functional Class*	Distance (Miles)	Crash Rate (MVMT)		umber of HSIP- Eligible Crash Clusters 2011–13**	Travel Time Index	Transit Service	Crowded or Late Bus	In or Near Environmental Justice Zone	Study, Project, or TIP Project	Safety Conditions	Congested Conditions	Multimodal Significance	Regional Significance	Regional Equity	Implementation Potential	Score	Priority Rating	Summary of Comments
	Holliston	MWRC	3	MassDOT and Town		3	2.3	4.4	0	2		MWRTA bus Route 6	None	None	MassDOT Project #605745, Reconstruction of Route 16 from Quail Run to the Sherborn town line; in preliminary	3	1	1	3	0	2	10	Low	Location has MassDOT projects and CTPS studies, which have not been implemented.
															design MassDOT Project #602462 will enhance safety and improve efficiency by installing a new traffic signal at the intersection of Route 16 at Route 126 and at Oak Street in Holliston;									The 495/MetroWest Partnership expressed interest in a Route 16 study. The section that experiences the most crashes is the town
															25% design stage 2011 CTPS study, Route 126 Corridor: Transportation									center portion (under Town jurisdiction). A road safety audit was performed for the town center portion in December 2012.
															Improvement Study 2008 CTPS study, Washington Street (Route 16/126) at									
															Hollis Street									
Route 135	Wellesley	MWRC	6	MassDOT and Town	Yes	3	3	6.2	0	1	1.3	MBTA Commuter Rail at Natick, Wellesley Square, and Wellesley Hills MWRTA bus Route 8	None	Yes Most of the segment lies adjacent to EJ zones.	No projects	3	1	2	3	0	1	10	Low	None
Route 20	Weston	MWRC	6	MassDOT	Yes	3	3.3	2.7	0	2	2.43	MBTA bus Route 70	Yes	Yes	No projects	1	2	2	4	0	1	10	Low	A congestion study was suggested through UPWP and LRTP outreach in 2012, 2013, and 2014 by MAGIC; a formal letter
												MBTA Commuter Rail at Waltham and Kendal Green		An EJ Zone is located 0.1 mi from the end of the segment.										was submitted and verbal comments were made at an MWRC subregion meeting. The location was resubmitted in comment on Draft FFY 2014 UPWP.
Memorial Drive (Routes 2 and 3)	Cambridge	ICC	6	DCR	Yes	2	4.2	3.0	0	5	1.3	MBTA bus Routes 747, 1, 47, 64, 66, 70, 70A,	Yes	Yes	DCR announced a \$500,000 comprehensive study of the parkway system for bike lanes in FFY 2015. The goals of	2	1	2	4	0	0	9	Low	None
												71, 73, 86, and 701 MBTA Rapid Transit available on the Red and Green Lines		Most of the segment lies within or adjacent to EJ Zones.	the study include updated traffic information, assessment of parkway conditions, and assessment and understanding of deficiencies along the heavily cycled parkways.									
												MBTA Commuter Rail at North Station, Back Bay, Yawkey, Porter Square, and Belmont												
Route 3A	Cohasset	SSC	5	MassDOT	Yes	3	3.1	4.0	0	2	1.09	MBTA Commuter Rail at Nantasket Junction,	N/A	None	FFY 2013 Subregional Priority Corridor Study.	2	0	2	2	2	1	9	Low	FFY 2013 Subregional Priority Corridor study was conducted within the segment.
												Cohasset, and North Scituate			MassDOT Project #608007, Corridor Improvements and Related Work on Justice Cushing Highway (Route 3A), from Beechwood Street to the Scituate town line, includes new traffic signal equipment and pedestrian and bicycle accommodation; preliminary design									MassDOT District 5 comments note two approved projects: MassDOT Projects #608007 (in preliminary design stage) and Project #605664 (100% design stage).
															The corridor is within the limits of MassDOT Project #605664, Resurfacing and Related Work on Route 3A (Duxbury town line northerly to Scituate town line); 100% design stage; no construction funding identified									
Route 2	Lincoln	MAGIC	4	MassDOT	Yes	2	3	0.9	1	1	2.68	MBTA Commuter Rail at Concord and Lincoln	N/A	None	MassDOT Project #602894, Crosby's Corner (2 at 2A) Improvements; under construction	2	2	2	2		1	9	Low	Route 2 was suggested during MPO outreach as a route experiencing congestion that affects MAGIC communities and Cambridge.
															MassDOT Project #604629, Resurfacing and Related Work on Route 2; completed in 2010 FFY 2013 Priority Corridors for LRTP Needs Assessment Study (Concord and Lincoln)									There are many projects and studies conducted for this corridor, including the Route 2 (Crosby's Corner) improvements.
Route 135	Natick	MWRC	3	Town	Yes	3	2.1	10.3	1	3	1.33	MWRTA bus Routes 10	None	None	MassDOT Project #600573 reconstructed Route 135 in	4	1	2	1	0	1	9	Low	Congestion in the downtown area; likely focus area would be
												and 11 MBTA Commuter Rail at Natick and West Natick			Natick in 2009. More extensive improvements were proposed in the downtown area, on East Central Street between North Main Street and Union Street, including signal upgrades, new sidewalks, pavement rehabilitation, and shoulders.									on the intersection of Route 135 at Route 27 and the intersection of Route 135 at Speen Street because of the crash history of those locations.
															2010 CTPS study, West Central Street (Route 135) at Speen Street.									
Route 1	Westwood	TRIC	6	MassDOT	Yes	3	1.1	1.1	0	0	1.3	None	N/A	None	MassDOT's I-95 South Corridor Study provided a comprehensive evaluation of the I-95 and Route 1 corridors south of Route 128 and included a recommended plan of short-term and long-term improvements (June 2010)	0	1	2	3	2	1	9	Low	Segment has MassDOT projects and studies.
															MassDOT Project #603162, Route 128 Add-a-Lane Bridges (Bridge III), Route 1 and 1A over I-95/128; completed in 2012									
Routes 4 and 225	Bedford	MAGIC	4	MassDOT and Town	d No	5	4	2.4	0	1	1.27	Three MBTA bus stops MBTA bus Route 62	Yes	None	Great Road Project: Master Plan and Conceptual Design, prepared by VHB for the Town of Bedford in 2011, in preliminary design	2	0	2	1	2	1	8	Low	The MAGIC subregion and the Towns of Bedford and Lexington requested that the FFY 2012 UPWP and FFY 2013 UPWP include a study of Routes 4 and 225.
Route 62	Concord	MAGIC	4	MassDOT and Town	Yes	3	2.3	4.4	0	1	1.31	MBTA Commuter Rail at Concord and West	N/A	None	No projects	3	1	1	1	1	1	8	Low	None
			<u> </u>	<u> </u>								Concord												

Arterial Segment	Community	MAPC Subregion	MassDOT District	Jurisdiction	National Highway System	Functional Class*	Distance (Miles)		High-Crash Locations	umber of HSIP- Eligible Crash Clusters 2011–13**	Time		Crowded or Late Bus	In or Near Environmental Justice Zone	Study, Project, or TIP Project	Safety Conditions	Congested Conditions	Multimodal Significance	Regional Significance	Regional Equity	Implementation Potential	Score	Priority Ratir	ng Summary of Comments
Route 9	Wellesley	MWRC	6	MassDOT	Yes	2	5.2	3.3	0	9		MBTA Commuter Rail at Wellesley Hills and Wellesley Farms MWRTA bus Route 1	None	None	MassDOT Project #601586, Intersection Improvements at Route 9 (Worchester Street) and Oak Street, from 1500 feet West of Oak Street to 300 feet East of Overbrook Drive; construction ended in spring 2015 MassDOT Project #607340, Resurfacing on Route 9, from Dearborn Street to the Natick town line; in preliminary design MassDOT Project #606530, Drainage Improvements along Route 9 Boulder Creek Culvert (Design Only); 25% design stage CTPS study: Route 9 Corridor in Wellesley, 2003 MAPC Land Use/Corridor Study (fall 2013)	1	1	2	3	0	1		Low	MassDOT has a preliminary assessment of this corridor that will develop into 25% design plans for roadway improvements.
Route 3A	Marshfield	SSC	5	MassDOT	Yes	3	7.3	2.1	0	0	1.09	GATRA bus MBTA Commuter Rail at Greenbush	None		The corridor is within the limits of MassDOT Project #605664, Resurfacing and Related Work on Route 3A (Duxbury town line northerly to Scituate town line), work includes patching and microsurfacing, shoulder reconstruction, and drainage structures; 100% design stage; no construction funding identified	0	0	2	2	2	1	7	Low	None
Route 16	Natick	MWRC	3	Town	Yes	3	2.6	1.4	0	0	1.19	None	N/A	Yes	No projects	0	0	2	3	0	2	7	Low	The 495/MetroWest Partnership expressed interest in a Route 16 study. Specific issues in this segments include improvements to accommodate pedestrians and bicyclists.
Route 129	Reading	NSPC	4	MassDOT and Town	Yes	3	2.9	3.2	0	0	1.56	11 MBTA bus stops MBTA bus Route 136 MBTA Commuter Rail at Wakefield, Reading, and Woburn	Yes	None	No projects	0	1	2	1	2	1	7	Low	None
Route 16	Sherborn	SWAP	3	Town	Yes	3	4.3	1.7	0	1	1.35	None	N/A		2002 CTPS study, Traffic Congestion in SWAP Subregion: Sherborn Town Center Traffic-Flow Improvement Study Conceptual TIP #915, Washington Street (Route 16)	1	1	1	2	0	2	7	Low	Location was suggested in 2014 LRTP outreach at a 495/MetroWest Partnership meeting. The section that experiences the most crashes and congestion is the town center portion, where Route 16 and Route 27 combine and split.
Route 3A	Scituate	SSC	5	MassDOT	Yes	3	4.8	1.0	0	0	1.04	MBTA Commuter Rail at Greenbush, North Scituate, and Cohasset	N/A		FFY 2013 Subregional Priority Corridor Study The corridor is within the limits of MassDOT Project #605664, Resurfacing and Related Work on Route 3A (Duxbury town line northerly to Scituate town line); no construction funding identified. Work includes patching and microsurfacing, shoulder reconstruction, and drainage structures; 100% design stage.	0	0	2	1	2	1	6	Low	The FFY 2013 Subregional Priority Corridors Study was conducted within the segment. MassDOT District 5 comments refer to MassDOT Project #605664 (in 100% design stage).
Route 9	Southborough	MWRC	3	MassDOT	Yes	2	0.79	4.6	0	0	1.83	MWRTA bus Route 7	None		MAPC Land Use/Route 9 Corridor Study (fall 2013). The CTPS Safety and Operations at Intersections study evaluated congestion and safety issues at the Route 9/Oak Hill Road/Central Street intersection in FFY 2012. MassDOT's I-495/Route 9 study, November 2013. The western section of Route 9 in Southborough between the I-95 interchange and Crystal Pond Road was evaluated for short-term and long-term improvements as part of this study. MassDOT Project #607172, Resurfacing and Related Work on Route 9, from Westborough to just west of White Bagley Road; construction ends in spring 2016		1	2	2	0	0	6	Low	Most of the intersections on this corridor have already been studied, as MassDOT District 3 has noted.

Source: Central Transportation Planning Staff.

Selection Criteria

Safety Conditions: Segment has a high crash rate for its functional class, contains an HSIP-eligible crash location, a top-200 high-crash location, and/or a significant number or HSIP-eligible clusters of pedestrian or bicycle crashes.

Congested Conditions: Segment has a Travel Time Index of at least 1.3, which is assigned a score of one point; or a Travel Time Index of at least 2.0, which is assigned an additional point. A score of one or two signifies that the segment experiences delays during peak periods.

Multimodal Significance: Segment supports transit, bicycle, or pedestrian activities; needs to have improvements for those activities; and/or has a high volume of truck traffic serving regional commerce.

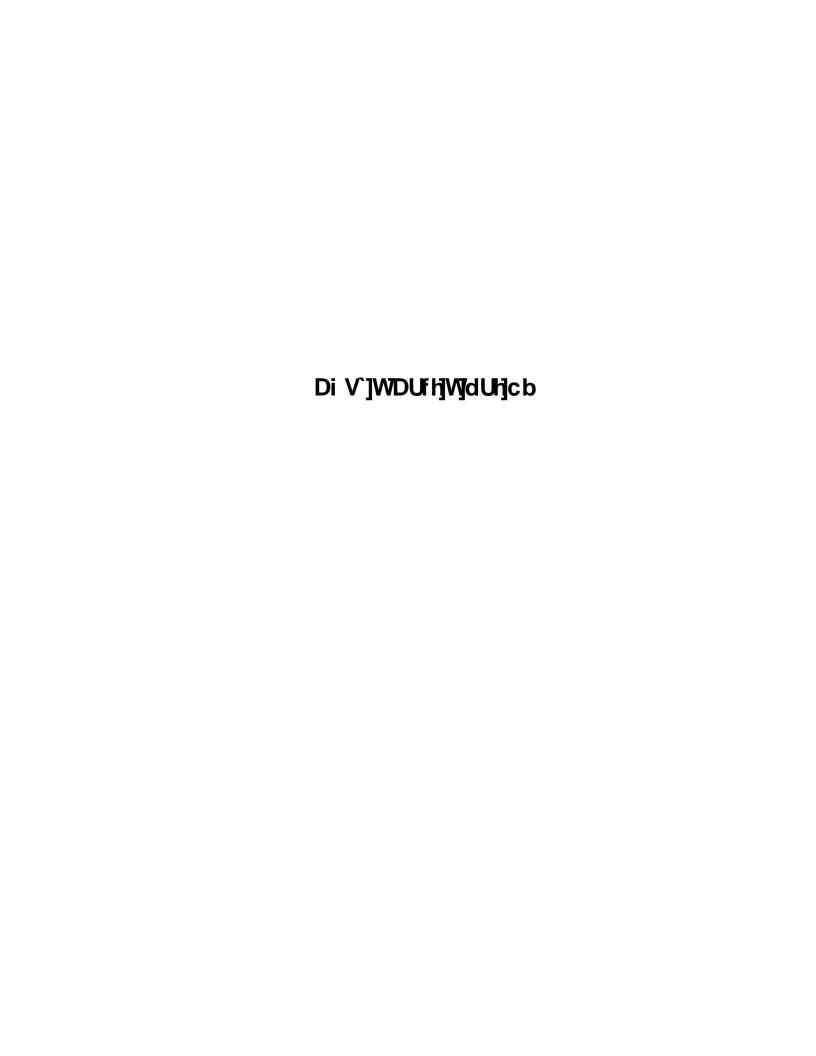
Regional Equity: Segment is proposed or endorsed by its roadway administrative agency or agencies; is proposed or endorsed by its roadway administrative agency or agencies; is proposed or endorsed by its roadway administrative agency or agencies; is proposed or endorsed by its roadway administrative agency or agencies; is proposed or endorsed by its roadway administrative agency or agencies; is proposed or endorsed by its roadway administrative agency or agencies; is proposed or endorsed by its proposed or endorsed by its roadway administrative agency or agencies; is proposed or endorsed by its proposed or endorsed by its roadway administrative agency or agencies; is proposed or endorsed by its proposed or endorsed by it

2 = principal arterial; 3 = principal arterial other (rural minor arterial or urban principal arterial); 5 = minor arterial (urban minor arterial or rural major collector)

HSIP-eligible crash clusters are defined by MassDOT as crash clusters that rank within the top 5 percent of crash clusters for each regional planning agency, based on the Equivalent Property Damage Only (EDPO) index, property-damage-only crashes are awarded one point each, crashes involving injuries are awarded five points each, and fatal crashes are given 10 points each. In the Boston region, the 896 intersections in the top 5 percent have crash clusters with an EDPO value of 42 or greater.

Abbreviations

AADT = annual average daily traffic. ADA = Americans with Disabilities Act. ADT = average daily traffic. BAT = Brockton Areas Transit Authority. CTPS = Central Transportation Planning Staff. DCR = Department of Conservation and Recreation. DEIR = Draft Environmental Impact Report. EJ = Environmental Justice. ENHC = Essex National Heritage Commission. EPDO = equivalent property damage only. FFY = federal fiscal year. GATRA = Greater Attleboro Taunton Regional Transit Authority. HSIP = Highway Safety Improvement Program. ICC = Inner Core Committee. LRTP = Long-Range Transportation Plan. MAGIC = Minuteman Advisory Group on Interlocal Coordination. MAPC = Metropolitan Area Planning Council. Massachusetts Department of Transportation. MBTA = Massachusetts Bay Transportation Authority. MPO = [Boston Region] Metropolitan Planning Organization. MVMT = million vehiclemiles traveled. MWRC = MetroWest Regional Collaborative. MWRTA = MetroWest Regional Transit Authority. NSPC = North Shore Task Force. PRC = MassDOT Project Review Committee. RSA = road safety audit. RTA = regional transit authority. SSC = South Shore Coalition. SWAP = South West Advisory Planning Committee. TIP = Transportation Improvement Program. TRIC = Three Rivers Interlocal Council. UPWP = Unified Planning Work Program.



intersection of Salem St and Vinnin St, for a mixed-use or multi-family development under 40R smart growth regulations that could see up to or over 100 units. No plans have been filed but development concepts are being put together currently.

STUDY AREA - ROAD SEGMENTS

We are looking for a technical study of the Vinnin Square area to identify problems and solutions that can be implemented in tandem between MassDOT, the City of Salem, and the Towns of Marblehead and Swampscott.

The area includes the primary Route 1A (Paradise Road) as well as feeder/arterials (many of which include additional lighted intersections). See the following map for reference. The street segments are:

- 330 Paradise Road north to Loring Ave intersection in Salem
- Essex Street/Loring Ave (from Stop & Shop signal east to Maple Ave, Salem)
- Vinnin Street (from Loring Ave east to Bank of America access)
- Salem Street (from Vinnin St south to Sunbeam Ln)



Initial Scoping Meeting Summary Route 1A-Vinnin Square Priority Corridor Study in Marblehead, Salem, and Swampscott Swampscott Town Hall, First Floor Conference Room May 5, 2016

Meeting started at 12:00 Noon.

Participants from the City of Salem, the Towns of Marblehead and Swampscott, MassDOT Office of Transportation Planning (OTP), MassDOT Highway Division's District 4 Office, and Central Transportation Planning Staff (CTPS) introduced themselves (see attached meeting roster).

Study Background

Mark Abbott of CTPS introduced the Boston Region Metropolitan Planning Organization (MPO) and the study background.

- The study is supported by funding from the MPO. The MPO is responsible for conducting federally required metropolitan transportation planning process. The work of the MPO is conducted by CTPS, staff to the MPO.
- The MPO's Long-Range Transportation Plan (LRTP), Charting Progress to 2040, identified needs for all modes of transportation in the MPO region. The LRTP identified arterial segments where roadways need improvements and modernization.
- The objectives of this study are to identify safety, mobility, access, and other transportation-related problems in the corridor and to develop multimodal solutions to the problems, including increasing the quantity and quality of walking and biking.
- CTPS went through an extensive and comprehensive process and selected this corridor from over 54 arterial segments in the MPO region for study.

Overview of Study Area

Seth Asante of CTPS provided an overview of the study area based on available transportation data. The area's roadway characteristics are summarized as below:

- Functional class: Principal urban arterials (Route 1A and Tedesco Street), minor arterial (Essex Street and Salem Street).
- Jurisdiction: MassDOT has jurisdiction over Route 1A in Salem and Swampscott; the Town of Swampscott has jurisdiction over Essex Street and Salem Street; the City of Salem has jurisdiction over Loring Avenue off of Route 1A and Vinnin Street; and the Town of Marblehead has jurisdiction over Tedesco Street.
- Route 1A and Tedesco Street are on the National Highway System.

- All of the roadways are two-lane, two-way undivided roadways.
- There are ten signalized intersections and several unsignalized intersections and major commercial driveways in the study area.
- Generally 30 mph is the speed limit on the roadways in study area.
- There are sidewalks on both sides of the roadway but few locations are missing sidewalks
- Generally, the study area's roadway has no shoulders or dedicated bike lanes.
- The adjacent land uses are mixed—residential, commercial, recreational, and educational.

Study Vision

Seth Asante said that based on previous discussions with some of the advisory tasks force members, the visions include:

- Transform Route 1A and ancillary roadways to increase safety for all users
- Renovate the study area's roadways into a pedestrian friendly boulevard
- Upgrade the traffic system to be more efficient
- Create a walkable, livable community that promotes human interaction

Study Tasks

Seth Asante presented the limits of the study area. Seth Asante provided an overview of each of the tasks that will be performed in this study, which are described below:

- Collect data: The data to be collected include traffic volumes, pedestrian and bicyclist volumes, vehicle speeds, crashes, traffic signal timings and sequence, and transit service data. MassDOT Highway Division will collect all the traffic volume and speed data and provide the signal timings and intersection layout information. The Massachusetts Bay Transportation Authority will provide transit service data.
- Existing conditions analyses: the analyses would include inventory of the corridor land uses, pedestrians and bicyclists needs, safety conditions (crashes involving vehicles, pedestrians and bicyclists), traffic signal equipment essentials, peak hour traffic operations analyses, and spot speed survey.
- Forecast future traffic: Use the regional travel demand model set to forecast 2040 traffic. The model was calibrated for 101 cities and towns in the MPO region and adopted for the Long-Range Transportation Plan.
- Develop and analyze improvements: Work in conjunction with the study task force to develop improvements and concepts that would reconfigure the roadways to improve safety and operations and make the roadways safer, convenient, and comfortable access for all users.
- Document the study and present products of the tasks to the advisory task force for comment and feedback. Prepare draft document for review and finalize report

Seth Asante said the study is expected to be completed in 12 month. Mark Abbott thanked the advisory force members for their participations and welcomed any suggestions or comments after the meeting via e-mails or phone calls.

Meeting was adjourned at 1:00 P.M.

Route 1A-Vinnin Square Priority Corridor Study

Thursday, May 5, 2016 12:00 AM—1:00 PM Swampscott Town Hall 22 Monument Avenue First Floor Conference Room

Project Team Members

 Stacey Beuttell, WalkBoston Ben Wood, MPH 3. Steve Dibble. City of Salem MassDOT Clark, Michael Connie Raphael MassDOT Peter Kane Town of Swampscott Thomas Younger Town of Swampscott 8. Tom Daniel City of Salem Becky Curran Town of Marblehead 40. Dominick Pangallo City of Salem 11. Gino Cresta Town of Swampscott 12. Pounds, Bryan MassDOT John Gregg MassDOT 14. Sara Timoner MassDOT 15. John Pelletier, Mass-In-Motion 16. Mark Abbott CTPS 17. Scott Peterson **CTPS** 18. Seth Asante CTPS

sbeuttell@walkboston.org Ben.Wood@MassMail.State.MA.US sdibble@salem.com michael.clark@state.ma.us connie.raphael@state.ma.us pkane@town.swampscott.ma.us; tyounger@town.swampscott.ma.us tdaniel@Salem.com rebeccac@marblehead.org dpangallo@Salem.com gcresta@town.swampscott.ma.us bryan.pounds@state.ma.us John.Gregg@state.ma.us sara.timoner@state.ma.us John pelletier@harvard.edu mabbott@ctps.org speterson@ctps.org sasante@ctps.org

Route 1A-Vinnin Square Priority Corridor Study in Marblehead, Salem, and Swampscott

Presentation and Discussion of Existing Conditions and Improvements Salem City Hall Annex, 120 Washington Street Third Floor Large Conference Room

October 24, 2016 Meeting Summary

Meeting started at 10:30 A.M.

Participants from the City of Salem, Towns of Marblehead and Swampscott, MassDOT Office of Transportation Planning (OTP), MassDOT Highway Division's District 4 Office, and the Central Transportation Planning Staff (CTPS) introduced themselves (see attached meeting roster).

Study Background

Mark Abbott of CTPS introduced the study and the objectives and informed participants that CTPS went through an extensive and comprehensive process to identify safety, mobility, access, and other transportation-related problems in the corridor. Mark said that CTPS staff has developed multimodal solutions to the problems.

Existing Conditions

Seth Asante of CTPS presented the data collected for the evaluating the existing conditions. They include traffic volumes, spot speeds, crashes, and transit performance data. Seth Asante said that based on analysis of existing conditions, field reconnaissance, and input from the previous meeting the following problems were identified in the study area:

- Wide roadways, which creates inequity by placing too much emphasis on vehicular use and encourages higher vehicle speeds.
- A lack of shoulders or bike lanes makes the roadways uncomfortable for bicyclists and places the sidewalks close to the travel lanes.
- A lack of crosswalks at some major intersections and side streets challenges pedestrians and put them at risk.
- Non-compliant ADA curb ramps and sidewalk connectivity problems (gaps)
 create an unfriendly environment for pedestrians and for people with disabilities.
- A lack of bus shelters at the stops with high number of rider creates problems for riders, especially during inclement weather.

- High vehicular speeds and acute horizontal curve on Route 1A near Leggs Hill Road results in many crashes.
- Outdated signal-timing plans need to be updated to make the flow of traffic efficient throughout the study area.
- High volumes of traffic on Route 1A and Vinnin Street creates congestion at Vinnin Square and Swampscott Mall area.
- A Lack of turn lanes and traffic queues causes high number of crashes on Route 1A at Swampscott Mall, Vinnin Square, and between Harrison Road and Sumner Road in Salem.
- The numerous driveways at Vinnin Square and a lack of trees and greenery do not provide a welcoming environment for pedestrians and bicyclists and contributes too many crashes.

Improvement Alternatives

Seth Asante provided an overview of the 2040 traffic projections and said that traffic on the area's roadways are expected to grow by about five percent between 2016 and 2040. Seth Asante presented the multimodal improvements that CTPS staff developed to address the problems. He said CTPS staff will work with the advisory task force and use their feedback to refine the improvements. Seth Asante said that most of the improvements and concepts fall within the existing roadways right-of-way width and they require no land takings and they would make the study area's roadways safer and more attractive to pedestrians and bicyclists while serving the needs of commuters, supporting economic activities, and livable communities. Seth Asante said a few of the improvements and concepts would require more space to build the improvements and they would involve land takings.

Comments and Feedback

There was a discussion after the presentation and the task force provided feedback including adding an alternative with a median for the Route 1A segment at the Swampscott Mall, converting shoulders into bike lanes, and refining the land use map. Seth Asante thanked the advisory task force members for their participation in the study and welcomed any suggestions or comments after the meeting via e-mails or phone calls.

Meeting was adjourned at 12:00 PM.

Route 1A-Vinnin Square Priority Corridor Study Monday, October 24, 2016 10:30 AM-12:00 PM City Hall Annex, 120 Washington Street 3rd Floor Large Conference Room, Salem

Project Team Members

1. Stacey Beuttell, WalkBoston Ben Wood, MPH 3. Steve Dibble, City of Salem 4. Clark, Michael MassDOT 5. Connie Raphael MassDOT 6. Peter Kane Town of Swampscott 7. Tom Daniel City of Salem V8. Becky Curran Town of Marblehead 9. Dominick Pangallo City of Salem Town of Swampscott 10. Gino Cresta 11. Pounds, Bryan MassDOT 12. John Gregg MassDOT 13. Sara Timoner MassDOT 14. John Pelletier, Mass-In-Motion 15. Mark Abbott CTPS 16. Scott Peterson CTPS 17. Seth Asante CTPS

sbeuttell@walkboston.org Ben.Wood@MassMail.State.MA.US sdibble@salem.com michael.clark@state.ma.us connie.raphael@state.ma.us pkane@town.swampscott.ma.us; tdaniel@Salem.com rebeccac@marblehead.org dpangallo@Salem.com gcresta@town.swampscott.ma.us bryan.pounds@state.ma.us John.Gregg@state.ma.us sara.timoner@state.ma.us John pelletier@harvard.edu mabbott@ctps.org speterson@ctps.org sasante@ctps.org

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Seth Asante

From: Clark, Michael (DOT)

Sent: Tuesday, October 25, 2016 3:00 PM

To: Seth Asante

Cc: Mark Abbott; Pounds, Bryan (DOT)

Subject: Comments on CTPS Vinnin Square study

Hi Seth and Mark,

Please find below OTP's comments on the Vinnin Square study:

- Route 1A is mislabeled on some of the introductory figures (follows Paradise Rd., not Essex St.)
- We encourage you to not treat the parking allowance at the medical facility on Paradise Rd. in Swampscott (near Oakledge Rd. and Franklin Ave.) as a constraint for improvement concepts. Any changes to that allowance can be considered by the state at a later date. A crosswalk should be considered across Paradise at Oakledge Rd. to accommodate individuals parked on that side street.
- Ensure report includes recommendations from projects and studies detailed on Figure 13
- On Figure 18, "traffic congestion and queues at signalized intersections" is putting it a bit strongly for the Loring Ave. segment in Salem (red). Of the three intersections and three peak periods (nine instances), only one instance is LOS D the rest are A or B. Congestion and queues seems to be incongruent with the other problems identified.
- Tie the new sidewalk at the Loring Street curve, the curve warning signs, and the reduced speed limits with explicit desire to improve safety at this location.
- Consider third leg of crosswalk at Harrison Rd. given future site of an elementary school. Pedestrians from Lincoln Rd. would need to cross Harrison Rd. to access school if using sole crosswalk across Loring Ave.
- Strongly favor Alternative 1 for Paradise Rd. at Swampscott Mall. Alternatives 2-4 do not have any bicycle accommodations.
- Show existing crosswalk at SE leg of traffic island at intersection of Vinnin St. and Paradise Rd. in all alternatives.
- So why no bicycle provisions proposed for stretch of Paradise Rd. at Vinnin Square? If unable to reduce from 4
 traffic lanes given need for multiple lanes at intersections demonstrate in report. No bicycle provisions here
 breaks up consistency in bike lanes along Loring Ave. in Salem and Paradise Rd. in Swampscott.
- Similarly, Loring Ave. at Vinnin Square also does not show bicycle provisions despite there seeming to be some space in ROW. This rendering should show existing parking and evaluate future concepts. Understand the high number of curb cuts complicates things here but the three legs of the Vinnin Square triangle on Figure 28 don't seem to have been developed to the same scrutiny as the other segments.
- Good job on the examples of different improvements. This is something we had asked for before so wanted to note its inclusion.

And one last note – I noticed Ben Wood from DPH's name on the sign-in list yesterday. How was he included on the study? Just curious – we work with Ben and DPH on other studies and I was surprised to see his name there.

Thanks, Michael

Michael Clark

Corridor Planning Unit – Office of Transportation Planning Massachusetts Department of Transportation
10 Park Plaza, Suite #4150, Boston, MA 02116

Phone: 857-368-8867



TOWN OF SWAMPSCOTT

DIRECTOR OF COMMUNITY DEVELOPMENT

S. PFTFR KANF

ANDREW LEVIN
ASSISTANT TOWN PLANNER

31 October 2016

PLANNING DEPARTMENT

ELIHU THOMSON ADMINISTRATION BUILDING 22 MONUMENT AVENUE, SWAMPSCOTT, MA 01907

Seth Asante Central Transportation Planning Staff Ten Park Plaza, Suite 2150 Boston, MA 02116

RE: Feedback on "Route 1A-Vinnin Square Priority Corridor Study" Initial Feedback

Seth:

Thank you for letting us provide feedback on the initial materials CTPS has developed as part of the Route 1A-Vinnin Square Priority Corridor Study that was presented to Swampscott, Salem, Marblehead, and MassDOT on Monday, October 24.

I've met with DPW, Fire, and Police in Swampscott in order to put together the below comments which we're hoping can be considered as you put together the draft study report. The comments here are broken down based on the Figure #s from the pages presented at the meeting.

- Figure 2 Study Area Map
 - o The portion of Vinnin St between Loring Avenue and Paradise Road was missing its highlight.
 - The 1A marking is displayed on Essex St in Swampscott but should be on Paradise Road in Swampscott.
 This correction needs to be applied on all subsequent maps.
- Figure 3 Roadway Jurisdiction Map
 - o Sunbeam Lane (off of Salem St in Swampscott) should be indicated as "Other" since it's a private road.
- Figure 8 Designated Speed Limit and Summary of Spot Speed by Direction
 - There's a portion of Paradise Road colored for "35 mph". The Town had MassDOT change that to 30 mph.
- Figure 14 General Land Use/Zoning Map
 - There's an area south of the pink "Commercial/Retail" area currently shown as "Park/Recreational" but is actually "Residential" (specifically the lands abutting the roads of Mountwood Road, Parsons Drive, and Alyward Drive (all in Swampscott).
- Figure 25 Proposed Improvements Paradise Road from Ellis Street to Longwood Drive
 - We'd prefer the shoulders to be bike lanes.
 - The Town currently allows (along with MassDOT okay) parking along the north side of the street for 250
 Paradise Road (in front of their building and the parking lots on either side of building). This would have to be balanced against the need for bike lanes.
- Figures 26-28 Proposed Improvements Paradise Road to Swampscott Mall
 - We prefer Option 1 far more than the other two options

- Medians are not a viable alternative due to the restriction they create on general drivers for left turns but also primarily the fire safety limitation it presents.
- o Add a Rectangular Rapid Flashing Beacon at the mid-block pedestrian crossing.
- o Pedestrian crosswalks for all commercial and multi-family driveway egress points on this portion.
- Figure 30 Proposed Improvements Essex Street and Loring Avenue South of Vinnin Street
 - o We'd prefer the shoulders to be bike lanes.

Please let me know if you have any questions about the feedback provided here. We look forward to the next step in this study.

Sincerely,

S. Peter Kane

Director of Community Development

CC: Interim Town Administrator

Seth Asante

From: Becky Curran

Sent: Wednesday, November 16, 2016 5:35 PM

To: Seth Asante (sasante@ctps.org)

Subject: marbelehad comments on MPO Route 1A -Vinnin Square Priority Corridor Study

Attachments: 201611161726.pdf

Hi Seth - I met with the Police Chief, DPW Director and the Town Administrator to go over the plans we have the following comments

In favor of sidewalk on the northside Tedesco from Marblehead into Vinnin Square and geometry changes at Brookhouse Drive.

Salem bridge at Legg's hill road and Loring Ave (small bridge program?) should be rebuilt

Crosswalk at Tedesco should be re located to location where people from the parking lot across Tedesco St. are going. they will cross there should be in most convenient location

The town already put in the Rapid Flashing beacon and crosswalk FB on Tedesco at Legg's Hill road we also and also put in a crosswalk at Legg's hill road. As is suggested in the plan.

Attached are two mark ups one is to show the area marked as Tedesco country club but it is a vacant parcel of land which has a n overlay district in both Marblehead and Salem that allows for a high density development 30 units per acre.

The second mark up show the driveway on Tedesco street into the staples shopping area. This is a difficult place to take a left into or out of there is poor visibility and if it could be widened to create a turning lane.

Along paradise road the town prefers the option of looking at with a median and breaks in between with one lane in each direction to improve pedestrian environment.

Rebecca Curran Cutting
Town Planner
Abbot Hall
188 Washington Street
Marblehead, Massachusetts 01945
781.631.0000 telephone
781.631.8571 fax
rebeccac@marblehead.org

----Original Message-----

From: selectmen@marblehead.org [mailto:selectmen@marblehead.org]

Sent: Wednesday, November 16, 2016 5:27 PM

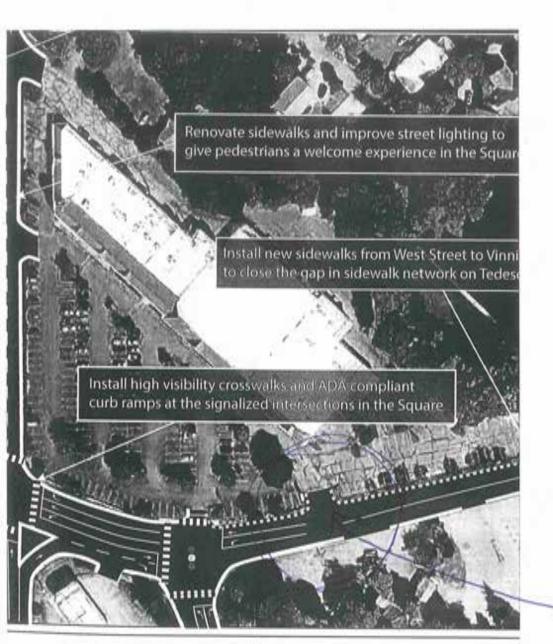
To: Becky Curran

Subject: Message from "RNP0026735B8EF7"

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Route 1A-Vir. Priority Corric Swampscott/Salem/l

Area of conflict people wassering out on taking left int problem with vishility

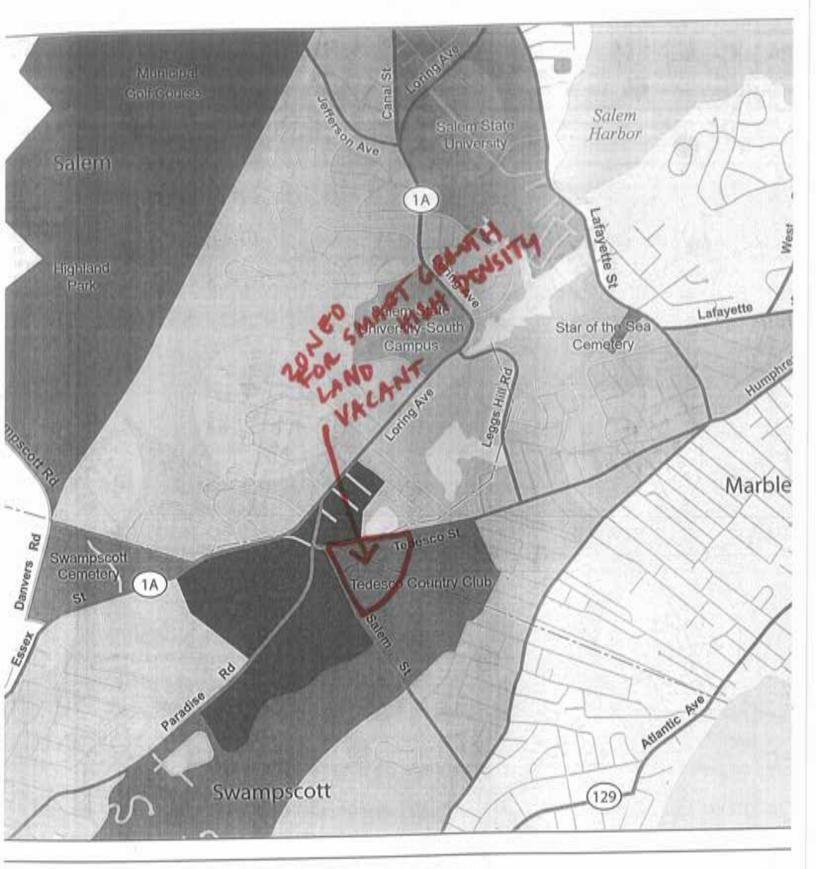




Figure 14
General Land Use/Zoning Map

Seth Asante

From: Raphael, Connie (DOT)

Sent: Thursday, November 03, 2016 2:38 PM

To: Seth Asante

Cc: Timoner, Sara (DOT); Gregg, John (DOT)

Subject: RE: Route 1A-Vinnin Square Priority Corridor Study

Hi Seth,

Thanks for sending the OTP and Town of Swampscott comments. In general we agree with these comments. Here are some specific comments.

- The minimum bike lane width is five feet. Please include this width for all recommendations with bike lanes.
- Sidewalks and bike lanes are required on both sides of the roadway. The District would only support alternatives meeting this criteria. Alternatives for improvements through Vinnin Square should conform to this requirement if possible.
- Try to avoid utility pole relocations when proposing alternatives. Example Where there are existing 5 foot sidewalks we could widen them to six feet to the outside of the existing surface. We would not relocate the poles just to add sidewalk width. Any additional pavement surface should be dedicated to the bike lane/shoulder.
- In areas that are high crash locations the recommendation should be to conduct an Road Safety Audit (RSA). The information in this report could be considered as part of the RSA.

Connie Raphael
District Four Planning Coordinator
MassDOT – Highway Division
519 Appleton Street
Arlington, MA 02476
781-641-8468

From: Seth Asante [mailto:sasante@ctps.org]
Sent: Tuesday, November 01, 2016 10:35 AM

To: Raphael, Connie (DOT); Timoner, Sara (DOT); Gregg, John (DOT)

Subject: Route 1A-Vinnin Square Priority Corridor Study

Hi All,

I wanted to share with you the comments from OTP and Town of Swampscott (attached) so you can review them.

Thank you, Seth

Seth A. Asante | Chief Transportation Planner CENTRAL TRANSPORTATION PLANNING STAFF 857.702.3644 | sasante@ctps.org www.ctps.org/bostonmpo

Ten Pesk Place, St. Yn 2180 - Berner, MA 02216-1986 Na: 'n 187,70018700 - Par 017,8709180 | TTY 017,8709180





Seth Asante

From: Eric Papetti

Sent: Monday, October 31, 2016 7:49 AM

To: sasante@ctps.org
Subject: Vinnin Square Comment

Seth,

Thank you for coming to present the CTPS plan for the Vinnin Square area. I have a few comments below - I am a Salem resident and member of the Parking & Traffic Commission and Bicycle Advisory Committee, but these comments are only my personal observations, I hope that the city will have a chance to weigh in with its own official position soon.

First of all, I appreciate the attention you gave to the importance of lane width in maintaining safe travel speeds, its impact on pedestrian crossing distances, and impact on safety for bicyclists.

Nevertheless, on this and future projects, I encourage CTPS staff to devote analysis and presentation time addressing the criteria which are outlined in its own long-range plan - those of Safety, Preservation, Capacity, Clean Air/Communities, Equity, and Economic Vitality. In the presentation at the city, I heard a lot of discussion of safety, capacity, and preservation of the system, but very little discussion of equity, economic vitality, or clean air & clean communities. Merely designing roadways which adhere to ADA and basic MassDOT guidelines is not adequate - that is a floor, not a ceiling. We need to hear some intense, carefully considered analysis of those criteria and ways to address them, especially considering that 17% of Salem households do not own cars.

I have a few specific recommendations on the plan which I hope you have time to consider, starting with two recommendations relative to the planned move of the Horace Mann school. Safe Routes to School needs to include bicycles, and roadways and intersections near the school should be designed with children as design users, safe enough that they can navigate their way to school independently.

To that end:

Intersection of Lincoln Rd. and Loring - This is a key connection to off-street paths which lead to the Marblehead rail trail and future Canal Street path, and in turn, connects to many quiet, safe, neighborhood roads. Making this crossing safe for kids, so they can enter the school property with having to ride on 1A itself, will effectively open up a huge swath of Salem and Marblehead neighborhoods so that they have safe, convenient access to the new school.

1A north of the new Horace Mann site, to where it connects with Jefferson & Loring reconstruction, should likewise be constructed to the same standard. This will ultimately allow people to make safe connections from many more nearby neighborhoods. This is a good place for the city to consider a protected bike lane.

- On "deadman's curve," pedestrians are currently on the exposed side of the guardrail. This needs to be identified and corrected, preferably while moving bicycle lanes to the protected side of the guardrail as well. Both on deadman's curve and south of it, in the areas where no parking is currently allowed, the appropriate treatment here is definitely a raised, protected bike lane, not just striped shoulders. The state struggles to implement protected bike lanes in areas where parking removal is a political obstacle, so when you have a section of road where that isn't any issue - why not just do it?

- In general, throughout the project area, any shoulders which are intended for bicycle travel should be striped as bike lanes, otherwise people will drive on them.
- In areas throughout the project where roads can be narrowed for safety, communities should consider the benefits of actually removing pavement rather than just re-striping, so as to reduce the burden on city stormwater systems and improve the health of Salem sound through the resultant reduction in non-point-source pollution.

Thanks again for your consideration of these comments.

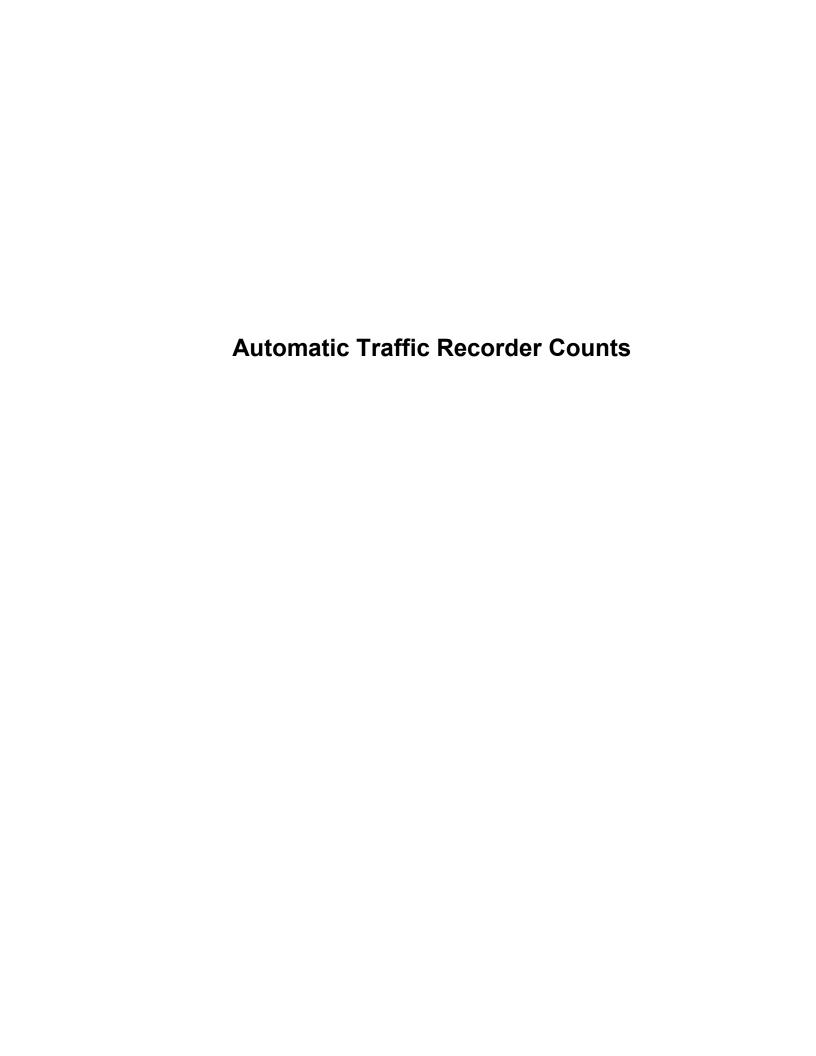
Regards,

Eric Papetti

11 Symonds St., #1 Salem, MA 01970

APPENDIX B

Traffic Data



STA. 1

Site Reference: 160070000795

Site ID: 110000000101

Location: RTE. 1 SOUTH OF PARSONS DR.

Direction: ROAD TOTAL

TOTA L

File: SPD1-0102.prn City: VINNIN SQUARE STUDY

County: SPEED N&S

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Page: 1

STA. INB

Site Reference: 160070000795

Site ID: 110000000101

Location: RTE. 1 SOUTH OF PARSONS DR. Direction: NORTH

File: SPD1-0102.prn City: VINNIN SQUARE STUDY County: SPEED N&S

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				693					620	
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						685			685	2056
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PM Times	18:00	18:00	18:00			18:00			18:00	
PM Peaks	868	861	876			868			868	

Page: 2

STA. 158

Site Reference: 160070000795

Site ID: 1100000000101

Location: RTE, 1 SCUTH OF PARSONS DR.

Direction: SOUTH

File: SPD1-0102.prn City: VINNIN SQUARE STUDY County: SPEED N&S

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PM Times 18:00 16:00 16:00 18:00 18:00	
PM Peaks 704 683 715 698 698	

STA. 2 NB

Site Reference: 160070000873

Site 1D: 110000000201

Location: RTE. 1A SOUTH OF LEGGS HILL RD.

Direction: NORTH

File: SPD-202.prn City: VINNIN SQUARE STUDY County: SPEED NB

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05:00		33	37	21		30			30	91
06:00		141	138	145		141			141	424
07:00		355	341	342		346			346	1030
08:00		718	737	744		733			733	2199
09:00		665	707	715		695			695	2087
10:00		588	618	570		592			592	1776
11:00		545	556			550			550	1101
12:00		536	595			565			565	1131
13:00		606	622			614			614	1228
14:00	608		631			606			606	1818
15:00	638	645	609			630			630	1892
16:00	717	666	681			688			688	2064
		647	703			677			677	2032
18:00	701	817	740			752			752	2258
19:00		655	718			687			687	2061
20:00	521	486	558			521			521	1565
21:00	393 297	371	395			383			383	1149
22:00	287	336	306			309			309	929
	190					190			190	572
24:00	105	126	136			122			122	367
TOTALS	5520	9803	10160	2680	0	9957	0	0	9957	28163
% AVG WKDY	55.4	98.4	102	26.9						
& AVG WEEK	55.4	98.4	102	26.9						
AM Times		08:00	08:00	08:00		08:00			08:00	
AM Peaks		718	737	744		733			733	
PM Times	16:00	18:00	18:00			18:00			18:00	
PM Peaks	717	817	740			752			752	

STA. 2 SB NO DATA

5TA.3

Site Reference: 160070000780

Site ID; 110000000301

Location: ESSEX ST., SOUTH OF CAROL WAY Direction: ROAD TOTAL

TOTAL

File: SPD-3-0102.prn City: VINNIN SQUARE STUDY County: SPEED N6S

	11	12	13	THU 14	FRI	WKDAY AVG	SAT	รบท	WEEK AVG	TOTAL
01:00		27	64 19	74 36		65 27			63	195
02:00		28	19	36		21			27	83 59
03:00		19	20	20		19			19	59
04:00		28	22 55	32		27 53			27	82 159
05:00		54	55	50		53			53	159
06:00		253	244	229		242			242	
07:00		664	665	629 1316		652			652 1315	1958
00:00		1309	1322	1316		1315			1315	3947
09:00		1199	1178	1248		1208			1208	3625
10:00		1080	1068	1036		1061			1061	3184
11:00		1054	1076			10 6 5			1065	2130
12:00		1074	1155			1114			1114	2229
13:00		1210	1251			1230			1230 1133	2461
14:00	1086	1097	1216			1133			1133	3399
15:00	12/9	1101	1320			1133			1255	3766
16:00	1341 1457	1233	1286			1286			1286	3860
17:00	1457	1393	1419			1423			1286 1423	4269
18:00	1349	1405	1405			1386			1386	4159
19:00	1225 854 552	1191	1189			1201			1201	3605
20:00	854	874	912			1201 880			1201 880	2640
21:00	552	624	636			604			604	1812
22:00	441	451	504			465			465	1396
23:00	247	281	311			279			774	830
22:00 23:00 24:00	148	144	176			156			156	468
27.00	240	134	1.0			150			130	400
TOTALS	9979	17883	18519	4670	0	18146	0	0	18146	51051
AVG WEEK	54.9	98.5	102	25.7						
AVG WEEK	54.9	98.5	102	25.7						
AM Times		0B:00	08:00	DB:00		08:00			09:00	
AM Peaks		1309	1322	1316		1315			1315	
PM Times	17:00	18:00	17:00			17:00			17:00	
PM Peaks	1457	1405	1419			1423			1423	
D%	50 15	55	50	50						
K%	15	8	8	28						

45 COMB AWD 18146 FAC , 93 COMB ADT 16,900

MassDOT Highway Division WEEKLY SUMMARY FGR LANE 1 Starting: 4/11/2016

STA. 3 NB

Site Reference: 160070000780

Site ID: 110000000301

Location: ESSEX ST., SOUTH OF CAROL WAY

Direction: NORTH

File: SPD-3-0102.prn City: VINNIN SQUARE STUDY County: SPEED N&S

Page: 1

TIME	MUN 11	12	ሎይሀ 13	14	FRI	WKUAY AVS	SAT	5 UN	WEEK AVG	LATUT
01:00		29	36	64		36			36	109
02:00		15	6 11	16 1€		13			13	39
03:00		10	11	1€		10			19	31
04:00			10	18		13			13	39
05:00				24		24			24	73
06:00		100		95		13C			100	30C
07:00		294 621	273	271		279			279	938
08:00		621	668	669		£52			652	195B
09:00		596	605	655		618			518	1956
10:00		520		501		508			soa	1526
11:00		522	534			528			528	1356
12:00		528	559			548			548	1097
13:00		643	641			612			642	1294
14:00	558	529	604			563			563	1691
15:00	665	€11	703			659			653	
	689					649			649	1947
17:00	738	696	723			719			719	2157
18:00	753	796	759			769			769	2307
19:00	753 682	672	690			681			681	2044
20:00						511			511	1535
	326					345				1037
	230					253				751
23:00	135	154	159			149			1.40	448
24:00	72	154 75	69			19			79	237
										201
TOTALS	5356	9149	9541	2303	Ú	9348	D	0	9348	26349
% AVG WKDY	57.2	97.8	102	24.6						
\$ AVG WEEK	57.2	97.8	102	24.6						
AM Times		06:00	09:00	C8:C0		08:00			09:00	
AM Peaks				669		652			652	
PK Times	18:00	18:00	18:00			18:00			13:00	
2K Peaks						759			769	

MassDOT Highway Division WEEXLY SUMMARY FOR LANE 2 Starting: 4/11/2016

STA . 3 SB

Site Reference: 160070000780

Site ID: 110000000301

Location: ESSEX ST., SOUTH OF CAROL WAY

Direction: SOUTH

File: SPD-3-0102.prn City: VINNIN SQUARE STUDY

Page: 2

County: SPEED N&S

BKIT		TUE 12		TKU 14	FRI	WKDAY AVG	SAT	WEEK AVG	
01:00		28	28	30		28		20	86
G2:D0		13	11	20		14		14	44
03:00		9	3 11 9	30 20 10		28 14 9		20 14 9	28
04:00		17	12	14		- 14		24	43
05:00		32	28 147	26		28		28	86
06:00		145	147	134		142		142	426
07::00		370	392	35B		373		373	1120
08:00		688		647		663		663	1989
09:00		693	573	593		589		589	
10:00		560	563	535		552		552	1658
11:00		532	542			537		537	
12:00		546	586			566		566	
13:00		567				500		588	
14:00	528	568	612			569		569	1708
15:00	614	550 615	623			595		595	1787
16:00	652	615	646			637		637	
	719		696			704		704	
	596		637			617		617	
19:00	543	519	499			520		520	1561
20:00	346	386	373			368		368	1105
21:00	226	278	271			258			775
22:00	211	197	227			211		211	
23:00	112	127	152			130		130	391
24:00	76	ρţ	87			77		90 77	231
TOTALS				2367					
# AVG WKOY	52.5	99.3	102.1	26.9					
8 AVG WEEK	52.5	99.3	102.1	26.9					
AM Times		08:00	08:00	08:00				08:00	
AM Peaks		688	654	647		663		663	
PM Times	17:00	17:00	17:00			17:00		17:00	
PM Peaks	719	697	696			704		704	

Page: 3

STA. 4

Site Reference: 160070000479

Site ID: 110000000401

Location: SALEM ST., SOUTH OF VINNIN ST. Direction: ROAD TOTAL

TOTAL

File: SPD-4-0102.prn City: VINNIN SQUARE STUDY

County: SPEED N&S

TIME	MON 11		13		FRI	WKDAY AVG	SAT	SUN	WEEK AVG	TOTAL
15:00 16:00 17:00 18:00 19:00 20:00 21:00 22:00 23:00	405 517 513 463 437 414 301 181 111 48	7 8 2 1 7 3 2 1 3 2 1 4 1 9 9 3 7 1 9 4 5 7 1 9 4 5 7 1 9 4 5 7 1 9 4 7 1 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	11 49 57 48 141 440 438 416 496 496 496 496 496 496 496 496 496 49	14 13 3 3		10 8 4 3 11 137 435 438 396 393 504 498 477 476 477 4712 203 127 29			434 383 396 393 504 498 476 477 476 477 412	9 35 412 1305 1304 1149 7927 1009 1285 1428 1429 1429 1429 1429 611
TOTALS				1531						17462
% AVG WKDY % AVG WEEK	54.8 54.8	95 95	105.2 105.2	24.5 24.5						
		08:00 419		09:00 466		08:00 435			08:00 435	
PM Times PM Peaks	15:00 517	13:00 519	15:00 584			13:00 504			13:00 504	
D% K%	50 15	55 9	55 9	SS 30						

45 COMB AWD 6244 FAC .93 COMB APT 5,800

MassDOT Highway Division WEEKLY SUMMARY FOR LANE 1 Starting: 4/11/2016

Page: 1

5TA . 4 NB

Site Reference: 160070000479

Site ID: 110000000401

Location: SALEM ST., SOUTH OF VINNIN ST. Direction: NORTH

File: SPD-4-0102.prn City: VINNIN SQUARE STUDY County: SPEED N65

TIME	11	12	WED 13	14	FRI	WKDAY AVG	SAT	SUN	WEEK AVG	TOTAL
01:00			5 3 5 3	12		5			5	15
02:00		3	3	3		4				2.2
03:00		ī	5	ō		2			2 2	6
04:00		1	3	3		2			2	7
05:00		11	5	8		В			8	24
06:00		1.8	28	24		23			23	70
07:00			78	77		76			76	229
08:00		222	227	234		227			227	683
09:00		210	219	217		215			215	646
10:00		168	186	176		176			176	530
11:00		183	197			190			190	38D
12:00		174	188			181			181	362
13:00	165 251 247	242	220			231			231	462
14:00	165	184	232			193			193	561
15:00	251	185	276			237			237	712
16:00	217	188	243			226			226	678
17:00		206	229			209			209	627
18:00	198	213 187	246			215		83	215	647
19:00	185	187	183			185			185	555
20:00	126	116	140			127			195	382
21:00	92	101	62			88			88	265
22:00	42	59	50			50				151
23:00	20 7	13	31			21			21	64 23
24:00	7	10	6			7			7	23
TOTALS						2898			2898	
& AUG WKDY	51.9	95.6	106.3	25 9						
# AVG WEEK	51.9	95.6	106.3	25.9						
AM Times		00:00	08:00	08:00		08:00				
AM Feaks		222	227	234		227			227	
PM Times									15:00	
PM Peaks	251	242	276			237			237	

STA. 4 SB

Site Reference: 160070000479

Site ID: 110000000401 Location: SALEM ST., SOUTH OF VINNIN ST. Direction: SCUTH

File: SPD-4-0102.prn City: VINNIN SQUARE STUDY County: SPEED N&S

TIME	MON 11	TUE 12	13	3.4	FRI	WKDAY AVG			WEEK AVG	
15:00 16:00 17:00 18:00 19:00	240 266 266 271 249 229 175	4 5 1 0 6 14 57 197 203 186 197 277 200 210 233 245 259 231 183	6 1 9 2 20 63 213 220 226 226 228 270 264 300 251 289 274 221	7 6		5 4 2 0 3 18 61 207 219 206 212 273 234 261 250 266 267 79 115			5 4 2 0 3 18 207 219 206 212 273 261 258 260 227 115	17 12 2 15 183 183 183 183 183 183 183 183 183 183
22:00 23:00 24:00	69 28 28	29 22	80 35 16			76 30 22			76 30 22	230 92 66
TOTALS NAVG WKDY AVG WEEK	57.5	94.7	104.5	23.3	Đ	3338	۵	Q	3338	9350
AM Times AM Peaks		10:00 203	12:00 228	09:00 249		09:00 219			09:00 219	
PM Times PM Peaks	17:00 271	13:00 — 277	15:00 308			13:00 273			13:00 -273	

STA . 5

Site Reference: 160070000758

Site ID: 110000000501

Location: TEDESCO ST., WEST OF WEST ST. Direction: ROAD TOTAL

TOTAL

File: SPD-5-0102.prn City: VINNIN SQUARE STUDY

County: SPEED N&S

	11	12	13	THU 14	WKDAY AVG	ииг	WEEK AVG	JATOT
01:00 02:00 03:00 04:00 05:00 06:00 07:00 08:00 09:00 10:00 11:00 12:00 13:00 14:00 15:00 16:00 17:00 18:00 19:00 20:00	1029 1080 1256 1192 1250 1082 768 474	27 10 10 6 44 181 693 1213 1156 962 954 912 1024 1130 1234 1236 1141 771 508	29 9 47 38 185 674 1181 1097 954 1062 11567 11567 1269 1364 1364 1379	32 17 6 11 52	990 1028 1022 1085 1184 1238 1294 1129 791		12 6 8 44 177 650 1199 1114 947 953 1022 1085 11294 1239 1229 791 520	531 1980 3598 3343 2842 1906 1981 2056 3058 3256 3553 3715 3887 2373 1561
24:00	299 111 58	62	66		302 127 62		302 127 62	186
TOTALS								
AVG WEEK	54 54	98.3 98.3	103.2 103.2	25.8 25.8				
AM Times AM Peaks		08:00 1213	08:00 1181	08:00 1204	08:00 1199		08:00 1199	
PM Times PM Peaks	16:00 1256	18:00 1269	18:00 1364		10:00 1294		18:00 1294	
D% K%	50 15	60 8	55 B	60 29				

U3 "

COMB AWD 15921 FAC ,94 COMB APT 15,000

MassDUT Highway Division WEEKLY SUMMARY FOR LANE 1 Starting: 4/11/2016

Page: 1

STA. 5 NB

Site Reference: 160070000758

Site ID: 110000000501 Location: TEDESCO ST., WEST OF WEST ST. Direction: NORTH

File: SPD-5-0102.prn City: VINNIN SQUARE STUDY County: SPEED N&S

TIME	MON 11	12	13	14	 WKDAY AVG	SAT	SUN	WEEK AVG	TOTA
01:00		13	12	16	13			13	41
02:00		4 5 1	0	7 3	13 4 2			2 2	13
03:00		5	0	3	2			2	8
04:00		1	Z	3 23	2			2	6
05:00		22	21	23	22			22	
06:00		114	122	116	117			117	352
07:00		492	489	444	475			475	1425
09:00		780	710	716	735			735	2206
09:00		674		642	657			657	1972
10:00		558	544	538	546			546	1640
.11:00		546	541		543			543	1087
12:00		501	523		512			512	1024
13:00		492	536		514			514	
14:00	525	431	531		495			495	1487
15:00	500		552		517			517	1551
16:00	637	573	576		595			595	1786
17:00	542		632		576			576	
18:00	535	507			550			550	
19:00	404	450	473		442			442	
	297	315			306				920
21:00	185	228			214				643
22:00	132	117	125		124			124	
23:00	49	117 51	67		55			55	166
24:00	22	29	26		55 25				77
TOTALS	3827	7957	8288	2508	8041	0	0	8041	22580
N AVG WKDY	47 6	60 0	103	21 1					
& AVG NEEK	47.5	98.9	103	31.1					
AM Times		08:00	08400	08-00	08:00			08:00	
AM Peaks		780	710	716				735	
PM Times	16:00	16:00	17:00		16:00			16:00	
PM Peaks	637	573	632		595			595	

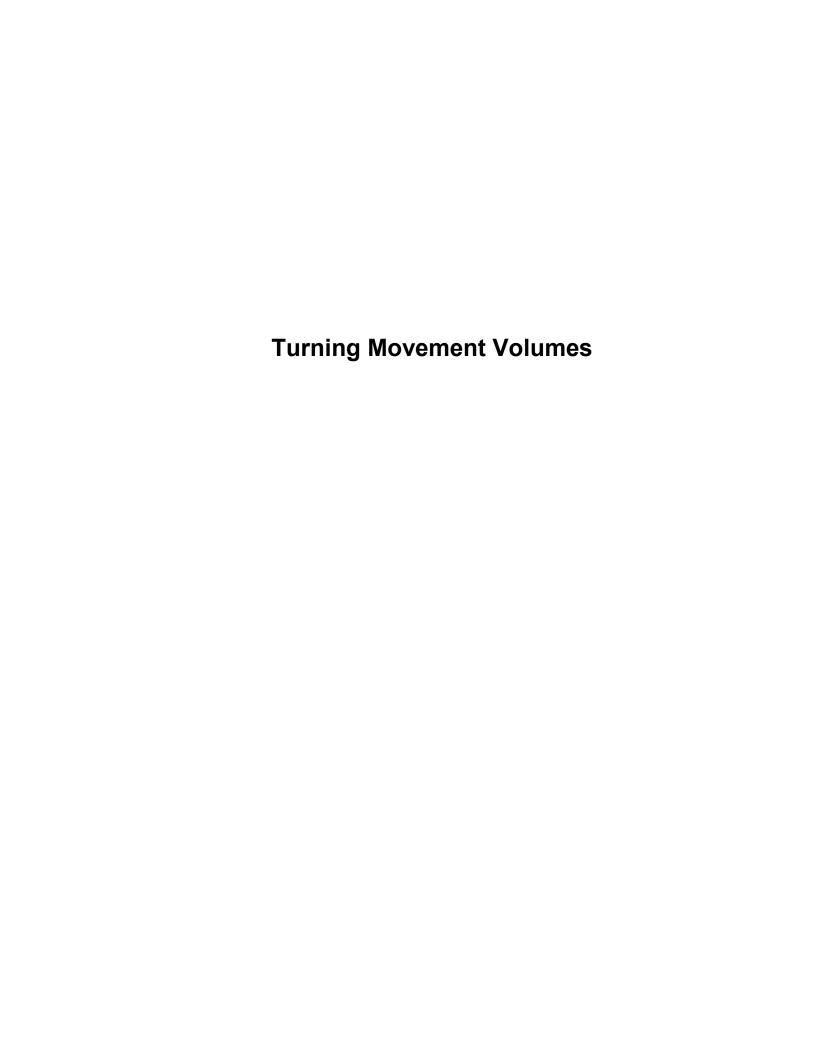
STA.55B

Site Reference: 160070000758

Site ID: 110000000501 Location: TEDESCO ST., WEST OF WEST ST. Direction: SOUTH

File: SPD-5-0102.prn City: VINNIN SQUARE STUDY County: SPEED NAS

∃MIT	MON 11	TUE 12	WED 13	THU 14	FRI	WKDAY AVG		SUN	WEEK AVG	TOTAL
01:00		14	17	16		15			15	47
02:00		6	7	10		3			7	23
03:00		5	4 5	3		4			4	12
04:00		7	5	8		6			- 6	20
05:00		22	17	29		22			22	68
06:00			63	49		59			59	
07:00		201	105	169		185			185	
08:00		433	471	488		464			4 5 4	1392
09:00		482	441	448		457			457	
10:00		404	410	388		400			400	
11:00		406	413			409			409	
12:00		443	514			478			478	
13:00		502	526			514			514	1028
14:00	504		596			527			527	
15:00	580	525	600			568			56B	
16:00	619	557	591			589			589	1767
17:00	650		657			662			662	1986
18:00	715 678	762	754			743			743	
19:00	678	691	691			686			686	2060
	471		526			484			484	1453
21:00	289	280	349			306				918
22:00 23:00	167	. 180	186			177			177	533
23:00	63	69	84			72				216
24:00	36	33	40			36			36	109
TOTALS				1608		7870			7870	
1 AVG WKDY	60 6	D7 D	102.5	20.4						
AVG WEEK	60.6	97.9	103.5	20.4						
AM Times		09:00	12:00	08:00		12:00			12:00	
AM Peaks			514			478			478	
PM Times	18:00	18:00	18:00			18:00			18:00	
PM Peaks						743	82		743	



Study Name Swampscott - Route 1A and Ellis Road TMC # 1 TMC
Start Date Saturday, April 09, 2016 12:00 PM
Tuesday, April 12, 2016 6:00 PM
Site Code

			9	Southwe	stboun	ıd			1	Northwe	stbour	ıd			ı	Northea	stboun	ıd			:	Southea	stbour	ıd				Crosswalk	
Time Period	Class.	R	Т	L	U	1	0	R	Т	L	U	1	0	R	Т	L	U	1	0	R	Т	L	U	1	0	Total		Pedestrians	Total
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	NE	21	21
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
7:00 AM - 9:00 AM	Cars	29	608	3	0	640	396	7	42	12	0	61	46	5	368	4	0	377	624	4	38	21	0	63	75	1141	SE	10	10
One Hour Peak	%	94%	92%	60%	0%	92%	89%	100%	95%	86%	0%	94%	79%	63%	88%	80%	0%	88%	92%	100%	84%	95%	0%	89%	94%	90%		100%	
7:30 AM - 8:30 AM	Light Goods Vehicles	2	37	2	0	41	31	0	0	2	0	2	12	3	30	1	0	34	39	0	7	1	0	8	3	85	SW	12	12
	%	6%	6%	40%	0%	6%	7%	0%	0%	14%	0%	3%	21%	38%	7%	20%	0%	8%	6%	0%	16%	5%	0%	11%	4%	7%		100%	
	Buses	0	7	0	0	7	6	0	0	0	0	0	0	0	6	0	0	6	7	0	0	0	0	0	0	13	NW	3	3
	%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%		100%	
	Single-Unit Trucks	0	4	0	0	4	9	0	0	0	0	0	0	0	9	0	0	9	4	0	0	0	0	0	0	13		46	46
	%	0%	1%	0%	0%	1%	2%	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	2%	1%	0%	0%	0%	0%	0%	0%	1%			
	Articulated Trucks	0	2	0	0	2	4	0	0	0	0	0	0	0	4	0	0	4	2	0	0	0	0	0	0	6			
	%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	0	1	0	0	1	0	0	2	0	0	2	0	0	0	0	0	0	1	0	0	0	0	0	2	3			
	%	0%	0%	0%	0%	0%	0%	0%	5%	0%	0%	3%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	3%	0%			
	Total	31	659	5	0	695	446	7	44	14	0	65	58	8	417	5	0	430	677	4	45	22	0	71	80	1261			
	PHF	0.6	0.98	0.31	0	0.97	0.94	0.58	0.42	0.7	0	0.48	0.72	0.67	0.93	0.62	0	0.92	0.96	0.33	0.62	0.61	0	0.89	0.56	0.91			
	Approach %					55%	35%					5%	5%					34%	54%					6%	6%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	NE	15	15
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
4:00 PM - 6:00 PM	Cars	15	386	4	0	405	617	2	8	1	0	11	34	11	589	2	0	602	389	2	19	26	0	47	25	1065	SE	0	0
One Hour Peak	%	100%	92%	80%	0%	92%	94%	100%	100%	100%	0%	100%	89%	85%	94%	100%	0%	94%	92%	100%	95%	93%	0%	94%	100%	93%		0%	
5:00 PM - 6:00 PM	Light Goods Vehicles	0	25	0	0	25	32	0	0	0	0	0	3	2	30	0	0	32	25	0	1	2	0	3	0	60	SW	1	1
	%	0%	6%	0%	0%	6%	5%	0%	0%	0%	0%	0%	8%	15%	5%	0%	0%	5%	6%	0%	5%	7%	0%	6%	0%	5%		100%	
	Buses	0	7	0	0	7	3	0	0	0	0	0	0	0	3	0	0	3	7	0	0	0	0	0	0	10	NW	0	0
	%	0%	2%	0%	0%	2%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	0%	0%	0%	0%	1%		0%	
	Single-Unit Trucks	0	2	0	0	2	3	0	0	0	0	0	0	0	3	0	0	3	2	0	0	0	0	0	0	5		16	16
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	0	0	1	0	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1			
	%	0%	0%	20%	0%	0%	0%	0%	0%	0%	0%	0%	3%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	15	420	5	0	440	655	2	8	1	0	11	38	13	625	2	0	640	423	2	20	28	0	50	25	1141			
	PHF	0.75	0.91	0.62	0	0.92	0.86	0.5	0.67	0.25	0	0.69	0.73	0.65	0.85	0.5	0	0.86	0.91	0.5	0.71	0.64	0	0.69	0.89	0.93			
	Approach %					39%	57%					1%	3%					56%	37%					4%	2%				

Study Name Swampscott - Route 1A and Ellis Road TMC # 1 TMC
Start Date Saturday, April 09, 2016 12:00 PM
End Date Tuesday, April 12, 2016 6:00 PM
Site Code

			S	outhw	estbour	nd			1	Northwe	estboun	nd				Northea	stboun	nd			:	Southe	astboun	d				Crosswalk	
Time Period	Class.	R	T	L	U	- 1	0	R	Т	L	U	- 1	0	R	Т	L	U	1	0	R	T	L	U	- 1	0	Total		Pedestrians	Total
Peak 1	Motorcycles	0	0	0	0	0	5	0	0	0	0	0	0	0	5	0	0	5	0	0	0	0	0	0	0	5	NE	2	2
Specified Period	%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	1%		100%	
12:00 PM - 2:00 PM	Cars	8	422	0	0	430	426	1	5	0	0	6	5	0	407	1	0	408	425	3	5	18	1	27	15	871	SE	2	2
One Hour Peak	%	100%	91%	0%	0%	91%	93%	100%	100%	0%	0%	100%	63%	0%	93%	100%	0%	93%	91%	100%	71%	95%	100%	90%	100%	92%		100%	
12:00 PM - 1:00 PM	Light Goods Vehicles	0	38	0	0	38	18	0	0	0	0	0	3	1	18	0	0	19	38	0	2	0	0	2	0	59	SW	4	4
	%	0%	8%	0%	0%	8%	4%	0%	0%	0%	0%	0%	38%	100%	4%	0%	0%	4%	8%	0%	29%	0%	0%	7%	0%	6%		100%	
	Buses	0	2	0	0	2	1	0	0	0	0	0	0	0	1	0	0	1	2	0	0	0	0	0	0	3	NW	6	6
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
	Single-Unit Trucks	0	1	0	0	1	7	0	0	0	0	0	0	0	6	0	0	6	1	0	0	1	0	1	0	8		14	14
	%	0%	0%	0%	0%	0%	2%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%	0%	0%	5%	0%	3%	0%	1%			
	Articulated Trucks	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	8	465	0	0	473	457	1	5	0	0	6	8	1	437	1	0	439	468	3	7	19	1	30	15	948			
	PHF	0.4	0.9	0	0	0.91	0.91	0.25	0.62	0	0	0.5	0.67	0.25	0.93	0.25	0	0.93	0.9	0.38	0.88	0.68	0.25	0.75	0.54	0.96			
	Approach %					50%	48%					1%	1%					46%	49%					3%	2%				

Study Name Swampscott - Route 1A and Norfolk Avenue TMC # 2 TMC
Start Date Saturday, April 09, 2016 12:00 PM
End Date Tuesday, April 12, 2016 6:00 PM
Site Code

			Sout	hwestb	ound _			Nort	heastb	ound _			Ea	astbour	nd				Crosswa	alk
Time Period	Class.	BR	T	U	1	0	Т	HL	U	1	0	HR	BL	U	- 1	0	Total		Pedestrians	T
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	NE	13	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
:00 AM - 9:00 AM	Cars	134	613	0	747	456	380	22	0	402	640	27	76	0	103	156	1252	SW	0	
One Hour Peak	%	92%	92%	0%	92%	88%	89%	96%	0%	89%	92%	96%	86%	0%	89%	93%	91%		0%	
:30 AM - 8:30 AM	Light Goods Vehicles	7	39	0	46	41	30	1	0	31	40	1	11	0	12	8	89	W	4	
	%	5%	6%	0%	6%	8%	7%	4%	0%	7%	6%	4%	13%	0%	10%	5%	6%		100%	
	Buses	0	7	0	7	6	6	0	0	6	7	0	0	0	0	0	13		17	
	%	0%	1%	0%	1%	1%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	1%			
	Single-Unit Trucks	3	4	0	7	9	8	0	0	8	4	0	1	0	1	3	16			
	%	2%	1%	0%	1%	2%	2%	0%	0%	2%	1%	0%	1%	0%	1%	2%	1%			
	Articulated Trucks	0	2	0	2	4	4	0	0	4	2	0	0	0	0	0	6			
	%	0%	0%	0%	0%	1%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	1	1	0	2	0	0	0	0	0	1	0	0	0	0	1	2			
	%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%			
	Total	145	666	0	811	516	428	23	0	451	694	28	88	0	116	168	1378			
	PHF	0.74	0.97	0	0.93	0.89	0.96	0.64	0	0.93	0.95	0.44	0.67	0	0.59	0.82	0.93			
	Approach %				59%	37%				33%	50%				8%	12%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	NE	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
:00 PM - 6:00 PM	Cars	37	398	0	435	662	609	20	0	629	412	14	53	0	67	57	1131	SW	0	
One Hour Peak	%	88%	93%	0%	92%	95%	95%	95%	0%	95%	93%	100%	96%	0%	97%	90%	94%		0%	
:00 PM - 6:00 PM	Light Goods Vehicles	5	23	0	28	28	26	1	0	27	23	0	2	0	2	6	57	W	1	
	%	12%	5%	0%	6%	4%	4%	5%	0%	4%	5%	0%	4%	0%	3%	10%	5%		100%	
	Buses	0	7	0	7	3	3	0	0	3	7	0	0	0	0	0	10		1	
	%	0%	2%	0%	1%	0%	0%	0%	0%	0%	2%	0%	0%	0%	0%	0%	1%			
	Single-Unit Trucks	0	2	0	2	3	3	0	0	3	2	0	0	0	0	0	5			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Articulated Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	, %	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	42	430	0	472	696	641	21	0	662	444	14	55	0	69	63	1203			
	PHF	0.7	0.94	0	0.94	0.86	0.83	0.75	0	0.85	0.95	0.88	0.62	0	0.69	0.79	0.95			
	Approach %				39%	58%				55%	37%				6%	5%				

Study Name Swampscott - Route 1A and Norfolk Avenue TMC # 2 TMC
Start Date Saturday, April 09, 2016 12:00 PM
End Date Tuesday, April 12, 2016 6:00 PM
Site Code

			Soutl	nwestb	ound			Nor	theastb	ound			E	astbou	nd				Cross	walk
Time Period	Class.	BR	Т	U	1	0	Т	HL	U	- 1	0	HR	BL	U	- 1	0	Total		edestria	Total
Peak 1	Motorcycles	1	0	0	1	5	5	0	0	5	0	0	0	0	0	1	6	NE	5	5
Specified Period	%	2%	0%	0%	0%	1%	1%	0%	0%	1%	0%	0%	0%	0%	0%	1%	1%		100%	
12:00 PM - 2:00 PM	Cars	57	420	0	477	462	421	11	1	433	431	10	41	0	51	68	961	SW	0	0
One Hour Peak	%	88%	91%	0%	91%	93%	94%	92%	100%	94%	91%	71%	80%	0%	78%	88%	92%		0%	
12:00 PM - 1:00 PM	nt Goods Vehi	7	35	0	42	24	14	1	0	15	39	4	10	0	14	8	71	W	3	3
	%	11%	8%	0%	8%	5%	3%	8%	0%	3%	8%	29%	20%	0%	22%	10%	7%		100%	
	Buses	0	2	0	2	1	1	0	0	1	2	0	0	0	0	0	3		8	8
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	ngle-Unit Truc	0	1	0	1	5	5	0	0	5	1	0	0	0	0	0	6			
	%	0%	0%	0%	0%	1%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	1%			
	ticulated Truc	0	1	0	1	1	1	0	0	1	1	0	0	0	0	0	2			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	0	1	0	1	0	0	0	0	0	1	0	0	0	0	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	65	460	0	525	498	447	12	1	460	475	14	51	0	65	77	1050			
	PHF	0.86	0.91	0	0.94	0.86	0.89	0.6	0.25	0.91	0.91	0.7	0.67	0	0.74	0.96	0.94			
	Approach %				50%	47%				44%	45%				6%	7%				
															·					

Study Name Swampscott - Route 1A at Whole Foods Market and Vinnin Liquors TMC # 3 TMC
Start Date Saturday, April 09, 2016 12:00 PM
End Date Tuesday, April 12, 2016 6:00 PM
Site Code

			Soutl	nwestb	ound _			W	estbou	nd			Nort	heastb	ound _				Cross	walk
Time Period	Class.	Т	HL	U	1	0	HR	BL	U	1	0	BR	Т	U	- 1	0	Total		:destria	Tota
Peak 1	Motorcycles	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1	1	NE	1	1
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
7:00 AM - 9:00 AM	Cars	812	37	0	849	662	25	11	0	36	39	2	637	0	639	823	1524	Ε	1	1
One Hour Peak	%	93%	100%	0%	93%	91%	89%	100%	0%	92%	98%	67%	91%	0%	91%	93%	92%		100%	
7:30 AM - 8:30 AM	nt Goods Vehi	43	0	0	43	47	3	0	0	3	0	0	44	0	44	43	90	SW	0	0
	%	5%	0%	0%	5%	6%	11%	0%	0%	8%	0%	0%	6%	0%	6%	5%	5%		0%	
	Buses	9	0	0	9	7	0	0	0	0	0	0	7	0	7	9	16		2	2
	%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%	0%	1%	1%	1%			
	ngle-Unit Truc	9	0	0	9	7	0	0	0	0	0	0	7	0	7	9	16			
	%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%	0%	1%	1%	1%			
	ticulated Truc	2	0	0	2	3	0	0	0	0	1	1	3	0	4	2	6			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	3%	33%	0%	0%	1%	0%	0%			
	icycles on Roa	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	876	37	0	913	726	28	11	0	39	40	3	698	0	701	887	1653			
	PHF	0.94	0.54	0	0.96	0.93	0.58	0.69	0	0.61	0.5	0.25	0.94	0	0.95	0.94	0.97			
	Approach %				55%	44%				2%	2%				42%	54%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	NE	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
4:00 PM - 6:00 PM	Cars	557	137	0	694	881	202	91	0	293	157	20	679	0	699	648	1686	Ε	1	1
One Hour Peak	%	92%	94%	0%	93%	94%	95%	92%	0%	94%	93%	87%	94%	0%	94%	92%	93%		100%	
4:30 PM - 5:30 PM	nt Goods Vehi	40	8	0	48	45	9	8	0	17	10	2	36	0	38	48	103	SW	0	0
	%	7%	6%	0%	6%	5%	4%	8%	0%	5%	6%	9%	5%	0%	5%	7%	6%		0%	
	Buses	6	0	0	6	1	0	0	0	0	0	0	1	0	1	6	7		1	1
	%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%			
	ngle-Unit Truc	1	0	0	1	7	1	0	0	1	1	1	6	0	7	1	9			
	%	0%	0%	0%	0%	1%	0%	0%	0%	0%	1%	4%	1%	0%	1%	0%	0%			
	ticulated Truc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	604	145	0	749	934	212	99	0	311	168	23	722	0	745	703	1805			
	PHF	0.89	0.86	0	0.92	0.96	0.91	0.88	0	0.94	0.86	0.64	0.92	0	0.9	0.9	0.96			
	Approach %				41%	52%				17%	9%				41%	39%				

Study Name Swampscott - Route 1A at Whole Foods Market and Vinnin Liquors TMC # 3 TMC

Start Date Saturday, April 09, 2016 12:00 PM
End Date Tuesday, April 12, 2016 6:00 PM
Site Code

			Soutl	hwestb	ound			w	estbou	nd			Nort	heastb	ound				Cross	walk
Time Period	Class.	Т	HL	U	l I	0	HR	BL	U	l I	0	BR	Т	U	- 1	0	Total		:destria	Tota
Peak 1	Motorcycles	1	0	0	1	7	1	0	0	1	0	0	6	0	6	1	8	NE	1	1
Specified Period	%	0%	0%	0%	0%	1%	1%	0%	0%	0%	0%	0%	1%	0%	1%	0%	0%		100%	
2:00 PM - 2:00 PM	Cars	609	169	0	778	785	187	79	0	266	183	14	598	0	612	688	1656	Ε	8	8
One Hour Peak	%	92%	93%	0%	92%	92%	96%	94%	0%	95%	93%	100%	91%	0%	92%	92%	92%		100%	
2:00 PM - 1:00 PM	nt Goods Vehic	48	12	0	60	49	6	5	0	11	12	0	43	0	43	53	114	SW	0	0
	%	7%	7%	0%	7%	6%	3%	6%	0%	4%	6%	0%	7%	0%	6%	7%	6%		0%	
	Buses	2	0	0	2	1	0	0	0	0	0	0	1	0	1	2	3		9	9
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	ngle-Unit Truc	2	1	0	3	7	1	0	0	1	1	0	6	0	6	2	10			
	%	0%	1%	0%	0%	1%	1%	0%	0%	0%	1%	0%	1%	0%	1%	0%	1%			
	ticulated Truc	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	664	182	0	846	849	195	84	0	279	196	14	654	0	668	748	1793			
	PHF	0.94	0.91	0	0.94	0.94	0.83	0.66	0	0.77	0.92	0.88	0.89	0	0.89	0.92	0.95			
	Approach %				47%	47%				16%	11%				37%	42%				

Study Name Swampscott - Route 1A at Swampscott Mall and Vinnin Square Plaza TMC #4 TMC
Start Date Saturday, April 09, 2016 12:00 PM
Tuesday, April 12, 2016 6:00 PM

Site Code

		•		South	bound					Westb	ound					North	bound					Eastb	ound					Cross	walk
Time Period	Class.	R	Т	L	U	1	0	R	Т	L	U		0	R	Т	L	U		0	R	Т	L	U		0	Total		:destria	
Peak 1	Motorcycles	1	0	0	0	1	1	0	0	1	0	1	0	0	0	0	0	0	1	0	0	1	0	1	1	3	N	0	0
Specified Period	%	1%	0%	0%	0%	0%	0%	0%	0%	2%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	1%	0%	0%		0%	
7:00 AM - 9:00 AM	Cars	126	721	21	0	868	649	11	39	53	0	103	43	7	571	65	0	643	839	65	15	67	0	147	230	1761	Ε	0	0
One Hour Peak	%	88%	92%	95%	0%	92%	91%	85%	95%	93%	0%	93%	96%	88%	92%	90%	0%	92%	93%	96%	100%	89%	0%	93%	89%	92%		0%	
7:30 AM - 8:30 AM	nt Goods Vehi	15	38	1	0	54	42	1	2	3	0	6	2	1	35	4	0	40	44	3	0	6	0	9	21	109	S	1	1
	%	10%	5%	5%	0%	6%	6%	8%	5%	5%	0%	5%	4%	13%	6%	6%	0%	6%	5%	4%	0%	8%	0%	6%	8%	6%		100%	
	Buses	1	9	0	0	10	7	0	0	0	0	0	0	0	7	0	0	7	9	0	0	0	0	0	1	17	W	1	1
	%	1%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%		100%	
	ngle-Unit Truc	0	11	0	0	11	8	1	0	0	0	1	0	0	6	2	0	8	11	0	0	1	0	1	2	21		2	2
	%	0%	1%	0%	0%	1%	1%	8%	0%	0%	0%	1%	0%	0%	1%	3%	0%	1%	1%	0%	0%	1%	0%	1%	1%	1%			
	ticulated Truc	1	2	0	0	3	3	0	0	0	0	0	0	0	3	1	0	4	2	0	0	0	0	0	2	7			
	%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	1%	0%	0%	0%	0%	0%	0%	1%	0%			
	icycles on Roa	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	144	782	22	0	948	710	13	41	57	0	111	45	8	622	72	0	702	907	68	15	75	0	158	257	1919			
	PHF	0.75	0.95	0.79	0	0.96	0.94	0.46	0.73	0.79	0	0.73	0.7	0.67	0.95	0.82	0	0.93	0.96	0.89	0.54	0.82	0	0.84	0.76	0.95			
	Approach %					49%	37%					6%	2%					37%	47%					8%	13%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	3	3
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
4:00 PM - 6:00 PM	Cars	191	454	38	0	683	866	25	87	87	0	199	115	24	654	173	0	851	711	170	53	187	0	410	451	2143	Е	1	1
One Hour Peak	%	93%	92%	97%	0%	92%	95%	100%	100%	97%	0%	99%	91%	89%	94%	96%	0%	94%	93%	93%	88%	95%	0%	94%	95%	94%		100%	
4:30 PM - 5:30 PM	nt Goods Vehi	13	35	1	0	49	41	0	0	3	0	3	11	3	34	8	0	45	48	10	7	7	0	24	21	121	S	0	0
	%	6%	7%	3%	0%	7%	4%	0%	0%	3%	0%	1%	9%	11%	5%	4%	0%	5%	6%	5%	12%	4%	0%	5%	4%	5%		0%	
	Buses	2	4	0	0	6	1	0	0	0	0	0	0	0	1	0	0	1	6	2	0	0	0	2	2	9	W	0	0
	%	1%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%		0%	
	ngle-Unit Truc	0	1	0	0	1	8	0	0	0	0	0	0	0	6	0	0	6	1	0	0	2	0	2	0	9		4	4
	%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%	0%	0%	1%	0%	0%	0%	0%			
	ticulated Truc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	% 1	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	206	494	39	0	739	916	25	87	90	0	202	126	27	695	181	0	903	766	182	60	196	0	438	474	2282			
	PHF	0.9	0.88	0.81	0	0.91	0.97	0.78	0.84	0.78	0	0.87	0.81	0.61	0.94	0.78	0	0.93	0.92	0.91	0.79	0.88	0	0.9	0.85	0.97			
	Approach %					32%	40%					9%	6%					40%	34%					19%	21%				

Study Name Swampscott - Route 1A at Swampscott Mall and Vinnin Square Plaza TMC #4 TMC

Start Date Saturday, April 09, 2016 12:00 PM End Date Tuesday, April 12, 2016 6:00 PM

Site Code

				South	bound					Westl	bound					North	bound					Eastb	ound					Cross	swalk
Time Period	Class.	R	Т	L	U	1	0	R	Т	L	U	ı	0	R	Т	L	U	1	0	R	Т	L	U	1	0	Total		:destria	Total
Peak 1	Motorcycles	1	1	0	0	2	6	0	0	0	0	0	0	0	6	0	0	6	1	0	0	0	0	0	1	8	Ν	12	12
Specified Period	%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
12:00 PM - 2:00 PM	Cars	214	473	52	0	739	725	26	133	113	0	272	186	41	502	175	0	718	783	197	93	197	0	487	522	2216	Ε	4	4
One Hour Peak	%	93%	90%	96%	0%	92%	93%	96%	98%	98%	0%	98%	97%	95%	93%	94%	0%	94%	92%	93%	98%	92%	0%	94%	95%	93%		100%	
12:00 PM - 1:00 PM	nt Goods Vehi	11	44	2	0	57	38	1	3	2	0	6	6	2	23	10	0	35	58	12	2	14	0	28	24	126	S	11	11
	%	5%	8%	4%	0%	7%	5%	4%	2%	2%	0%	2%	3%	5%	4%	5%	0%	5%	7%	6%	2%	7%	0%	5%	4%	5%		100%	
	Buses	0	2	0	0	2	1	0	0	0	0	0	0	0	1	0	0	1	2	0	0	0	0	0	0	3	W	1	1
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
	ngle-Unit Truc	3	2	0	0	5	7	0	0	0	0	0	0	0	5	2	0	7	4	2	0	2	0	4	5	16		28	28
	%	1%	0%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	1%	1%	0%	1%	0%	1%	0%	1%	0%	1%	1%	1%			
	ticulated Truc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	1	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	229	523	54	0	806	777	27	136	115	0	278	192	43	537	187	0	767	850	212	95	213	0	520	552	2371			
	PHF	0.81	0.92	0.84	0	0.99	0.96	0.84	0.81	0.82	0	0.83	0.91	0.83	0.93	0.87	0	0.94	0.93	0.85	0.82	0.93	0	0.88	0.87	0.97			
	Approach %					34%	33%					12%	8%					32%	36%					22%	23%				

Study Name Swampscott - Route 1A and Vinnin Street TMC # 5 TMC

Start Date Saturday, April 09, 2016 12:00 PM

End Date Tuesday, April 12, 2016 6:00 PM

Site Code

				South	bound					Westb	ound					North	bound					Eastb	ound					Cross	walk
Time Period	Class.	R	Т	L	U	1	0	R	Т	L	U	1	0	R	Т	L	U	1	0	R	Т	L	U	1	0	Total		destria	
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
7:00 AM - 9:00 AM	Cars	20	357	60	0	437	451	65	370	300	0	735	484	163	366	31	0	560	697	40	261	20	0	321	421	2053	Е	0	0
One Hour Peak	%	80%	92%	79%	0%	89%	91%	90%	90%	92%	0%	91%	85%	86%	92%	97%	0%	90%	91%	80%	86%	80%	0%	85%	90%	89%		0%	
7:30 AM - 8:30 AM	nt Goods Vehi	3	21	15	0	39	32	4	36	18	0	58	68	19	26	0	0	45	47	8	34	2	0	44	39	186	S	0	0
	%	12%	5%	20%	0%	8%	6%	6%	9%	6%	0%	7%	12%	10%	7%	0%	0%	7%	6%	16%	11%	8%	0%	12%	8%	8%		0%	
	Buses	0	4	0	0	4	4	0	1	5	0	6	4	3	4	0	0	7	9	0	1	0	0	1	1	18	W	2	2
	%	0%	1%	0%	0%	1%	1%	0%	0%	2%	0%	1%	1%	2%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%		100%	
	ngle-Unit Truc	2	6	1	0	9	10	3	3	2	0	8	12	4	4	1	0	9	10	2	7	3	0	12	6	38		2	2
	%	8%	2%	1%	0%	2%	2%	4%	1%	1%	0%	1%	2%	2%	1%	3%	0%	1%	1%	4%	2%	12%	0%	3%	1%	2%			
	ticulated Truc	0	0	0	0	0	0	0	0	2	0	2	0	0	0	0	0	0	2	0	0	0	0	0	0	2			
	%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	25	388	76	0	489	497	72	410	327	0	809	568	189	400	32	0	621	765	50	303	25	0	378	467	2297			
	PHF	0.62	0.95	0.61	0	0.94	0.92	0.86	0.92	0.97	0	0.97	0.89	0.8	0.9	0.8	0	0.92	0.95	0.83	0.88	0.69	0	0.94	0.89	0.97			
	Approach %					21%	22%					35%	25%					27%	33%					16%	20%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	1	1
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
4:00 PM - 6:00 PM	Cars	12	420	92	0	524	504	56	277	203	0	536	783	341	440	29	0	810	675	52	350	8	0	410	318	2280	Е	1	1
One Hour Peak	%	75%	95%	93%	0%	94%	92%	88%	89%	94%	0%	91%	94%	95%	93%	91%	0%	94%	94%	88%	93%	73%	0%	92%	89%	93%		100%	
4:30 PM - 5:30 PM	nt Goods Vehi	4	18	6	0	28	36	8	29	10	0	47	43	15	27	3	0	45	34	6	22	1	0	29	36	149	S	3	3
	%	25%	4%	6%	0%	5%	7%	13%	9%	5%	0%	8%	5%	4%	6%	9%	0%	5%	5%	10%	6%	9%	0%	7%	10%	6%		100%	
	Buses	0	3	1	0	4	2	0	1	3	0	4	2	1	0	0	0	1	6	0	0	2	0	2	1	11	W	3	3
	%	0%	1%	1%	0%	1%	0%	0%	0%	1%	0%	1%	0%	0%	0%	0%	0%	0%	1%	0%	0%	18%	0%	0%	0%	0%		100%	
	ngle-Unit Truc	0	0	0	0	0	5	0	3	1	0	4	5	1	5	0	0	6	2	1	4	0	0	5	3	15		8	8
	%	0%	0%	0%	0%	0%	1%	0%	1%	0%	0%	1%	1%	0%	1%	0%	0%	1%	0%	2%	1%	0%	0%	1%	1%	1%			
	ticulated Truc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	16	441	99	0	556	547	64	310	217	0	591	833	358	472	32	0	862	717	59	376	11	0	446	358	2455			
	PHF	0.5	0.94	0.82	0	0.9	0.86	0.73	0.88	0.89	0	0.95	0.9	0.9	0.84	0.73	0	0.91	0.94	0.82	0.89	0.55	0	0.91	0.84	0.95			
	Approach %					23%	22%					24%	34%					35%	29%					18%	15%				

Study Name Swampscott - Route 1A and Vinnin Street TMC # 5 TMC
Start Date Saturday, April 09, 2016 12:00 PM
Tuesday, April 12, 2016 6:00 PM

Site Code

				South	bound					West	oound					North	bound					Eastb	ound					Cross	swalk
Time Period	Class.	R	Т	L	U	- 1	0	R	Т	L	U	ı	0	R	Т	L	U	1	0	R	Т	L	U	1	0	Total		edestria	Total
Peak 1	Motorcycles	0	1	0	0	1	6	0	0	1	0	1	2	0	6	0	0	6	2	0	2	0	0	2	0	10	N	3	3
Specified Period	%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	1%	0%	0%	1%	0%	0%	1%	0%	0%		100%	
12:00 PM - 2:00 PM	Cars	25	407	87	0	519	481	95	321	258	0	674	613	282	365	38	0	685	732	67	244	21	0	332	384	2210	Ε	3	3
One Hour Peak	%	86%	93%	86%	0%	91%	93%	90%	87%	92%	0%	90%	89%	93%	93%	90%	0%	93%	93%	93%	87%	95%	0%	88%	88%	91%		100%	
12:00 PM - 1:00 PM	nt Goods Vehi	4	26	12	0	42	30	11	41	19	0	71	62	18	18	3	0	39	50	5	32	1	0	38	48	190	S	1	1
	%	14%	6%	12%	0%	7%	6%	10%	11%	7%	0%	9%	9%	6%	5%	7%	0%	5%	6%	7%	11%	5%	0%	10%	11%	8%		100%	
	Buses	0	1	1	0	2	0	0	0	1	0	1	2	1	0	0	0	1	2	0	0	0	0	0	0	4	W	9	9
	%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
	ngle-Unit Truc	0	2	1	0	3	3	0	5	0	0	5	7	3	3	1	0	7	2	0	3	0	0	3	6	18		16	16
	%	0%	0%	1%	0%	1%	1%	0%	1%	0%	0%	1%	1%	1%	1%	2%	0%	1%	0%	0%	1%	0%	0%	1%	1%	1%			
	ticulated Truc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	0	1	0	0	1	0	0	0	0	0	0	1	0	0	0	0	0	1	0	1	0	0	1	0	2			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	29	438	101	0	568	520	106	367	279	0	752	687	304	392	42	0	738	789	72	282	22	0	376	438	2434			
	PHF	0.66	0.96	0.77	0	0.97	0.88	0.95	0.96	0.92	0	0.97	0.96	0.89	0.84	0.66	0	0.89	0.96	0.75	0.89	0.69	0	0.97	0.89	0.97			
	Approach %					23%	21%					31%	28%					30%	32%					15%	18%				

Study Name Swampscott - Route 1A and Loring Avenue TMC # 6 TMC
Start Date Saturday, April 09, 2016 12:00 PM
End Date Tuesday, April 12, 2016 6:00 PM
Site Code

			So	uthbou	nd			No	rthbou	ınd			Nort	heastb	ound				Cross	walk
Time Period	Class.	BR	Т	U	- 1	0	T	HL	U	ı	0	HR	BL	U	ı	0	Total		:destria	Total
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
7:00 AM - 9:00 AM	Cars	169	410	0	579	683	458	6	0	464	424	14	225	0	239	175	1282	S	0	0
One Hour Peak	%	85%	90%	0%	89%	90%	92%	67%	0%	91%	88%	56%	86%	0%	83%	85%	88%		0%	
7:30 AM - 8:30 AM	nt Goods Vehi	16	31	0	47	52	29	2	0	31	35	4	23	0	27	18	105	SW	0	0
	%	8%	7%	0%	7%	7%	6%	22%	0%	6%	7%	16%	9%	0%	9%	9%	7%		0%	
	Buses	5	5	0	10	11	5	0	0	5	5	0	6	0	6	5	21		0	0
	%	3%	1%	0%	2%	1%	1%	0%	0%	1%	1%	0%	2%	0%	2%	2%	1%			
	ngle-Unit Truc	6	10	0	16	15	6	1	0	7	17	7	9	0	16	7	39			
	%	3%	2%	0%	2%	2%	1%	11%	0%	1%	4%	28%	3%	0%	6%	3%	3%			
	ticulated Truc	0	0	0	0	1	1	0	0	1	0	0	0	0	0	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	2	0	0	2	0	0	0	0	0	0	0	0	0	0	2	2			
	%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%			
	Total	198	456	0	654	762	499	9	0	508	481	25	263	0	288	207	1450			
	PHF	0.88	0.89	0	0.95	0.94	0.96	0.75	0	0.96	0.92	0.57	0.9	0	0.86	0.89	0.97			
	Approach %				45%	53%				35%	33%				20%	14%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
4:00 PM - 6:00 PM	Cars	266	492	0	758	802	535	14	0	549	512	20	267	0	287	280	1594	S	2	2
One Hour Peak	%	92%	94%	0%	94%	93%	93%	93%	0%	93%	94%	83%	93%	0%	92%	92%	93%		100%	
5:00 PM - 6:00 PM	nt Goods Vehi	18	22	0	40	49	34	1	0	35	26	4	15	0	19	19	94	SW	2	2
	%	6%	4%	0%	5%	6%	6%	7%	0%	6%	5%	17%	5%	0%	6%	6%	6%		100%	
	Buses	2	6	0	8	2	0	0	0	0	6	0	2	0	2	2	10		4	4
	%	1%	1%	0%	1%	0%	0%	0%	0%	0%	1%	0%	1%	0%	1%	1%	1%			
	ngle-Unit Truc	2	1	0	3	6	4	0	0	4	1	0	2	0	2	2	9			
	%	1%	0%	0%	0%	1%	1%	0%	0%	1%	0%	0%	1%	0%	1%	1%	1%			
	ticulated Truc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	1	0	0	1	1	0	0	0	0	0	0	1	0	1	1	2			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	289	521	0	810	860	573	15	0	588	545	24	287	0	311	304	1709			
	PHF	0.93	0.97	0	0.96	0.91	0.96	0.31	0	0.91	0.96	0.6	0.82	0	0.8	0.84	0.91			
	Approach %				47%	50%				34%	32%				18%	18%				

Study Name Swampscott - Route 1A and Loring Avenue TMC # 6 TMC
Start Date Saturday, April 09, 2016 12:00 PM
End Date Tuesday, April 12, 2016 6:00 PM
Site Code

			So	uthbou	nd			No	rthbou	ınd			Nort	heastb	ound				Cross	walk
Time Period	Class.	BR	Т	U	ı	0	T	HL	U	1	0	HR	BL	U	- 1	0	Total		:destria	Total
Peak 1	Motorcycles	0	1	0	1	6	6	0	0	6	1	0	0	0	0	0	7	N	0	0
Specified Period	%	0%	0%	0%	0%	1%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%		0%	
12:00 PM - 2:00 PM	Cars	200	477	0	677	684	454	18	0	472	508	31	230	0	261	218	1410	S	1	1
One Hour Peak	%	90%	90%	0%	90%	91%	92%	95%	0%	92%	91%	97%	90%	0%	91%	90%	91%		100%	
12:00 PM - 1:00 PM	nt Goods Vehi	19	46	0	65	50	31	0	0	31	46	0	19	0	19	19	115	SW	4	4
	%	9%	9%	0%	9%	7%	6%	0%	0%	6%	8%	0%	7%	0%	7%	8%	7%		100%	
	Buses	2	2	0	4	1	0	0	0	0	2	0	1	0	1	2	5		5	5
	%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%			
	ngle-Unit Truc	1	2	0	3	7	2	1	0	3	3	1	5	0	6	2	12			
	%	0%	0%	0%	0%	1%	0%	5%	0%	1%	1%	3%	2%	0%	2%	1%	1%			
	ticulated Truc	0	0	0	0	1	1	0	0	1	0	0	0	0	0	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	1	1	0	2	0	0	0	0	0	1	0	0	0	0	1	2			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	223	529	0	752	749	494	19	0	513	561	32	255	0	287	242	1552			
	PHF	0.82	0.95	0	0.92	0.89	0.85	0.68	0	0.84	0.96	0.8	0.87	0	0.88	0.86	0.93			
	Approach %				48%	48%				33%	36%				18%	16%				

Study Name Swampscott - Route 1A and Leggs Hill Road TMC # 7 TMC
Start Date Saturday, April 09, 2016 12:00 PM
End Date Tuesday, April 12, 2016 6:00 PM
Site Code

			Soutl	hwestb	ound _			Nort	hwestb	ound _			Nort	heastb	ound _				Cross	walk
Time Period	Class.	Т	L	U	1	0	R	L	U	- 1	0	R	Т	U	1	0	Total		:destria	Tota
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	NE	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
7:00 AM - 9:00 AM	Cars	544	61	0	605	841	140	5	0	145	85	24	701	0	725	549	1475	SE	0	0
One Hour Peak	%	87%	91%	0%	87%	90%	92%	100%	0%	92%	92%	96%	89%	0%	90%	87%	89%		0%	
7:30 AM - 8:30 AM	nt Goods Vehi	54	5	0	59	64	9	0	0	9	6	1	55	0	56	54	124	SW	0	0
	%	9%	7%	0%	8%	7%	6%	0%	0%	6%	7%	4%	7%	0%	7%	9%	7%		0%	
	Buses	9	0	0	9	11	0	0	0	0	0	0	11	0	11	9	20		0	0
	%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%	0%	1%	1%	1%			
	ngle-Unit Truc	17	1	0	18	18	3	0	0	3	1	0	15	0	15	17	36			
	%	3%	1%	0%	3%	2%	2%	0%	0%	2%	1%	0%	2%	0%	2%	3%	2%			
	ticulated Truc	2	0	0	2	3	0	0	0	0	0	0	3	0	3	2	5			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	2	0	0	2	0	0	0	0	0	0	0	0	0	0	2	2			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	628	67	0	695	937	152	5	0	157	92	25	785	0	810	633	1662			
	PHF	0.91	0.8	0	0.9	0.93	0.83	0.62	0	0.82	0.82	0.89	0.92	0	0.92	0.91	0.96			
	Approach %				42%	56%				9%	6%				49%	38%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	NE	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
4:00 PM - 6:00 PM	Cars	764	155	0	919	804	83	11	0	94	199	44	721	0	765	775	1778	SE	2	2
One Hour Peak	%	93%	95%	0%	93%	93%	93%	92%	0%	93%	95%	96%	93%	0%	93%	93%	93%		100%	
5:00 PM - 6:00 PM	nt Goods Vehi	46	9	0	55	48	6	0	0	6	11	2	42	0	44	46	105	SW	0	0
	%	6%	5%	0%	6%	6%	7%	0%	0%	6%	5%	4%	5%	0%	5%	6%	6%		0%	
	Buses	8	0	0	8	2	0	0	0	0	0	0	2	0	2	8	10		2	2
	%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	1%			
	ngle-Unit Truc	4	0	0	4	9	0	1	0	1	0	0	9	0	9	5	14			
	%	0%	0%	0%	0%	1%	0%	8%	0%	1%	0%	0%	1%	0%	1%	1%	1%			
	ticulated Truc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	823	164	0	987	863	89	12	0	101	210	46	774	0	820	835	1908			
	PHF	0.92	0.82	0	0.94	0.98	0.93	0.6	0	0.9	0.78	0.68	0.99	0	0.97	0.93	0.97			
	Approach %				52%	45%				5%	11%				43%	44%				

Study Name Swampscott - Route 1A and Leggs Hill Road TMC # 7 TMC
Start Date Saturday, April 09, 2016 12:00 PM
Tuesday, April 12, 2016 6:00 PM
Site Code

			Sout	hwestb	ound			Nort	hwestb	ound			Nort	heastb	ound				Cross	walk
Time Period	Class.	Т	L	U	ı	0	R	L	U	l l	0	R	T	U	- 1	0	Total		:destria	Total
Peak 1	Motorcycles	1	0	0	1	6	0	0	0	0	0	0	6	0	6	1	7	NE	0	0
Specified Period	%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	1%	0%	1%	0%	0%		0%	
12:00 PM - 2:00 PM	Cars	642	79	0	721	778	117	15	0	132	99	20	661	0	681	657	1534	SE	3	3
One Hour Peak	%	90%	94%	0%	90%	92%	95%	94%	0%	95%	93%	87%	92%	0%	91%	90%	91%		100%	
12:00 PM - 1:00 PM	nt Goods Vehi	64	5	0	69	50	6	1	0	7	8	3	44	0	47	65	123	SW	0	0
	%	9%	6%	0%	9%	6%	5%	6%	0%	5%	7%	13%	6%	0%	6%	9%	7%		0%	
	Buses	3	0	0	3	1	0	0	0	0	0	0	1	0	1	3	4		3	3
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	ngle-Unit Truc	3	0	0	3	10	0	0	0	0	0	0	10	0	10	3	13			
	%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	1%	0%	1%	0%	1%			
	ticulated Truc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	714	84	0	798	845	123	16	0	139	107	23	722	0	745	730	1682			
	PHF	0.94	0.81	0	0.94	0.91	0.88	0.8	0	0.89	0.86	0.72	0.91	0	0.9	0.94	0.93			
	Approach %				47%	50%				8%	6%				44%	43%				

Study Name Swampscott - Essex Street and Stop and Shop Driveway TMC # 8 TMC

Start Date Saturday, April 09, 2016 12:00 PM

Tuesday, April 12, 2016 6:00 PM

Site Code

			Sout	nwestb	ound _			No	rthbou	ınd			Nort	heastb	ound _				Cross	swal
Time Period	Class.	Т	BL	U		0	BR	HL	U	L	0	HR	Т	U		0	Total		edestria	
Peak 1	Motorcycles	0	0	0	0	0	0	1	0	1	1	1	0	0	1	1	2	NE	0	(
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	1%	0%	0%	0%	0%	0%		0%	
:00 AM - 9:00 AM	Cars	600	43	0	643	526	20	183	0	203	174	131	506	0	637	783	1483	S	1	:
One Hour Peak	%	90%	98%	0%	91%	82%	91%	90%	0%	90%	90%	88%	81%	0%	83%	90%	87%		100%	
7:30 AM - 8:30 AM	nt Goods Vehic	46	0	0	46	75	2	15	0	17	13	13	73	0	86	61	149	SW	0	(
	%	7%	0%	0%	6%	12%	9%	7%	0%	8%	7%	9%	12%	0%	11%	7%	9%		0%	
	Buses	8	0	0	8	6	0	1	0	1	0	0	6	0	6	9	15		1	
	%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	1%	0%	1%	1%	1%			
	ngle-Unit Truc	10	1	0	11	35	0	2	0	2	5	4	35	0	39	12	52			
	%	2%	2%	0%	2%	5%	0%	1%	0%	1%	3%	3%	6%	0%	5%	1%	3%			
	ticulated Truc	0	0	0	0	1	0	1	0	1	0	0	1	0	1	1	2			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	2	0	0	2	0	0	0	0	0	0	0	0	0	0	2	2			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	666	44	0	710	643	22	203	0	225	193	149	621	0	770	869	1705			
	PHF	0.9	0.85	0	0.93	0.93	0.69	0.79	0	0.79	0.88	0.89	0.92	0	0.93	0.89	0.96			
	Approach %				42%	38%				13%	11%				45%	51%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	NE	0	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
4:00 PM - 6:00 PM	Cars	566	65	0	631	737	99	244	0	343	300	235	638	0	873	810	1847	S	0	
One Hour Peak	%	90%	98%	0%	91%	92%	92%	94%	0%	93%	94%	93%	92%	0%	92%	91%	92%		0%	
4:30 PM - 5:30 PM	nt Goods Vehic	57	1	0	58	55	9	14	0	23	19	18	46	0	64	71	145	SW	0	
	%	9%	2%	0%	8%	7%	8%	5%	0%	6%	6%	7%	7%	0%	7%	8%	7%		0%	
	Buses	2	0	0	2	4	0	0	0	0	0	0	4	0	4	2	6		0	
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%			
	ngle-Unit Truc	6	0	0	6	4	0	1	0	1	1	1	4	0	5	7	12			
	%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	1%	0%	1%	1%	1%			
	ticulated Truc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	0	0	0	0	1	0	0	0	0	0	0	1	0	1	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	631	66	0	697	801	108	259	0	367	320	254	693	0	947	890	2011			
	PHF	0.92	0.87	0	0.92	0.93	0.84	0.79	0	0.87	0.85	0.85	0.92	0	0.94	0.88	0.93			
	Approach %				35%	40%				18%	16%				47%	44%				

Study Name Swampscott - Essex Street and Stop and Shop Driveway TMC # 8 TMC

Start Date Saturday, April 09, 2016 12:00 PM

Tuesday, April 12, 2016 6:00 PM

Site Code

			Soutl	nwestb	ound			No	rthbou	ınd			Nort	heastb	ound				Cross	swalk
Time Period	Class.	T	BL	U	1	0	BR	HL	U	- 1	0	HR	T	U	1	0	Total		:destria	Total
Peak 1	Motorcycles	1	0	0	1	1	0	0	0	0	0	0	1	0	1	1	2	NE	2	2
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
12:00 PM - 2:00 PM	Cars	503	96	0	599	595	131	278	0	409	358	262	464	0	726	781	1734	S	2	2
One Hour Peak	%	87%	97%	0%	89%	88%	94%	96%	0%	95%	93%	92%	87%	0%	89%	90%	90%		100%	
12:30 PM - 1:30 PM	nt Goods Vehi	54	3	0	57	64	8	10	0	18	23	20	56	0	76	64	151	SW	0	0
	%	9%	3%	0%	8%	9%	6%	3%	0%	4%	6%	7%	10%	0%	9%	7%	8%		0%	
	Buses	3	0	0	3	1	0	0	0	0	0	0	1	0	1	3	4		4	4
	%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	ngle-Unit Truc	13	0	0	13	12	0	2	0	2	2	2	12	0	14	15	29			
	%	2%	0%	0%	2%	2%	0%	1%	0%	0%	1%	1%	2%	0%	2%	2%	2%			
	ticulated Truc	0	0	0	0	0	0	1	0	1	0	0	0	0	0	1	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	1	0	0	1	2	1	0	0	1	0	0	1	0	1	1	3			
	%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	575	99	0	674	675	140	291	0	431	383	284	535	0	819	866	1924			
	PHF	0.96	0.88	0	0.96	0.9	0.83	0.87	0	0.86	0.97	0.99	0.92	0	0.94	0.96	0.95			
	Approach %				35%	35%				22%	20%				43%	45%				

Study Name Swampscott - Loring Avenue and Vinnin Street TMC # 9 TMC
Start Date Saturday, April 09, 2016 12:00 PM
End Date Tuesday, April 12, 2016 6:00 PM
Site Code

			Soutl	nwestb	ound _			W	estbou	nd			Nort	heastb	ound _				Cross	walk
Time Period	Class.	Т	HL	U	L	0	HR	BL	U		0	BR	Т	U		0	Total		edestria	Tota
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	NE	1	1
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		100%	
7:00 AM - 9:00 AM	Cars	131	14	0	145	204	16	321	0	337	284	270	188	0	458	452	940	Ε	0	0
One Hour Peak	%	85%	100%	0%	86%	86%	94%	90%	0%	90%	83%	82%	86%	0%	83%	88%	86%		0%	
7:15 AM - 8:15 AM	nt Goods Vehi	9	0	0	9	16	1	30	0	31	44	44	15	0	59	39	99	SW	0	0
	%	6%	0%	0%	5%	7%	6%	8%	0%	8%	13%	13%	7%	0%	11%	8%	9%		0%	
	Buses	6	0	0	6	5	0	2	0	2	4	4	5	0	9	8	17		1	1
	%	4%	0%	0%	4%	2%	0%	1%	0%	1%	1%	1%	2%	0%	2%	2%	2%			
	ngle-Unit Truc	8	0	0	8	10	0	4	0	4	12	12	10	0	22	12	34			
	%	5%	0%	0%	5%	4%	0%	1%	0%	1%	3%	4%	5%	0%	4%	2%	3%			
	ticulated Truc	0	0	0	0	1	0	0	0	0	0	0	1	0	1	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	154	14	0	168	236	17	357	0	374	344	330	219	0	549	511	1091			
	PHF	0.79	0.58	0	0.79	8.0	0.47	0.74	0	0.72	0.77	0.76	0.84	0	0.82	0.75	0.81			
	Approach %				15%	22%				34%	32%				50%	47%				
Peak 2	Motorcycles	3	0	0	3	0	0	6	0	6	0	0	0	0	0	9	9	NE	5	5
Specified Period	%	1%	0%	0%	1%	0%	0%	2%	0%	2%	0%	0%	0%	0%	0%	1%	1%		100%	
4:00 PM - 6:00 PM	Cars	268	24	0	292	297	25	278	0	303	427	403	272	0	675	546	1270	Ε	1	1
One Hour Peak	%	92%	96%	0%	92%	92%	100%	89%	0%	89%	92%	92%	91%	0%	91%	90%	91%		100%	
4:45 PM - 5:45 PM	nt Goods Vehic	17	1	0	18	22	0	27	0	27	32	31	22	0	53	44	98	SW	1	1
	%	6%	4%	0%	6%	7%	0%	9%	0%	8%	7%	7%	7%	0%	7%	7%	7%		100%	
	Buses	1	0	0	1	2	0	1	0	1	1	1	2	0	3	2	5		7	7
	%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%			
	ngle-Unit Truc	2	0	0	2	1	0	2	0	2	5	5	1	0	6	4	10			
	%	1%	0%	0%	1%	0%	0%	1%	0%	1%	1%	1%	0%	0%	1%	1%	1%			
	ticulated Truc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	1	0	0	1	1	0	0	0	0	0	0	1	0	1	1	2			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	292	25	0	317	323	25	314	0	339	465	440	298	0	738	606	1394			
	PHF	0.91	0.62	0	0.95	0.87	0.62	0.82	0	0.8	0.9	0.91	0.84	0	0.91	0.91	0.92			
	Approach %				23%	23%				24%	33%				53%	43%				

Study Name Swampscott - Loring Avenue and Vinnin Street TMC # 9 TMC
Start Date Saturday, April 09, 2016 12:00 PM
End Date Tuesday, April 12, 2016 6:00 PM
Site Code

			South	nwestb	ound			W	estbou	nd			Nort	heastb	ound				Cross	walk
Time Period	Class.	T	HL	U	- 1	0	HR	BL	U	L	0	BR	T	U	1	0	Total		:destria	Total
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	2	2	0	0	2	0	2	NE	3	3
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	1%	0%	0%	0%	0%	0%		100%	
12:00 PM - 2:00 PM	Cars	213	44	0	257	278	44	335	0	379	352	308	234	0	542	548	1178	Ε	0	0
One Hour Peak	%	90%	100%	0%	92%	92%	94%	88%	0%	88%	89%	88%	92%	0%	89%	89%	90%		0%	
12:00 PM - 1:00 PM	nt Goods Vehi	20	0	0	20	18	2	39	0	41	36	36	16	0	52	59	113	SW	0	0
	%	8%	0%	0%	7%	6%	4%	10%	0%	10%	9%	10%	6%	0%	9%	10%	9%		0%	
	Buses	2	0	0	2	1	0	0	0	0	0	0	1	0	1	2	3		3	3
	%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	ngle-Unit Truc	1	0	0	1	4	1	8	0	9	5	5	3	0	8	9	18			
	%	0%	0%	0%	0%	1%	2%	2%	0%	2%	1%	1%	1%	0%	1%	1%	1%			
	ticulated Truc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	0	0	0	0	0	0	0	0	0	1	1	0	0	1	0	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	236	44	0	280	301	47	382	0	429	396	352	254	0	606	618	1315			
	PHF	0.86	0.73	0	0.83	0.96	0.84	0.88	0	0.88	0.93	0.93	0.95	0	0.95	0.94	0.95			
	Approach %				21%	23%				33%	30%				46%	47%				

Study Name Swampscott - Vinnin Street at Salem Street and Paradise Plaza Exit Driveway TMC # 10 TMC
Start Date Saturday, April 09, 2016 12:00 PM
End Date Tuesday, April 12, 2016 6:00 PM
Site Code

			So	uthbou	nd			W	estbou	nd			No	rthbou	ınd			<u> </u>	astboui	nd				Cross	sw <u>alk</u>
Time Period	Class.	R	T	L		0	Т	L	U		0	R	L	U	L	0	R	T	U	1	0	Total		edestria	
Peak 1	Motorcycles	0	0	0	0	0	0	1	0	1	0	0	0	0	0	1	0	0	0	0	0	1	N	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	2%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
7:00 AM - 9:00 AM	Cars	4	1	0	5	0	596	56	0	652	428	63	130	0	193	184	127	365	0	492	730	1342	Ε	0	0
One Hour Peak	%	100%	100%	0%	100%	0%	91%	89%	0%	91%	87%	93%	90%	0%	91%	86%	84%	86%	0%	86%	91%	89%		0%	
7:30 AM - 8:30 AM	nt Goods Vehi	0	0	0	0	0	49	3	0	52	53	4	6	0	10	19	16	49	0	65	55	127	S	0	0
	%	0%	0%	0%	0%	0%	7%	5%	0%	7%	11%	6%	4%	0%	5%	9%	11%	12%	0%	11%	7%	8%		0%	
	Buses	0	0	0	0	0	2	0	0	2	1	0	6	0	6	2	2	1	0	3	8	11	W	0	0
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	4%	0%	3%	1%	1%	0%	0%	1%	1%	1%		0%	
	ngle-Unit Truc	0	0	0	0	0	7	1	0	8	8	1	1	0	2	7	6	7	0	13	8	23		0	0
	%	0%	0%	0%	0%	0%	1%	2%	0%	1%	2%	1%	1%	0%	1%	3%	4%	2%	0%	2%	1%	2%			
	ticulated Truc	0	0	0	0	0	0	1	0	1	1	0	2	0	2	1	0	1	0	1	2	4			
	%	0%	0%	0%	0%	0%	0%	2%	0%	0%	0%	0%	1%	0%	1%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	0	0	0	0	0	0	1	0	1	0	0	0	0	0	1	0	0	0	0	0	1			
	%	0%	0%	0%	0%	0%	0%	2%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	4	1	0	5	0	654	63	0	717	491	68	145	0	213	215	151	423	0	574	803	1509			
	PHF	0.5	0.25	0	0.62	0	0.97	0.88	0	0.97	0.93	0.94	0.86	0	0.93	0.88	0.8	0.93	0	0.91	0.97	0.97			
	Approach %				0%	0%				48%	33%				14%	14%				38%	53%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
4:00 PM - 6:00 PM	Cars	22	2	0	24	0	402	63	0	465	694	93	109	0	202	249	184	601	0	785	533	1476	Ε	0	(
One Hour Peak	%	100%	100%	0%	100%	0%	91%	90%	0%	91%	93%	96%	90%	0%	93%	93%	93%	93%	0%	93%	91%	92%		0%	
4:45 PM - 5:45 PM	nt Goods Vehic	0	0	0	0	0	34	4	0	38	48	4	11	0	15	12	8	44	0	52	45	105	S	0	(
	%	0%	0%	0%	0%	0%	8%	6%	0%	7%	6%	4%	9%	0%	7%	4%	4%	7%	0%	6%	8%	7%		0%	
	Buses	0	0	0	0	0	1	2	0	3	1	0	1	0	1	4	2	1	0	3	2	7	W	0	C
	%	0%	0%	0%	0%	0%	0%	3%	0%	1%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%		0%	
	ngle-Unit Truc	0	0	0	0	0	4	0	0	4	2	0	0	0	0	3	3	2	0	5	4	9		0	(
	%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	1%	2%	0%	0%	1%	1%	1%			
	ticulated Truc	0	0	0	0	0	0	1	0	1	0	0	0	0	0	1	0	0	0	0	0	1			
	%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	22	2	0	24	0	441	70	0	511	745	97	121	0	218	269	197	648	0	845	584	1598			
	PHF	0.5	0.5	0	0.55	0	0.93	0.83	0	0.91	0.9	0.87	0.92	0	0.92	0.93	0.95	0.9	0	0.91	0.94	0.95			
	Approach %				2%	0%				32%	47%				14%	17%				53%	37%				

Study Name Swampscott - Vinnin Street at Salem Street and Paradise Plaza Exit Driveway TMC # 10 TMC
Start Date Saturday, April 09, 2016 12:00 PM
End Date Tuesday, April 12, 2016 6:00 PM
Site Code

			So	uthbou	ınd			W	estbou	nd			No	rthbou	ınd			Ea	astbou	nd				Cross	swa
Time Period	Class.	R	T	L	ı	0	T	L	U	L	0	R	L	U	- 1	0	R	T	U	- 1	0	Total		:destria	То
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2	0	0	2	0	2	N	1	
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	1%	0%	0%	0%	0%	0%		100%	
2:00 PM - 2:00 PM	Cars	32	10	0	42	0	464	135	0	599	545	90	164	0	254	301	156	455	0	611	660	1506	Ε	0	
One Hour Peak	%	100%	91%	0%	98%	0%	88%	92%	0%	89%	91%	94%	89%	0%	90%	89%	85%	90%	0%	89%	89%	89%		0%	
2:00 PM - 1:00 PM	nt Goods Vehi	0	1	0	1	0	56	8	0	64	48	4	16	0	20	28	19	44	0	63	72	148	S	2	
	%	0%	9%	0%	2%	0%	11%	5%	0%	10%	8%	4%	9%	0%	7%	8%	10%	9%	0%	9%	10%	9%		100%	
	Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2	0	0	2	0	2	W	3	
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	1%	0%	0%	0%	0%	0%		100%	
	ngle-Unit Truc	0	0	0	0	0	6	2	0	8	6	1	5	0	6	6	4	5	0	9	11	23		6	
	%	0%	0%	0%	0%	0%	1%	1%	0%	1%	1%	1%	3%	0%	2%	2%	2%	1%	0%	1%	1%	1%			
	ticulated Truc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	0	0	0	0	0	1	1	0	2	2	1	0	0	1	1	0	1	0	1	1	4			
	%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	32	11	0	43	0	527	146	0	673	601	96	185	0	281	340	183	505	0	688	744	1685			
	PHF	0.89	0.55	0	0.77	0	0.95	0.89	0	0.94	0.96	0.92	0.91	0	0.92	0.89	0.88	0.96	0	0.97	0.97	0.97			
	Approach %				3%	0%				40%	36%				17%	20%				41%	44%				

Study Name Swampscott - Tedesco Street and Brookhouse Drive TMC # 11 TMC
Start Date Saturday, April 09, 2016 12:00 PM
End Date Tuesday, April 12, 2016 6:00 PM
Site Code

			W	estbou	nd			Nor <u>t</u> l	hwestb	ound			Ea	astbour	nd				Cross	walk
Time Period	Class.	Т	HL	U	1	0	HR	BL	U	1	0	BR	Т	U	1	0	Total		edestria	
Peak 1	Motorcycles	0	0	0	0	0	0	1	0	1	0	0	0	0	0	1	1	Е	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	1%	0%	1%	0%	0%	0%	0%	0%	0%	0%		0%	
7:00 AM - 9:00 AM	Cars	586	28	0	614	424	75	67	0	142	94	66	349	0	415	653	1171	SE	1	1
One Hour Peak	%	91%	88%	0%	91%	86%	99%	92%	0%	95%	86%	86%	84%	0%	84%	91%	89%		100%	
7:30 AM - 8:30 AM	nt Goods Vehi	47	3	0	50	51	1	5	0	6	12	9	50	0	59	52	115	W	0	0
	%	7%	9%	0%	7%	10%	1%	7%	0%	4%	11%	12%	12%	0%	12%	7%	9%		0%	
	Buses	2	0	0	2	0	0	0	0	0	0	0	0	0	0	2	2		1	1
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	ngle-Unit Truc	5	1	0	6	15	0	0	0	0	3	2	15	0	17	5	23			
	%	1%	3%	0%	1%	3%	0%	0%	0%	0%	3%	3%	4%	0%	3%	1%	2%			
	ticulated Truc	2	0	0	2	1	0	0	0	0	0	0	1	0	1	2	3			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	643	32	0	675	491	76	73	0	149	109	77	415	0	492	716	1316			
	PHF	0.94	0.62	0	0.92	0.87	0.73	0.76	0	0.85	0.85	0.88	0.9	0	0.93	0.95	0.91			
	Approach %				51%	37%				11%	8%				37%	54%				
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	E	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
4:00 PM - 6:00 PM	Cars	492	58	0	550	602	51	42	0	93	186	128	551	0	679	534	1322	SE	3	3
One Hour Peak	%	91%	98%	0%	91%	93%	98%	89%	0%	94%	94%	92%	93%	0%	93%	91%	92%		100%	
4:45 PM - 5:45 PM	nt Goods Vehic	43	1	1	45	38	1	5	0	6	11	10	36	0	46	48	97	W	0	0
	%	8%	2%	100%	7%	6%	2%	11%	0%	6%	6%	7%	6%	0%	6%	8%	7%		0%	
	Buses	2	0	0	2	2	0	0	0	0	0	0	2	0	2	2	4		3	3
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	ngle-Unit Truc	4	0	0	4	2	0	0	0	0	1	1	2	0	3	4	7			
	%	1%	0%	0%	1%	0%	0%	0%	0%	0%	1%	1%	0%	0%	0%	1%	0%			
	ticulated Truc	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1	1			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	542	59	1	602	644	52	47	0	99	198	139	591	0	730	589	1431			
	PHF	0.9	0.78	0.25	0.89	0.91	0.93	0.78	0	0.85	0.88	0.94	0.91	0	0.92	0.91	0.93			
	Approach %				42%	45%				7%	14%				51%	41%				

Study Name Swampscott - Tedesco Street and Brookhouse Drive TMC # 11 TMC
Start Date Saturday, April 09, 2016 12:00 PM
End Date Tuesday, April 12, 2016 6:00 PM
Site Code

			W	estbou	nd			Nort	hwestb	ound			E	astbou	nd				Cross	walk
Time Period	Class.	Т	HL	U	- 1	0	HR	BL	U	- 1	0	BR	T	U	- 1	0	Total		edestria	Total
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	Ε	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	
12:00 PM - 2:00 PM	Cars	585	39	0	624	473	29	43	0	72	125	86	444	0	530	628	1226	SE	0	0
One Hour Peak	%	93%	100%	0%	93%	92%	94%	84%	0%	88%	91%	87%	92%	0%	91%	92%	92%		0%	
12:15 PM - 1:15 PM	nt Goods Vehi	39	0	0	39	36	2	6	0	8	11	11	34	0	45	45	92	W	0	0
	%	6%	0%	0%	6%	7%	6%	12%	0%	10%	8%	11%	7%	0%	8%	7%	7%		0%	
	Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0	0
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	ngle-Unit Truc	6	0	0	6	4	0	2	0	2	2	2	4	0	6	8	14			
	%	1%	0%	0%	1%	1%	0%	4%	0%	2%	1%	2%	1%	0%	1%	1%	1%			
	ticulated Truc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	icycles on Roa	2	0	0	2	1	0	0	0	0	0	0	1	0	1	2	3			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%			
	Total	632	39	0	671	514	31	51	0	82	138	99	483	0	582	683	1335			
	PHF	0.91	0.81	0	0.91	0.91	0.65	0.8	0	0.82	0.91	0.88	0.94	0	0.95	0.9	0.95			
	Approach %				50%	39%				6%	10%				44%	51%				

File Name : TedescoLeggsHillAM+PM Site Code : 06071601

Site Code : 060/1601 Start Date : 6/7/2016

Grou	ps Printe	d- Unshifted	d - Bank 1	l - Bank 2

								Group	s Printe	a- Unsnitte	u - Bank	ı - Banı	(ı
		-	rom Nor	415				rom Eas	_1			-	rom Sou	.41-				rom We	-4		
<u> </u>										-											
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
07:00 AM	14	0	10	0	24	20	154	0	0	174	0	0	0	0	0	0	67	22	0	89	287
07:15 AM	17	0	14	0	31	33	137	0	0	170	0	0	0	0	0	0	81	21	0	102	303
07:30 AM	14	0	16	0	30	33	137	0	0	170	0	0	0	0	0	0	105	28	0	133	333
07:45 AM	10	0	20	2	32	41	166	0	0	207	0	0	0	0	0	0	83	38	0	121	360
Total	55	0	60	2	117	127	594	0	0	721	0	0	0	0	0	0	336	109	0	445	1283
08:00 AM	19	1	12	0	32	47	170	0	0	217	0	0	0	0	0	0	81	47	0	128	377
08:15 AM	18	0	25	0	43	53	144	0	0	197	0	0	0	0	0	0	83	49	0	132	372
08:30 AM	29	0	13	0	42	33	138	0	0	171	0	0	0	0	0	0	78	41	0	119	332
08:45 AM	36	0	15	1	52	28	126	0	0	154	0	0	0	0	0	0	97	28	1	126	332
Total	102	1	65	1	169	161	578	0	0	739	0	0	0	0	0	0	339	165	1	505	1413
*** BREAK ***																					
04:00 PM	20	0	23	0	43	21	129	0	0	150	0	0	0	0	0	0	119	37	0	156	349
04:15 PM	36	0	26	0	62	30	108	0	0	138	0	0	0	0	0	0	136	36	0	172	372
04:30 PM	37	0	24	0	61	27	101	0	0	128	0	0	0	0	0	0	134	30	0	164	353
04:45 PM	30	0	25	2	57	19	110	0	0	129	1	0	0	0	1	0	134	36	0	170	357
Total	123	0	98	2	223	97	448	0	0	545	1	0	0	0	1	0	523	139	0	662	1431
05:00 PM	44	0	24	0	68	18	115	0	0	133	0	0	0	0	0	0	125	38	0	163	364
05:15 PM	50	0	25	0	75	23	108	0	Ö	131	0	0	0	0	0	0	123	29	0	152	358
05:30 PM	56	0	41	0	97	25	93	0	0	118	0	0	0	0	o l	0	132	32	0	164	379
05:45 PM	37	Ö	22	Ö	59	34	120	0	Ö	154	Ö	Ö	Ö	Ö	ő	Ö	154	34	Ö	188	401
Total	187	0	112	0	299	100	436	0	0	536	0	0	0	0	0	0	534	133	0	667	1502
Grand Total	467	1	335	5	808	485	2056	0	0	2541	1	0	0	0	1	0	1732	546	1	2279	5629
Apprch %	57.8	0.1	41.5	0.6	000	465 19.1	2056 80.9	0	0	2041	100	0	0	0	'	0	76	24	0	2219	3029
Total %	8.3	0.1	41.5	0.0	14.4	8.6	36.5	0	0	45.1	0	0	0	0	0	0	30.8	9.7	0	40.5	
Unshifted	462	0	327	5	794	479	2009	0	0	2488	1	0	0	0	1	0	1664	9.7 540	1	2205	5488
% Unshifted	98.9	0	327 97.6	100	98.3	98.8	2009 97.7	0	0	97.9	100	0	0	0	100	0	96.1	98.9	100	96.8	97.5
Bank 1	3	1	7	0	90.3	90.0	44	0	0	49	0	0	0	0	0	0	64	90.9	0	68	128
% Bank 1	0.6	100	2.1	0	1.4	J 1	2.1	0	0	1.9	0	0	0	0	0	0	3.7	0.7	0	3	2.3
Bank 2	2	0	<u> </u>	0	3	1	3	0	0	1.9	0	0	0	0	0	0	<u> </u>	2	0	<u>5</u>	13
% Bank 2	0.4	0	0.3	0	0.4	0.2	0.1	0	0	0.2	0	0	0	0	0	0	0.2	0.4	0	0.3	0.2
70 Balik 2	0.4	U	0.3	U	0.4	0.2	U. I	U	U	0.2	U	U	U	U	U	U	0.2	0.4	U	0.5	0.2

File Name : TedescoLeggsHillAM+PM Site Code : 06071601

Site Code : 06071601 Start Date : 6/7/2016

																					l .
		F	rom Nor	th			F	rom Ea	st			F	rom Sou	ıth			F	rom We	st		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analys	is From 0	7:00 AM	to 11:45	AM - Pe	eak 1 of 1			•					•			•					
Peak Hour for Ent	ire Interse	ection Be	gins at 0	7:30 AM	1 .																
07:30 AM	14	0	16	0	30	33	137	0	0	170	0	0	0	0	0	0	105	28	0	133	333
07:45 AM	10	0	20	2	32	41	166	0	0	207	0	0	0	0	0	0	83	38	0	121	360
08:00 AM	19	1	12	0	32	47	170	0	0	217	0	0	0	0	0	0	81	47	0	128	377
08:15 AM	18	0	25	0	43	53	144	0	0	197	0	0	0	0	0	0	83	49	0	132	372
Total Volume	61	1	73	2	137	174	617	0	0	791	0	0	0	0	0	0	352	162	0	514	1442
% App. Total	44.5	0.7	53.3	1.5		22	78	0	0		0	0	0	0		0	68.5	31.5	0		1
PHF	.803	.250	.730	.250	.797	.821	.907	.000	.000	.911	.000	.000	.000	.000	.000	.000	.838	.827	.000	.966	.956

File Name : TedescoLeggsHillAM+PM Site Code : 06071601

Site Code : 06071601 Start Date : 6/7/2016

																					1
		_	NI	41-					_1			_	0	.41-			_	\\/-	_4		
			rom Nor	τn				From Ea	St			FI	<u>rom Sou</u>	itn				rom We	St		1
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. To
Peak Hour Analy				5 AM - Pe	eak 1 of 1																
Peak Hour for Ea	ach Appro	ach Begir	ns at:																		_
	08:00 AM	1				07:45 AM					07:00 AM					07:30 AM					1
+0 mins.	19	1	12	0	32	41	166	0	0	207	0	0	0	0	0	0	105	28	0	133	
+15 mins.	18	0	25	0	43	47	170	0	0	217	0	0	0	0	0	0	83	38	0	121	
+30 mins.	29	0	13	0	42	53	144	0	0	197	0	0	0	0	0	0	81	47	0	128	
+45 mins.	36	0	15	1	52	33	138	0	0	171	0	0	0	0	0	0	83	49	0	132	
Total Volume		1	65	1	169	174	618	0	0	792	0	0	0	0	0	0	352	162	0	514	
% App. Total	60.4	0.6	38.5	0.6		22	78	0	0		0	0	0	0		0	68.5	31.5	0		
PHF		250	650	250	813	821	909	000	000	912	000	000	000	000	000	000	838	827	000	966	

File Name : TedescoLeggsHillAM+PM Site Code : 06071601

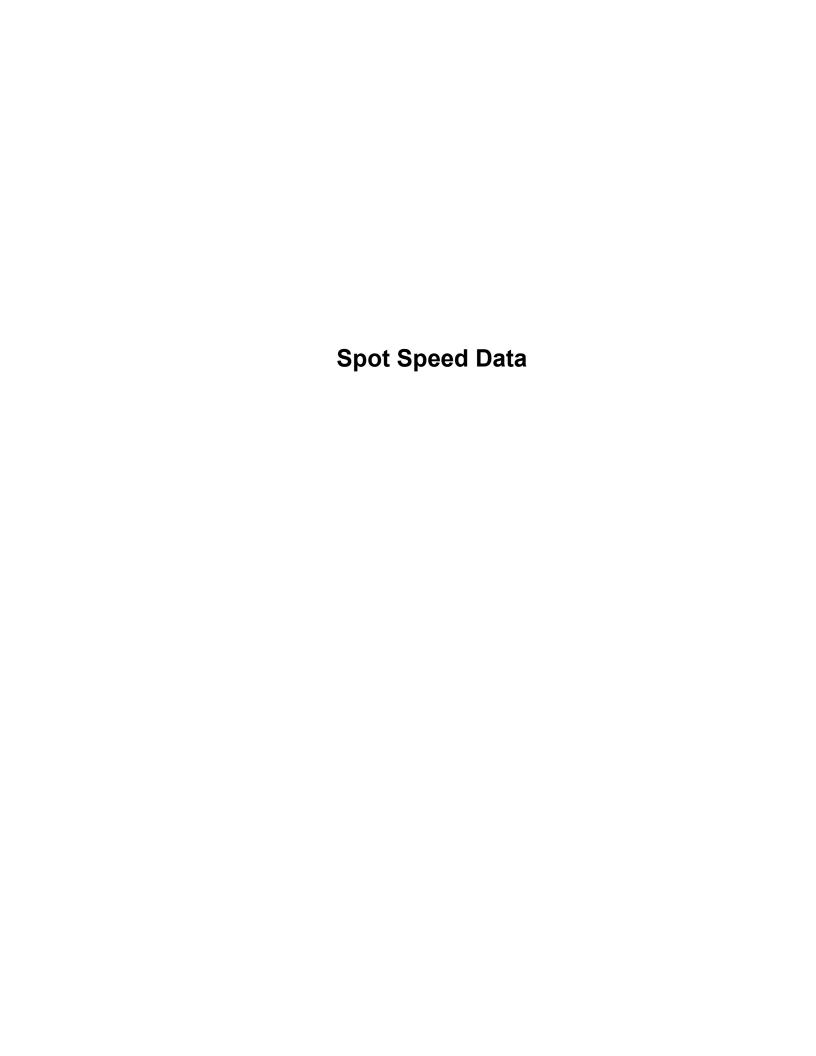
Site Code : 06071601 Start Date : 6/7/2016

		F	rom Nor	th			F	rom Ea	st			F	rom Sou	ıth			F	rom We	st		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analys	is From 0	4:00 PM	to 05:45	PM - Pe	eak 1 of 1	,				,	,	•				•	•			•	
Peak Hour for Ent	ire Interse	ection Be	gins at 0	5:00 PM	1 .																
05:00 PM	44	0	24	0	68	18	115	0	0	133	0	0	0	0	0	0	125	38	0	163	364
05:15 PM	50	0	25	0	75	23	108	0	0	131	0	0	0	0	0	0	123	29	0	152	358
05:30 PM	56	0	41	0	97	25	93	0	0	118	0	0	0	0	0	0	132	32	0	164	379
05:45 PM	37	0	22	0	59	34	120	0	0	154	0	0	0	0	0	0	154	34	0	188	401
Total Volume	187	0	112	0	299	100	436	0	0	536	0	0	0	0	0	0	534	133	0	667	1502
% App. Total	62.5	0	37.5	0		18.7	81.3	0	0		0	0	0	0		0	80.1	19.9	0		
PHF	.835	.000	.683	.000	.771	.735	.908	.000	.000	.870	.000	.000	.000	.000	.000	.000	.867	.875	.000	.887	.936

File Name: TedescoLeggsHillAM+PM

Site Code : 06071601 Start Date : 6/7/2016

																					i
		F	rom Nor	th			F	rom Ea	st			F	rom Sou	ıth			F	rom We	st		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. To
Peak Hour Analys	sis From (04:00 PM	to 05:45	PM - P6	eak 1 of 1		-													•	
Peak Hour for Ea	ch Approa	ach Begir	ıs at:																		_
	05:00 PM					04:00 PM					04:00 PM					04:15 PM					1
+0 mins.	44	0	24	0	68	21	129	0	0	150	0	0	0	0	0	0	136	36	0	172	
+15 mins.	50	0	25	0	75	30	108	0	0	138	0	0	0	0	0	0	134	30	0	164	
+30 mins.	56	0	41	0	97	27	101	0	0	128	0	0	0	0	0	0	134	36	0	170	
+45 mins.	37	0	22	0	59	19	110	0	0	129	1	0	0	0	1	0	125	38	0	163]
Total Volume	187	0	112	0	299	97	448	0	0	545	1	0	0	0	1	0	529	140	0	669	
% App. Total	62.5	0	37.5	0		17.8	82.2	0	0		100	0	0	0		0	79.1	20.9	0]
PHE	835	000	683	000	771	808	868	000	000	908	250	000	000	000	250	000	972	921	000	972	



Site Reference: 160070000795

Site ID: 2100000000101

Location: RTE. 1 SCUTH OF PARSONS DR.

Direction: ROAD TOTAL

STA.1

TOTA L

File: SPD1-3182.prn

City: VINNIN SQUARE STUDY

County: SPEED NES

TIME	19	24	29	34	39	44	19	\$4	59	54	69	74	79	85	86+	Cota
13:00	5	54	295	530	315	41	3	Ð	0	0	0	Q	J	0	Ů.	1244
14:00	11	50	233	526	261	39	2	-9-	1	0	0	0	0	0	Ç.	1213
15:00	1.5	91	403	567	234	3 €	Ċ	e	0	C·	0	0	0	C	C	1344
16:00	T	78	381	569	322	42	2	C	0	0	0	D	0	0	C	1407
17:00	. 9	74	394	663	334	36	Ċ	0	0	C	0	0	0	Q.	Q	1510
18:CC	5	62	439	658	353	33	2	· C	0	C	0	0	0	9	J	1572
19:0C	16	77	330	612	357	40	2	0	0	C	. 0	O.	0	Ð	0	1434
20:00	2	37	263	445	228	26	2	9	0	G	0	0	0	0	J	2003
21:00	3	51	191	280	158	2:	0	Ð	0	C	0	0	0	-0-	3	704
22:00	0	21	107	211	143	29	2	O.	0		0	0	0	0	Ĵ	513
23:00	2	9	52	123	e c	16	4	1	0	0	0	0	0	0	- 3	284
24:00	1	5	37	52	50	10	1	1	0		0	0	0	0	-J	157
DAY TOTAL	 76	639	3192	5233	2855	367	20	2	i	D			0	0	0	12385
FERCENTS	0.7%		25.6%	-		2.9%	0.15	C.Ji	0.0%	C.C3	0.0%	0.0%	0.09	C. C4	D. 36	1005

Statistical Information...

15th Percentile Speed 25.8 mph

Median Speed 31.2 mgh

10 HPH Pace Speed
24 mph to 34 mph
6425 vehicles in pace
Representing 68.0% of the total vehicles

85th Percentile Speed 36.4 mph

Average Speed 31.0 mph

Vehicles > 65 MPH 0 0.0%

MassDOT Highway Division SPEED SUMMARY Tue 4/12/2016

Page: 10

Site Reference: 160070000795

Site ID: 1100C0000101

Location: RTE. 1 SOUTH OF PARSONS DR. Direction: ROAD TOTAL

File: SPD1-0102.prn

City: VINNIN SQUARE STUDY

County: SPRED NGS

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01 00																
01:00	D	3	13	28	21	11	2	0	0	1	0	0	Ω	0	0	
02:00	0	0	8	12	9	6	0	T.	0	5	0	0	() ()	0 D	0	
03:00	0	0 2	3	9	15	7	0	0	0	0	0	0	0	0	0	
	2	9	a	17		11	5		ő	0	0	0	0		0	
05:00	2	7	31	82	32 116	37	7	2	ő		. 0	0	U		0	
06:00	2	27	176	400	308	56	11	_	0	0	0	ŏ	0	0	ő	282
08:00	12	55	387	726	335	29		. 0	0	0	0	Ô	D	G.	0	980 1545
09:00	13	62	359	636	305	28	1	ő	Û	٥	5 O	0	- I	D	0	1407
10:00	1.3	45	226	465	255	36	2	ő	. 0	ő		Ď	0	0	0	1030
11:00		73	268	426	244	34	2	ő	· 0	ő	ŏ	Û	Ó	0	0	1051
12:00	5	47	232	461	285	33	5	1	Č	ŏ	ő	0	0	័	0	1069
13:00	5	106	463	554	184	30	2	ō	G	ŏ	Õ	0	0	0	ŏ	1344
14:00		72	356	542	215	25	1	Ö	G	0	0	0	ų Đ	П	ŏ	1216
15:00	7	59	373	526	274	24	Ô	0	a	- 0	0	0	0	0	0	1263
16:00	ś	B 6	418	621	298	19	2	ŏ	a	٥	0	0	0	0	0	
17:CO	10	85	467	625	257	29	2	ŏ	0	0	Ó	Û	0	0	. 0	1475
18:CO	10	54	379	683	382	28	- 6	ŏ	C	. 0	ŏ	0	ů.	Ó	0	1537
19:00	í	46	346	642	329	39	1	1	Ċ	ŏ	ŏ	0	ŏ	ä	Ď	1408
20:00	4	51	237	437	244	35	3	ō	Č	ŏ	ŏ	0	ŏ	o o	0	1007
	· ·	34	159	317	155	18	3	ŏ	G	. 0	0	0	ŏ	5	. 0	
21:00	0	39	136	208	133	30	0	0	G	Ö	. 0	0	· ö	5	. 0	
23:00	٥	20	49	112	89	15	1	ő	0	ó	ŏ	0	0	0	0	
24:00	0	20 5	37	59	65	23	3	ŏ	٥	1	ő	0	o o	0	Ď	
24:60	U	3	11	23	93	23	J		U	_			Ů			132
DAY TOTAL	91	982	\$135	8593	4557	604	65	б	G	2	0	0	0	0	0	20035
PERCENTS	0.5%		25.7%			3.0%	0.3%	0.0%	0.0%	0.08	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information ...

15th Percentile Speed 25.9 mph

Median Speed 31.2 mph

10 MPH Pace Speed 24 mph to 34 mph 13728 vehicles in pace Representing 68.5% of the total vehicles 85th Percentile Speed 36.5 mph

Average Speed 31.1 mph

Vehicles > 65 MPH 0.0%

MassEOT Highway Division SPEED SUMMARY Wed 4/13/2016

Page: 11

Site Reference: 160070000795

Site 10: 110000000101

Location: RTE, 1 SOUTH OF PARSONS DR. Direction: ROAD TOTAL

File: SPD1-0102.prn City: VINNIN SQUARE STUDY

County: SPEED N&S

TIME	19	24	29	34	39	44	49	54	59	64	69	34	79	85	86+	Tota
						-										
01:00	1	0	18	38	28	12	3	1	0	0	b	0	0	0	0	10
02:00	0	4	9	16	13	5	1	0	0	0	Ç	0	0	0	0	41
03:00	0	0	3	В	10	5	2	-0	0	0	0	0	0	Û	0	25
04:00	0	0	3	6	14	11	1	-0	0	Q	O.	0	ዑ	Q	C	3:
05:00	1	1	9	13	27	13	8	3	0	0	0	0	0	0	C	75
06:00	2	5	42	88	114	41	13	-0	Q	0	0	0	O	0	C	30:
07:00	6	17	99	335	413	83	5	0	0	0	0	Q	0	Q	C	956
08:00	15	57	363	731	347	31	2	2	0	Û	0	0	D-	a	0	1546
09:00	4	30	261	674	393	51	3	0	0	Û	0	0	Ū.	O	0	1424
10:00	6	22	190	437	340	47	4	1	0	0	0	0	0	0	ū	104
11:00	9	54	223	475	319	45	5	0	D	0	0	0	Ū.	0	Q	113
12:00	12	48	253	513	302	45	2	Ð	D	0	0	0	D-		0	117:
13:00	11	48	342	536	296	41	2	0	0	1	0	0	Ð	Ū	. 0	127
14:00	6	60	280	547	321	27	3	0	0	Q.	0	0	-0	0	-0	1244
15:00	36	87	356	550	293	20	4	0	0	0	0	0	-0	0	-0	134
16:00	50	74	419	551	349	34	4	1	C	- 0	0	0	0	0	0	145
17:00	49	84	370	639	359	4.9	3	0	1	0	0	0	Ð	0	-0	155
18:00	58	97	450	649	309	26	2	0	G	0	0	0	0	D	0	1593
19:00	2	67	384	601	338	22	1	0	G	0	0	0	0	0	0	1413
20:00	6	44	280	473	250	25	3	0	Q	10	0	G	0	-0	0	1083
21:00	4	39	182	369	174	19	1	0	0	D-	0	G	0	- 0	0	781
22:00	2	22	120	250	109	16	3.	1	-0	0	0	0	. 0	Ð	0	523
23:00	Ð	14	59	120	103	10	2	33.1	-0	0	0	Ċ	O.	0	0	32
24:00	Ð	2	32	59	67	27	3	1	0	0	0	Ū	٥	0	0	19
AY TOTAL	250	884	4747	8686	5288	713	80	11	1	1	0				0	2066
ERCENTS	1.3%			42.1%		3.4%	0.3%	0.0%	0.08	0.00	0.0%	0.08	0.0%	0.0%	0.0%	100

Statistical Information...

15th Percentile Speed 26.1 mph

Median Speed 31.6 mph

10 MPH Pace Speed
29 mph to 39 mph
13974 vehicles in pace
Representing 67.6% of the total vehicles

85th Percentile Speed 36.8 mph

Average Speed 31.4 mph

Vehicles > 65 MPH D 0.0%

MassDOT Highway Division SPEED SUMMARY Thu 4/14/2016

Page: 12

Site Reference: 160070000795

Site ID: 110000000101

City: VINNIN SQUARE STUDY

County: SPEED N&S

File: SPD1-0102.prn

Location: RTE. 1 SOUTH OF PARSONS DR. Direction: ROAD TOTAL

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
																
01:00	0	4	12	E 25	34	19	1	0	0	0	0	0	0	0	0	95
02:00	0	1	6	25	21	14	1	0	0	0	0	0	0	0	0	68
03:00	0	3	5	6	16	0	0	0	0	0	0	0	0	0	0	30
04:00	0	4	5	9	16	10	0	0	0	0	0	0	0	0	0	44
05:00	0	3	8	13	27	20	3	0	1	0	0	0	0	0	0	75
06:00	1	3	34	84	109	52	9	0	0	0	0	0	0	0	0	292
07:00	3	15	106	330	394	78	4	1	0	0	0	0	0	0	0	931
08:00	7	62	332	722	412	37	4	0	0	0	0	0	. 0	0	0	1576
09:00	4	33	324	643	396	42	1	1	0	0	0	0	0	0	0	1444
10:00	1	40	222	430	314	51	5	0	0	0	0	0	0	0	0	1063
11:00	6	36	220	456	298	42	2	0	0	0	0	0	0	0	0	1060
								To								
DAY TOTAL	22	204	1274	2743	2037	365	30	2	1	0	0	0	0	0	0	6678
PERCENTS	0.4%	3.1%	19.1%	41.1%	30.5%	5.4%	0.4%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 27.1 mph

Median Speed 32.4 mph

10 MPH Pace Speed
29 mph to 39 mph
4780 vehicles in pace
Representing 71.5% of the total vehicles

85th Percentile Speed 37.5 mph

Average Speed 32.3 mph

Vehicles > 65 MPH 0 0.0%

MassDCT Highway Division SPESD SUMMARY Mon 4/11/2016

STA INB

Site Reference: 16007000079S

Site ID: 110000000101

Location: RTE. 1 SCUTH OF PARSONS DR.

Direction: HCRTH

Lane: 1

File: SFD1-0102.prn City: VINNEW SQUARE STODY

County: SPEED N&S

TIME	19	24	29	34	39	44	49	54	5.9	64	69	74	79	85	8 5 +	1cta
													Vive			
13:00	3	23	138	284	141	23	0	Ò	0	0	ą	0	0	0	0	662
14:00	6	38	149	248	144	19	1	0	1	0	-O-	0	D	0	0	608
15:00	14	53	254	287	130	20	0	0	0	0	0	0	C	0	Ó	758
16:00	Ç.	46	219	284	151	20	0	0	0	0	Ð	0	Q	Q	0	720
17:00	4	34	220	346	198	25	Q	0	0	0	-0	0	Ç.	0	0	827
18:00	C	47	259	348	193	19	2	0.1	0	0	e	0	С	0	0	866
19:00	14	26	181	379	21T	22	2	0	0	0	0	Ç.	C	0	0	843
20:00	a	23	149	223	120	10	1	Ç.	0	0	0	Ç.	Ç	0	0	533
21:00	2	23	125	140	12	8	Ċ	C	0	0	0	C C	Ç	0	0	350
22:00	ŋ	17	46	92	57	12	ŋ	C	0	0	0	C-	D	0	0	224
23:00	1	7	36	58	41	5	1	C	0	0	3	C	D	0	0	149
24:00	·J	5	33	27	19	5	1	1	D	0	ð	C	Đ	0	0	80
DAY TOTAL	46	347			1463	188	. 7	1	1	0	Ó	3	-3	0	Q	6632
PERCENTS	0.7%	5.3%	27.64	£1.0%	22.34	2.8%	C.1%	0.04	0.0%	3.0%	1.01	0.0%	6.0%	0.0%	0.3%	1004

Statistical Information...

15th Percentile Speed 25.6 mph

Median Speed 31.0 mph

10 MPH Pace Speed 24 mph to 34 mph 4559 vehicles in pace Representing 66.7% of the total vehicles ESth Fercentile Speed 36.3 mpt.

Average Speed 30.9 mph

Vehicles > 65 MPH 0 0.0% Page: 1

MassDOT Highway Division SPEED SUMMARY Tue 4/12/2016

Page: 2

Site Reference: 160070000795

Site ID: 110000000101

Location: RTE. 1 SOUTH OF PARSONS DR.

Direction: NORTH

Lane: 1

file: SP01-0102.prn

City: VINNIN SQUARE STUDY County: SPEED NAS

Lane: 1																
TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	D	3	11	19	9	3	1	0	D	0	0	0	0	0	0	46
02:00	0	0	5	7	3	1	0	1	-0	0	Đ.	0	0	0	Q.	17
03:00	0	0	3	' 4	2	0	0	0	0	Ō	0	0	0	0	Q	9
04:00	0	2	ď	1	4	1	O	D	Đ	Q	0	Đ.	0	0	0	12
05:00	1	2	2	12	14	3	3	1	0	Q	0	0	0	0	0	30
36:00	1	7	18	35	30	12	3	0	0	0	0	0	0	0	0	114
07:00	1	18	77	99	82	23	6	Ú	0	Q	0	0	۵	0	0	306
08:00	1	32	185	274	87	16	1	J	0	O	Ð	0	0	0	0	596
09:00	11	41	196	276	129	10	2	Û	0	0	0	0	D	0	0	665
10:00	1	32	127	252	118	7	1	þ	0	0	0	0	0	0	0	538
11:0D	0	46	157	221	105	15	Q.	0	0	0	0	0	0	0	D	544
12:00	5	27	139	208	123	16	2	1	0	0	0	0	0	0	-0	521
13:00	4	79	283	271	78	13	2	0	0	0	0	0	0	0	0	730
14:00	0	41	175	242	100	11	D	0	0	0	0	O.	0	Ď	- 0	569
15:00	2	40	228	237	129	9	0	0	0	D	0	0	. 0	. 0	0	645
16:00	6	47	238	349	119	9	1	0	0	D-	0	0	0	0	0	769
17:00	2	46	296	377	114	9	1	0	0	Ð	D	0	0	0	0	845
18:00	2	31	204	394	212	17	1	0	0	Ð	Φ	0	٥	0	0	861
19:00	1	25	218	371	177	23	1	1	0	Q.	Q.	Q	0	0	0	817
20:00	C	25	143	223	123	11	Ð	0	0	-0	0	Q	þ	Q.	0	525
21:00	1	29	96	15C	59	7	2	0	Û	-0	0	0	o	O	0	344
22:00	C	34	75	89	37	8	-0	0	0	Ð	D.	0	C	0	0	243
23:00	۵	17	32	41	40	5	0	0	0	-0	C	0	0	Q.	0	135
24:00	٥	5	27	35	24	5	0	0	0	0	C	0	D	0	0	96
						<u>ā</u>										

Statistical Information...

15th Percentile Speed 25.4 mph

Median Speed: 30.7 mph

10 MPH Pace Speed
24 mph to 34 mph
7126 vehicles in pace
Representing 71.3% of the total vehicles

85th Percentile Speed 35.8 mph

Average Speed 30.6 mph

Vehicles > 65 MPH 0 0.0%

Page: 3

File: SPD1-0102.prn

County: SPEED N&S

City: VINNIN SQUARE STUDY

Site Reference: 160070000795

Site ID: 110000000101

Location: RTE. 1 SOUTH OF PARSONS DR.

Direction: NORTH

Lane: 1

TIME 19 ي. 4د 96+ Tota Ð. 0 3 5 Ç 02:00 - 0 Ð -0 + (1 03:00 Ð g . 04:00 Û ¢ C. C θ 05:00 D - 6 C C 06:00 C Ð ů. 07:00 1 16 - 0 G Ð 00:00 D -0 ¢. Ð Ó 09:00 Đ, C () ť) 10:00 3: D C ÷) 11:00 13C D. -0 J. ð. - 0 5 28 -0 12:00 ÷3 O. 13:00 Ð Ĵ Ú D. 14:00 Ď Ď Ð Ĵ D 15:00 C - 5 ò ŋ 16:00 C Û Q D 30 5B 17:00 - 0 19:00 54 46 .0 G D 19:00 D 3, 23 20:00 D D a , E 4 21:00 C-J 22:D0 P. Ò Ď Û. 23:00 0 10 - 3 Ð 24:00 - 0 Đ.

Statistical Information...

15th Percentile Speed 25.5 mph

Median Speed 30.9 mph

10 MPH Page Speed 24 mph to 34 mph 7101 vehicles in page Representing 68.7% of the total vehicles 85th Percentile Speed 36.2 mph

Average Speed 30.7 mph

Vehicles > 65 MPM 0.0%

HassDOT Eighway Division SPEED SUMMARY Thu 4/14/2016

Page: 4

Site Reference: 160070000795

Site ID: 1100000000101

Location: RTE. 1 SOUTH OF PARSONS DR.

Direction: NGRTH

Lane: 1

File: SPD1-0102.prm City: VIBNIN SQUARE STUDY

County: SPEED N&S

TIME	19	24	29	36	3.9	44	49	54	59	64	69	74	79	65	86+	Tota
												=				
01:00	Ū	6	3	15	13)	1	3	0	0	¢	3	C	0	0	48
02:00	Û	1	4	15	8	S	Q	3	0	0	0	a	Ô	Ó	0	33
03:00	3	3	5	2	4	G-	a	3	0	0	G-	Û	Ď	0	0	14
04:00	J	4	2	2	5	2	g	Ð	0	0	Ģ.	Û	j j	0	Q	15
05:00	0	3	ī	5	5	5	1	Ð	i	0	G	D	Ĵ	0	0	27
06:00	0	3	24	35	35	7	1	Ð	0	0	C-	C	Ð	0	0	105
07:00	2	1 €	\$3	93	85	12	1	Ð	0	0	C-	C	Ð	0	0	265
08:00	1	41	194	251	136	14	1	J	0	0	C-	Ċ	Ð	0	0	628
09:00	1	22	191	298	158	22	C	1	0	0	C	D	Ð	Ö	0	693
10:06	0	19	157	245	116	25	3	0	0	0	C	Đ	-Đ	0	0	565
11:C0	ڌ	17	135	236	125	17	C	0	Û	٥	Ċ	J	Ð	C-	0	534
Dati #00017																
PERCENTS	0.3¶	131 4.5%			690 23.69	116 3.9%	0.2%	0.01	0.04	0 3.0%	0.03	0.0è	0.0%	0.0%	0.0%	2927 100%

Statistical Information ...

15th Percentile Speed 25.9 mgh

Median Speed 31.3 mph

10 MPH Pace Speed
26 mph to 34 mph
1973 vehicles in pace
Representing 67.4% of the total vehicles

85th Percentile Speed 36.7 mgb

Average Speed 31.3 mgh

Vehicles > 65 MPH 0 0.0%

MassCOT Highway Division SPEED SCHMARY Mon 4/11/2016

Page: 5

Site Reference: 160070000795 Site ID: 110000000101

Location: BTE. 1 SOUTH OF FARSONS DR.

Direction: SOUTH

Lane: 2

STANSB File: SFD1-0102.prn

City: VINNIN SQUARE STUDY

County: SPEED WES

7 IME	19	24	29	34	39	44	49	54	59	51	69	74	79	85	86+	Tota
	7															
13:00	2	31	108	246	174	19	3	0	0	0	0	a	0	0	0	582
14:00	3	22	144	273	137	20	1	0	0	0	0	0	0	ò	0	605
15:00	1	38	149	230	104	14	Ĵ	0	0	0	0	0	0	Ó	0	566
15:00	7	32	168	235	171	22	2	0	Ò	0	0	0	Ö	Ö	0	667
17:00	5	40	174	317	136	11	0	0	0	0	0	0	0	Ó	0	663
18:00	5	35	160	310	160	14	0	Q	0	C	0	0	0	0	0	704
19:00	S	51	147	233	140	18	. 0	Q-	0	Ç	0	0	0	0	Ó	591
20:00	2	9	114	220	108	1.6	1	9	0	C	0	0	0	Q	0	470
21:00	1	28	76	140	86	13	0	9	D	Q	0	0	0	0	0	344
22:00	·C·	4	61	139	86	17	2	Ð.	¢	ũ	0	0	0	0	0	289
23:00	1	2	16	52	39	11	3	1	0	3	D-	0	0	0	0	135
24:00	1	3	- 14	25	31	5	2	0	D	3	O-	0	C	0	0	77
DAY TOTAL	30	292	1351	2515	1372	179	13	1	C		·	D	0	9	0	5753
PERCENTS	0.6%	5.14		43.79		3.1%	0.2%	0.0%	0.0€	ე.ე <u>ზ</u>	0.0%	0.0%	0.04	3.04	0.0%	1008

Statistical Information...

15th Percentile Speed 26.0 rph

Median Speed 31.4 aph

10 MPH Pace Speed 29 aph to 39 mph 3607 wehicles in pace Representing 67.5% of the total vehicles 85th Percentile Speed 36.6 mgh

Average Speed 31.3 oph

Vehicles > 65 NPH 0.0%

MassOOT Highway Division SPEED SUMMARY Tue 4/12/2016

Page: 6

Site Reference: 160070000795 Site ID: 1100000000101

Location: RTE, 1 SOUTH OF PARSONS DR.

Direction: SOUTH

Lane: 2

File: SPD1-0102.prn City: VINNIH SQUARE STUDY

County: SPEED NAS

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	8.5	86+	Tota
01:00	0	٥	2	9	12	8	1	0	D	1	ō	0	0	0	0	33
02:00	0	0			6	- 5	- 0	0	0	0	Ō	0	0	0	Ó	
03:60	۵	Ð	0	5	5	1	3	0	0	0	0	Ō	Ō	0	0	
04:00	Q	0	0	4	11	6	0	1	0	0	0	0	0	0	0	
05:00	1	2	- 6	5	18	8	2	1	0	0	0	0	0	0	0	
06:00	1	0		47	78	25	4	0	0	0	0	0	ò	Ó	0	
07:00	1	9	99	301	226	33	5	Û	0	0	0	0	ō	Ō	0	
08:00	11	23	202	452	248	13	0	0	0	0	0	0	0	٥	D.	
09:00	2	21	163	360	176	18	2	0	Θ	0	~ 0	0	ō.	0	D	
10:00	0	13	99	213	137	29	1	0	Đ	0	0	0	Ó	ō	D.	492
11:00	- 4	27	111	205	139	19	2	0	0	0	0	0	Q	Ó	Ō	507
12:00	0	20	93	253	162	17	3	0	0	0	0	0	0	Ó	Ō	548
13:00	1	27	180	283	106	17	0	0	0	0	0	0	0	0	0	614
14:00	5	31	181	300	115	14	1	0	0	0	0	0	0	0	D	647
15:00	5	19	145	289	145	15	0	0	0	D-	Ð	0	0	0	-0	618
16:00	2	39	180	272	179	10	1	-0	0	0	Ð	0	0	D	0	683
17:00	8	39	171	248	143	20	1	-0	0	0	Đ	0	0	0	0	630
18:00	5	23	175	289	170	11	3	0	G	0	Đ	Ç	0	0	0	676
19:00	3	21	128	271	152	16	0	0	C	-0	Đ	C	0	0	0	591
20:00	Ç	26	94	214	121	24	3	0	Ū	0	t)	Ō	ė.	0	Ō	482
21:00	3	5		167	96	11	2	0	Ü	0	0	0	Ó	Ď.	ō	347
22:00	a	5	61	119	96	22	0	٥	0	0	0	Ð	Ó	Ō	0	303
23:00	0	3	17	71	49	10	1	0	0	0	0	- 0	Ð	Ò	Ó	151
24:00	0	Ð	10	24	41	18	3	0	0	1	0	0	0	ם	Ö	97
DAY TOTAL	52	353			2631	370	39	2	0	2	0	0	Ó	0	0	10050
PERCENTS	. 0.6%	3.6%	21.9%	43.9%	26.1%	3.6%	0.3%	0.01	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%
Shabietica) Inform	ation														

Statistical Information...

15th Percentile Speed 26.5 mph

Median Speed 31.8 mph

10 MPH Pace Speed 29 mph to 39 mph 7037 vehicles in pace Representing 70.0% of the total vehicles 85th Percentile Speed 36.9 mph

Average Speed 31.7 mph

Vehicles > 65 MPH 0 0.0%

MassDOT Highway Division SPEED SUMMARY Wed 4/13/2016

Page: 7

Site Reference: 160070000795

Site ID: 110000000101

Location: RTE. 1 SOUTH OF PARSONS DR.

Direction: SOUTH

Lane: 2

TIME եԿ 86+ Tota 02:00 0 1 - 0 û O ¢ - 0 a. 03:00 Ð Э -J C D g. D D a. 04:00 G g. g Ō ¢ Q î 05:00 α C a. 06:00 3:1 g. J : O C 07:00 g. 08:00 12 31 O. п ů n 09:00 J J ű. 10:00 g Ç. Ü 11:00 ·ĵ Ď n ù J e. 12:00 B 13 13:00 - 0 Ç. ů 14:00 -6 9.2 ú a. C 15:00 g. Ð C 16:00 g. a. C Ō. -0 ū 17:00 r. 16:00 :3 Ü C - 0 ¢ ú Ď 19:00 - 0 20:00 . 4 g Ď. Ð C 21:00 Q Ċ. Q J ¢ Ú :2 D - 0 r. ú D 22:00 23:00 Ď - 4 O. ŗ. Ð Ċ Ü 24:00 Ú DAY TOTAL 354 1917 4415 3033 445 58 7 1 1 0 0 0 0 0 10333 PERCENTS

Statistical Information...

15th Percentile Speed 26.9 mph

Median Speed 2.2 mph

10 MPH Page Speed
29 mph to 39 mph
7448 vehicles in page
Representing 72.0% of the total vehicles

85th Percentile Speed 37.3 mpt

Average Speed 32.0 mph

File: SPD1-0102.prn

County: SPEED N&S

City: VINNIN SQUARE STUDY

Vehicles > 65 MPE 0.0%

MassDOT Bigaway Division SPEED SUMMARY Thu 4/14/2016

Page: 0

Site Reference: 160070900795 Site ID: 110000000101 Location: RTE. 1 SCUTH OF FARSONS DR.

Circation: SCUTE

lane: 2

File: SPD1-0102.ptm

City: VISSIN SQUARE STODY

County: SPEED NAS

1.7																
TIME	15	24	23	34	39	4.4	49	54	59	64	€9	74	79	85	35+	Tota
				UU								==				
01:00	a	0	4	10	21	12	· 0	0	Ð	0	0	C	c	G	C	47
32:00	O.	0	2	10	13	Ġ.	1	C-	-0	0	0	C	Ç	D-	Ċ	35
03:00	0	0	C	ς.	12	C	¢	Ċ	Ð	0	0	C	0	ŷ.	Ō	16
04:00	0	0	3	7	11	. 8	C	¢	Ð	0	Ò	D.	0	Ĵ	ġ	29
05:00	0	0	1	3	22	15	2	C	0	0	0	0	a	5	þ	48
05: CC	1	G.	10	49	74	45	B	C	J	0	0	0	g	2	3	187
07:00	1	1	48	237	309	66	3	1	Ĵ	0	01	D-	Ð	3	- J	666
06:00	Б	21	149	471	276	23	3	9	Ĵ	Ó	Ċ	C-	-5	Ō	á	948
09:00	3	11	133	345	23E	23	1	Ü	0	0	C	C-	ð	0	ð	753
10:00	- 4	21	65	195	198	26	2	J	0	0	C	Ç	Q.	0	0	498
21:00	3	1.9	84	220	173	25	2	0	0	0	0	Ç	0	0	0	526
CAY TOTAL	15	13	498	1546	1347	249	22	1	0	C	3	Đ	0	0	0	3751
FERCENTS	-0.4%	2.0%	13.3%	41.3%	35.9%	6.6%	0.5%	0.0%	0.0%	0.0%	0.0%	0.0%	0.01	0.0%	0.0%	1004

Statistical Information ...

15th Percentile Speed 23.6 mph

Median Speed 33.2 mpt.

10 MPH Page Speed 29 mph to 39 mph 2993 vehicles in pace Representing 77.1% of the total vehicles 95th Percentile Speed 37.9 ægh

Average Speed 33-1 mph

Vehicles > 65 MPE 0.0%

STA. 2 NB

Site Reference: 160070880873

Site ID: 110000000001 Location: RTE, 1A SOUTH OF LEGGS HILL RD.

Direction: NORTH

TIME 19. 24 3 € 86+ Tota 5 2 2 0 1 0 2 0 47 9 39 1 42 9 50 9 - 3 14:00 15:00 C 16:00 17:00 ů. 46 2 11 18:00 Ð 19:00 20:00 (i g Ü 0 0 2 21:00 22:00 23:00 Ú 8 C n ù Ç. 24:00 DAY TOTAL 267 38 78 735 2691 1416 219 26 6 1 8 2 16 8 9 5520 PERCENTS 4.9% 0.7% 1.5% 13.4% 48.6% 25.7% 4.0% 0.4% 0.1% 0.0% 0.1% 0.0% 0.2% 0.1% 0.1% 100% 9 5520

Statistical Information...

15th Percentile Speed 32.0 ach

Kedian Speed 37.11 mich

10 MPH Page Speed 34 mgh to 44 mgh 4107 vehicles in pace Representing 74.4% of the total vehicles 85th Percentile Speed 42.1 mph

Average Speed 36.3 mob

File: SPD-202.prm

County: SPEEC NB

City: VINNIN SQUARE STUDY

Vehicles > 65 HPB C. G&

SB - NO DATA

MassDOT Highway Division SPEED SUMMARY Tue 4/12/2016

Page: 2

Site Reference: 160070000873

Site ID: 1100000000201

Location: RTE. 1A SOUTH OF LEGGS HILL RD.

Direction: NDRTH

Lane: 1

File: SPO-202.prn

City: VINNIN SQUARE STUDY

County: SPEED NB

3MIT	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
				-	- -											
01:00	0	0	2	8	29	18	2	1	0	0	0	0	0	0	Ó	60
02:00	0	0	0	3	9	7	7	O		0	0	0	0	0	0	24
03:00	0	0	0	2	2	6	- 0	1	0	0	0	1	D	0	0	12
04:00	0	0	0	4	3	4	3	3	0	0	0	0	D	Q	Q.	1.
05:00	1	0	1	5	13	6	6	1	0	Ü	0	0	Ð	O	0	30
06:00	2	0	2	23	48	50	11	5	0	0	0	0	Ð	O	0	143
07:00	15	0	3	31	167	100	36	3	Ò	0	. 0	0	-0	0	D	35
00:00	38	12	38	140	329	138	23	0	0	0	0	0	-0	0	D	716
09:00	25	0	4	106	329	181	13	0	0	0	0	3	0	2	2	66
10:00	29	6	5	77	280	170	17	0	3	۵	1	0	-0	0	J	588
11:00	13	3	1.2	114	269	116	15	1	Q	۵	0	2	0	0	0	543
12:00	17	Ð	10	82	260	145	18	2	0	0	0	2	0	0	0	536
13:00	5	1	6	103	315	148	23	3	-0	Ð	O	0	0	2	O.	604
14:00	21	1	11	102	288	134	17	2	0	1	0	0	- 0	0	2	579
15:00	8	0	22	111	324	157	17	3	2	-0	0	0	0	0	1	645
16:00	25	2	23	91	319	175	28	2	0	0	0		0	0	1	660
17:00	11	1	19	113	341	140	18	0	0	1	2	1	0	0	0	647
18:00	29	2	24	146	412	172	26	. 2	0	0	0		4	Q.	. 0	817
19:00	20	1	6	75	321	203	19	4	0	1	0	0	1	3	1	655
20:00	7	1	3	68	242		20	4	0	0	0	0	O	Ů.	2	486
21:00	4	0	3	56	196		21	0	0	0	0	0	O.	()	0	371
22:00	1	0	4	48	169		11	2	2	0	0	0	0	0	Q	336
23:00	0	1	þ	22	72		15	3	0	0	Ů.	0	0	0	0	17:
24:00	0	0	2	14	51	42	16	1	0	0	D.	0	O	۵	0	120
DAY TOTAL	271	31	200	1544	4788	2501	382	43	7	3	3	 9	5	7	 9	9803
PERCENTS	2.8%	0.4%		15.8%			3.9%	0.5%	0.0%	0.0%	0.0%	0.0%	0.01	0.0%	0.0%	1001

Statistical Information...

15th Percentile Speed 32.1 mph

Median Speed 37.0 mph

10 MPH Pace Speed 34 mph to 44 mph 7289 vehicles in pace Representing 74.3% of the total vehicles 05th Percentile Speed 42.0 mph

Average Speed 36.6 mph

Vehicles > 65 MPH 33 0.3%

MassDOT Highway Division SPEED SUMMARY Wed 4/13/2016

Page: 3

Site Reference: 160070000873 Site ID: 110000000201

Location: RTE, 1A SOUTH OF LEGGS HILL RD.

Direction: NORTH

Lane: 1

File: SPD-202.prn City: VINNIN SQUARE STUDY

County: SPEED NB

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
										38						
01:00	0	0	1	5	25	23	12	1	0	0	D	0	0	0	0	67
02:00	2	1	2	1	16		2	0	0	Ð	0	0	0	0	D	29
03:00	0	D	0	0	4	6	4	1	0	-0	Ū	0	Q	0	D-	15
04:00	Ð	Ū	1	0	2	2	4	2	1	0	Ū	0	Q	0	0	12
05:00	0	Û	3	5	11	10	5	3	0	0	0	0	0	0	0	37
06:00	0	0	0	15	49	42	29	3	0	0	0	0	0	0	0	138
07:00	3	0	1	20	147	132	34	3	1	-0	0	0	0	Ō	0	341
08:00	24	15	25	93	353	195	34	3	0	0	Û	2	0	2	1	7,37
09:00	31	0	9	44	335	243	33	4	2	2	D	0	O	4	0	707
10:00	38	3	7	63	269	200	24	3:1	3	3	6	0	0	2	D	618
11:00	55	5	θ	43	266	160	14	3	0	0	Ū	O	0	2	D	556
12:00	24	7	48	240	233	40	2	0	0	0	0	1	0	0	0	595
13:00	13	8	76	275	208	39	1	0	O.	0	0	1	0	0	1	622
14:00	23	26	109	269	153	39	1	3	0	0	3	2	0	3	0	631
15:00	69	0	4	85	252	153	27	2	1	0	6	3	2	2	3	609
16:00	112	1.2	17	116	270	123	24	2	Q.	2	2	0	0	0	1	681
17:00	100	0	3	73	331	166	1.7	3	2	0	2	0	2	3	1	703
18:00	70	1	1	101	336	199	18	0	O	2	1	1	0	6	4	740
19:00	22	0	11	85	359	204	34	0	0	0	C D	0	2	0	1	718
20:00	10	1	5	87	305	135	15	0	D	0	0	Ó	0	0	0	558
21:00	0	1	15	85	191	91	11	1	0	0	0	0	0	0	O.	399
22:00	2	0	6	52	160	75	9	0	2	: O	0	0	0	0	0	306
23:00	0	0	5	33	94	62	12	3	Q	0	0	0	0	0	0	209
24:00	Đ	0	D	22	48	48	12	6	0	Ō	9	0	0	0	0	136
DAY TOTAL	597	60	357	1812	4416	2383	378	44	12	9	20	10	6	24	12	10160
PERCENTS	5.9%	0.8%	3.6%	17.9%		23.5%	3.8%	0.5%	0.1%	0.0%	0.18	0.0%	0.0%	0.2%	0.1%	100%

Statistical Information ...

15th Percentile Speed 30.4 mph

Median Speed 36.5 mph

10 MPH Pace Speed 34 mph to 44 mph 6799 vehicles in pace Representing 66.9% of the total vehicles 85th Percentile Speed 41.9 mph

Average Speed 35.5 mph

Vehicles > 65 MPH 72 0.7%

MassDOT Highway Division SPEED SUMMARY The 4/14/2016

Page: 4

Site Reference: 160070000873 Site ID: 110000000201

Location: RTS. 1A SOUTH OF LEGGS HILL RD.

Direction: SORTH

Lane: 1

File: SPC-202.pm

City: VINNIN SQUARE STUDY

County: SPEED NB

FINE	19	24	29	34	39	44	49	54	59	64	-69	74	79	95	36+	7ota
31:03	Û-	0	1	5	21	24	. 7	٤	0	2	2	0	: 0	0	9	65
02:00	D-)	J	3	10	11	- 4	5	0	0	0	- 0	G	D	D	38
03:00	G	J	·J	3	6	7	1	Q.	0	0	0	0	Ċ	D	D	17
04:60	C	1	1	3	3	12	2	1	0	0	0	0	Ū	j	0	23
05:00	C	0	1	1	6	€	3	2	2	C	0	0	Ū	D	D	21
06:00	1	2	1	11	57	47	19	7	3	1	0	0	D	Ð	Û	145
07:00	В	0	6	56	123	107	38	4	3	D	0	0	D D	0	3	342
08:00	29	2	13	76	317	252	52	C	Ð	0	0	0	- O	2	1	744
09:00	45	5	16	66	325	225	20	6	1	1	0	0	Ð	1	3	715
10:00	42	0	:	55	269	176	23	3	ij	0	0	¢	-0	1	1	570
CAY TOTAL PERCENTS	125	11 J.59	40 1.59	234 10.69	1136	867 32.4%	168 6.31	31 1.25	3	0.1%	2 0.0%	0.04	0.0%	4 0.1%	5	2680

Statistical Information...

15th Percentile Speed 33.მ ⊐ემ

Median Speed 37.9 ლები

10 MPH Page Speed 34 mgh to 44 mph 2003 vehicles in pace Representing 74.7% of the total vehicles 85th Fercentile Speed 42.9 mph

Average Speed 37.1 mph

Vehicles > 65 MFH 11 0.48

MassDOT Highway Division SPEED SUMMARY Mon 4/11/2016

Page: 9

Site Reference: 160070000780

Site ID: 110000000301

Location: ESSEX ST., SCOTH OF CAROL WAY

Direction: RCAD COTAL

STA. 3 TOTÁL

File: SPD-3-0102.prn City: VIXXIN SQUARE STUDY

County: SFEED Nas

TIME	19	24	23	34	39	44	49	54	5.9	Ε¢	69	74	79	85	86+	Tota
14:50	39	49	209	479	258	47	5	0	i Q	0	0	0	C	0	Ð	1086
15:00	53	98	305	592	269	20	2	D	0	0	0	0	Ġ	0	Ĵ	1279
16:00	82	92	293	61.5	239	28	1	D	0	0	0	0	0	0	0	1341
17:00	79	116	308	657	268	24	4	1	0	D.	0	0	J	0	- 0	1457
19:00	30	49	237	632	349	50	1	Ç.	0	Ç.	0	0	J	0	1	1349
19:00	27	45	241	547	329	31	б	C.	0	C-	0	Ò	-O	Q	0	1225
23:00	12	25	148	391	240	33	ę.	1	0	C-	0	0	-0	0	0	854
21:00	1	7	70	274	157	36	5	2	0.	Ç	0	0	Ð	0	0	552
22:00	¢	?	57	178	158	39	2	3	0	C	Ç-	0	็ป	0	0	441
23:00	2	3	33	66	88	31	2	2	0	C	Ç-	0	. · · · · · · · · · · · · · · · · · · ·	0	0	247
24:00	0	G	15	57	54	Şū	2	Ĵ	O	C	Ç.	0	0	0	0	248
DAY FOTAL PERCENTS	325 3.3%	461 4.9%	1916 19.3%	4509 45.2%		359 3.5%	34 G.3i	6 0.0%	0.04	0. Q4	C 0.64	0.0%	0 0.03	0.0%	3.0%	3979 1009

Statistical Information...

15th Percentile Speed 25.8 mph

Median Speed 31.5 mph

10 MPH Pace Speed
29 mph to 39 mph
6857 vehicles in pace
Representing 68.7% of the total vehicles

85th Percentile Speed 36.7 mph

Average Speed 31.0 mph

Vehicles > 55 MPH 1 0.09

MassDOT Highway Division SPEED SUMMARY Tue 4/12/2016

Page: 10

Site Reference: 160070000780

Site ID: 110000000301

Location: ESSEX ST., SOUTH OF CARCL WAY Direction: ROAD TOTAL

File: SPD-3-0102.prn City: VINNIN SQUARE STUDY

County: SPEED N&S

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	0	0	3	23	18	10	3	D	0	0	0	Ð	0	0	0	57
02:00	0	0	5	10	7	5	O	1	0	ð	0	0	O	0	0	28
03:00	0	0	0	7	7	4	1	Ð	0	0	0	0	G	. 0	0	19
04:00	٥	0	2	8	12	5	1	0	0	0	0	-0	0	0	0	29
05:00	٥	0	7	9	21	14	3	٥	0	0	0	0	0	0	0	54
06:00	0	2	15	77	107	44	7	1	0	0	0	0	0	Q	0	253
07:00	1	7	67	245	252	83	9	0	0	0	0	O.	0	Ü	0	664
08:00	66	77	310	553	270	30	3	0	0	0	0	0	0	O.	0	1309
09:00	49	59	252	564	240	33	2	0	0	0	٥	0	Q	· G	0	1199
10:00	27	52	191	470	310	27	3	0	0	0	٥	0	0	¢	0	1080
11:00	26	28	273	479	212	35	1	0	0	0	0	0	0	C	0	1054
12:00	25	59	254	510	200	24	1	0	0	0	0	0	0	C	1	1074
13:00	29	75	381	523	181	21	0	0	D-	0	Q	0	Q	0	0	1210
14:00	28	73	292	502	184	17	1	0	D-	0	0	0	0	0	0	1097
15:00	57	60	331	50C	196	16	1	0	D	0	C C	0	0	Ð	0	1161
16:00	77	67	338	504	215	29	3	Q	-0	0	0	0		0	0	1233
17:00	76	112	350	589	238	23	2	1	-0	0	2	0	0	0	0	1393
18:00	26	68	213	730	331	34	3	0	0	Q.	G	0	0	- 0	0	1405
19:00	25	42	227	536	314	4.4	3	Ġ	0	Q.	C	0	O.	0	0	1191
20:00	10	20	141	425	246	28	3	1	- 0	0	C	0	O.	0	0	874
21:00	Ü	18	70	308	194	30	3	1	0	0	C	0	0	0	0	624
22:00	2	9	45	213	149	30	2	1	0	Ů.	0	0	Û	0	0	451
23:00	1	1	23	121	108	23	2	1	1	0	C	0	Q	0	D	281
24:00	0	7	10	48	65	15	4	1	0	0	0	0	0	0	D	144
DAY TOTAL	525	830	3800	7954	4077	624	61		1	 G	2	0	0	0	1	17893
PERCENTS	3.0%		21.3%			3.48	0.3%	0.0%	0.0%	0.0%	0.0%	0.01	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 25.8 mph

Median Speed 31.4 mph

10 MPH Pace Speed 29 mph to 39 mph 12031 vehicles in pace Representing 67.2% of the total vehicles 85th Percentile Speed 36.6 mph

Average Speed 30.9 mph

Vehicles > 65 MPH 0.0%

MassDOT Highway Division SPEED SUMMARY Wed 4/13/2016

Page: 11

Site Reference: 160070000780

Site ID: 110000000301

Location: ESSEX ST., SOUTH OF CAROL WAY

Direction: ROAD TOTAL

File: SPD-3-0102.prn City: VINNIW SQUARE STUDY

County: SPEED N&S

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	0	D	5	18	27	5	5	3	0	0	1	Q	0	Ū	O	64
02:00	D	0	0	В	7	4	Õ	D	0	0	0	0	0	0	Q	19
03:00	0	-0	1	В	8	2	1	D-	0	D-	. 0	-0	0	Ò	0	20
04:00	Û	0	3	4	10	3	5	D-	0	0	0	-0	0	0	0	22
05:00	0	0	4	10	23	14	2	2	0	0	0	0	0	0	0	55
06:00	0	2	17	52	111	54	8	0	0	0	0	0	0	0	0	244
07:00	1	12	75	206	269	93	6	3	O	D	0	0	0	0	0	665
08:00	21	53	270	600	334	38	6	0	0	0	0	Ð	0	0	0	1322
09:00	22	51	153	531	364	52	5	Ð	Q	O-	0	0	Ð	Ð	. 0	1178
10:00	31	31	161	475	320	4.4	6	0	Q	D-	0	0	0	0	0	1068
11:00	27	45	178	504	266	51	4	7	0	0-	0	0	0	0	0	1076
12:00	20	37	190	540	318	46	4	0	0	0	Q	0	0	0	0	1155
13:00	63	77	219	570	283	33	1	0	0	1	0	2	0	0	2	1251
14:00	28	60	294	511	281	35	3	0	0	2	0	0	0	0	2	1216
15:00	94	98	338	560	201	30	3	1	1	0	0	0	0	0	D	1326
16:00	76	46	235	562	304	53	4	O	0	2	0	0	0	0	4	1286
17:00	70	63	261	607	354	53	7	0	0	0	0	2	0	0	2	1419
18:00	81	95	279	607	293	44	2	2	0	0	0	0	0	2	0	1405
19:00	49	46	229	571	249	39	5	1	0	0	0	0	0	0	0	1189
20:00	3	25	158	481	205	33	7	0	Ç	- 0	0	0	0	0	0	912
21:00	1	16	75	317	197	24	5	1	C	0	0	0	0	0	0	636
22:00	1	2	68	217	188	24	3.	1	0	0	0	O	O	Ů	0	504
23:00	3	8	19	132	118	26	3	2	0	0	0	0	0	0	D	311
24:00	e	0	6	70	71	25	4	Û	C	Ð	0	0	0	0	D	176
DAY TOTAL	591	767	3238	8161	4801	825	96	17	1	5	1		0	2	10	18519
PERCENTS	3.2%		17.58			4.58	0.5%	0.06	0.00	0.0%	0.09	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 26.2 mph

Median Speed 31.9 mph

10 MPH Pace Speed 29 mph to 39 mph 12962 vehicles in pace Representing 69.9% of the total vehicles 85th Percentile Speed 37.1 mph

Average Speed 31.4 mph

Vehicles > 65 MPH 17 0.1%

MassSOT Enghway Division SPEED SUMMARY Thu 4/14/2015

Fage: 12

Site Reference: 160070000780

Site IC: 11000000003C1

location: ESSEX ST., SOUTH OF CARCL WAY Direction: ROAD TOTAL

File: 52D-3-0102.prm City: VINNER SQUARE STUDY

County: SEEED N&S

TIME	19	24	29	34	39	64	45	54	59	64	69	74	79	35	66+	7ota
01:00	O.	C	7	31	27	7	1	1	C	0	0	Ð	D	Û	0	74
02:00	0	C	3	16	9	4	1	1	C	0	0	-0	J	Ū	0	36
03:00	1	û	2	5	5	7	0	D	C	0	0	Ð	J	Û	0	20
04:00	ū	ŋ	4	6	15	4	1	Ĵ	Ç	Ď.	0	Ð	·O	0	0	32
05:00	_ 0	Û	f	13	11	11	2	3	¢	D	0	Ð	-3	0	Ĵ	50
06:00	Û	3	16	53	94	52	9	2	C	Û	0	Ð	÷	0	J.	229
07:00	2	7	47	221	260	30	12	D	G	0	0	Ð	Ð	0	0	689
09:00	52	43	243	538	331	55	4	J	Q	C	0	0	-0	Ċ	ú	1316
09:00	16	33	193	570	370	56	10	Ú	0	0	0	0	0	Û	0	1248
10:00	13	19	126	506	319	45	5	3	2	C	0	0	-Ĵ	Û	0	1036
					1 . 57						~					1270
DAY TOTAL	84	105	545			322	45	7	2	0	0	0	2.01	0	0	1670
PERCENTS	1. Bil	2.15	13.5%	42.14	323	5.8%	0.9%	3.1%	0.0%	0.0%	0.0%	0.3%	0.0%	0.0%	0.01	1001

Statistical Information...

15th Percentile Speed 23.0 mph

Hedian Speed 32. B asph

10 MPH Face Speed 29 mph to 39 mph 345C vehicles in page Representing 74.0% of the total vehicles 85th Percentile Speed 37.9 ≕¢h

Average Speed 32.7 mph

Vehicles > 65 MPH j 0.08

MassDOT Highway Division SPEED SUMMARY Mon 4/11/2016

Page: 1

Site Reference: 160070000780

Site ID: 110000000301

Location: ESSEX ST., SOUTH OF CAROL WAY

Direction: NORTH

Lane: 1

57A.3 NB File: SPD-3-0102.prn

City: VINNIN SQUARE STUDY

County: SPEED N&S

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
			15													
14:00	1	16	105	263	146	26	1	0	0	0	0	0	0	0	0	558
15:00	3	44	165	356	92	3	2	0	0	0	0	0	0	0	0	665
16:00	8	20	151	379	116	14	1	0	0	0	0	0	0	0	0	689
17:00	6	35	147	389	142	16	2	1	0	0	0	0	0	0	. 0	738
18:00	1	15	121	408	187	20	0	0	0	0	0	0	0	0	1	753
19:00	5	15	125	325	188	20	4	0	0	0	0	0	0	0	0	682
20:00	4	6	92	243	146	15	1	1	0	0	0	0	0	0	0	508
21:00	1	2	39	177	82	21	3	1	0	0	0	0	0	0	0	326
22:00	0	4	35	97	74	19	1	0	0	0	0	0	0	0	0	230
23:00	2	2	15	44	56	14	1	1	0	0	0	0	0	0	0	135
24:00	0	0	7	28	30	6	1	0	0	.0	0	0	0	0	0	72
DAY TOTAL	31	159	1002	2709	1259	174	17	4	0	0	0	0	0	0	1	5356
PERCENTS	0.6%	3.0%	18.8%	50.6%	23.5%	3.2%	0.3%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 27.1 mph

Median Speed 31.8 mph

10 MPH Pace Speed
29 mph to 39 mph
3968 vehicles in pace
Representing 74.0% of the total vehicles

85th Percentile Speed 36.6 mph

Average Speed 31.7 mph

Vehicles > 65 MPH 1 0.0%

MassDOT Highway Division SPEED SUMMARY Tue 4/12/2016

Page: 2

Site Reference: 160070000780

Site ID: 110000000301

Location: ESSEX ST., SOUTH OF CARGL WAY

Direction: NORTH

Lane: 1

TIME 86+ Tota 01:00 02:00 C 03:00 Ō Δ O Ð O ä G a - 0 a Ð 04:00 O. Û Û ¢ Ð 05:00 Q. O ø Û Ð Û 06:00 O В 4.0 Λ Ò Ô Ů. a 07:00 Ö Ó Ó 06:00 Ō Û Ó Ô 09:00 Ů. O Ω n 10:00 В ø 11:00 Ō - 0 12:00 C Û 13:00 C ø Ò 14:00 Ó 15:00 В D-Ō. Ď Ò 16:00 O Ď 17:00 a 18:00 19:00 Û ø Ů. Û Ó 20:00 Û Ó 21:00 Û 22:00 Đ. ñ ø ø Ô 23:00 0· Ō Ó D D Ω 24:00 DAY TOTAL

Statistical Information ...

PERCENTS

15th Percentile Speed 27.0 mph

Median Speed 31.7 mph

10 MPH Pace Speed 29 mph to 39 mph 6610 vehicles in pace Representing 72.2% of the total vehicles 85th Percentile Speed 36.6 mph

Average Speed 31.7 mph

File: 5PD-3-0102.prn

County: SPEED NAS

City: VINNIN SQUARE STUDY

Vehicles > 65 MPH 1: 0.0%

MassDOT Highway Division SPEED SUMMARY Wed 4/13/2016

Page: 3

File: SPD-3-0102.prn

County: SPEED NAS

City: VINNIN SQUARE STUDY

Site Reference: 160070000780

Site ID: 110000000301

Location: ESSEX ST., SOUTH OF CAROL WAY

Direction: NORTH

Lane: 1

TIME 86+ Tota n 01:00 D 02:00 Θ O-03:00 - 1 n. D-ũ 04:00 05:30 θ Ū. O-D П (i) п (î) 06:00 07:00 Û. . 0 D 08:00 Û D D . O 09:00 Ò D Π Ď 10:00 11:00 ñ D-Ω D-Ω 12:00 D· D-Û Û Q 13:00 Û 14:00 Ð D-O Ð П D П 15:00 ŀ D D Ū. Ū D-16:00 D D П D 17:00 O - 1 18:00 4 3 19:00 - 3 D D-Ò Û Ó 20:00 21:00 22:00 П Ò Δ Û 23:00 Ω Ω Ω 24:00

Statistical Information...

15th Percentile Speed 27.8 mph

Median Speed 32.2 mph

10 MPH Pace Speed
29 mph to 39 mph
7219 vehicles in pace
Representing 75.6% of the total vehicles

85th Percentile Speed 37.2 mph

Average Speed 32.4 mph

Vehicles > 65 MPH 0.0%

MassDCT Highway Division SPEED SUMMARY Thu 4/14/2015

Page: 4

Site Reference: 160070000780

Site ID: 1100000000301

Location: ESSEX ST., SOUTH OF CAROL WAY

Direction: MCRTH

Lane: 1

File: \$90-3-0102.pm City: VINNIR SQUARE STUDY

County: SPEED N&S

	TEME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	95+	Îota
-																	
	01:00	Û	С	5	17	19	2	1	0	0	g	C.	0	Ç.	0	9	44
	02:00	Û-	C	2	11	1	1	1	0	g	Ĵ	Ċ	0	C-	0	3	16
	03:00	Ç-	G	0	3		6	-Ç	- 0	Ĵ	. 3	Q.	· 0	Ç.	Q	0	10
	04:00	C	Q	2	5	3	3	Ð	0	Ð	0	Û:	0	Ċ	Ó	- 0	18
	05:30	Ç	0	Ţ	5	10	5	Ð	3	Ð	0	Û	0	C	0	0	24
	06:00	C	1	5	22	39	13	7	2	0	0	Ç-	0	C	Û	0	95
	07:00	C	J	16	109	100	35	9	0	:I	0	C	C·	Û	0	0	271
	06:00	2	:0	119	313	500	25	O.	0	Ð	0	C	Ç	J	0	0	669
	09:00	1	2	80	324	214	32	2	0	ij.	0	C	Ç	J	0	0	655
	10:00	1	Û	46	275	156	21	0	0	2	0	5	C	Ĵ	0	0	501
												9					
-	AY TOTAL	٠	13	279	1094	746	143	20	5	2	()	3	0	0	С	0	2303
	ERCENTS	0.28		_		32.5%	6.48	0.Bî	0.29	0.0%	0.03	0.03	0.0%	0.0%	0.09	0.0%	100%

Statistical Information...

15th Percentile Speed 29.2 mph

Hedian Speed 33.0 თებ

10 NPH Page Speed 29 agh to 39 mgh 1812 vehicles in page Representing 79.5% of the total vehicles 85th Percentile Speed 37.9 mgh

Average Speed 33.3 mph

Vehicles > 65 MPR Ó. 0.0%

MassDO7 Highway Division SPEED SUMMARY Mon 4/11/2016

Site Reference: 160070000780

Site ID: 1100000000301

Location: ESSEX ST., SOUTH OF CARCL WAY Direction: SOUTH

Lane: 2

5TA.35B File: SPO-3-0102.prn City: VINNIN SQUARE STUDY

County: SPEED NeS

TIME	19	24	29	34	39	44	49	54	59	64	65	74	79	35	86+	Tota
														=		
14:00	36	33	104	216	112	21	4	o o	Ò	.0	0	0	0	.û	С	52B
15:00	50	54	140	236	117	17	C	J	0	0	0	Ç	0	¢	Ċ	614
16:00	74	62	142	237	123	14	C)	0	0	0	0	0	£	Ċ	652
17:00	73	81	161	268	126	Ġ	5	0	0	0	0	C	0	¢	ŋ	719
13:00	29	34	116	224	162	10	1	0	0	0	0	C	0	Ð	J	596
19:00	22	30	116	222	140	11	2	0	0	Û	0	ŋ	0	D	ĵ	543
20:00	8	19	56	146	94	18	3	0	0	0	0	J	0	Ð	0	346
21:00	C	5	31	97	75	15	2	1	0	0	0	Ĵ	Ç	-0	0	226
22:00		3	22	81	84	20	1	0	0	0.	0	Ĵ	0	Ð	0	211
23:00	ĵ	1	16	42	32	17	1	1	0	0	0	Ð	Ç.	อ	- 0	112
24:00	D	0	Е	29	24	14	1	0	D	0	0	Ð	C	ú	. 0	76
DAY TOTAL	294	322	914	TEOC	1089	165	17	2	0	0	0	0	0	0	0	4623
PERCENTS	6.48	7.0%	19,8%	39.0%	23.59	4.0%	0.3%	0.0%	0.0%	0.0%	C-0%	0.0%	0.0%	0.01	1.0%	100%

Statistical Information...

15th Percentile Speed 24.4 mph

Median Speed 31.2 mph

10 MFH Page Speed 29 cpt to 39 mph 2009 vehicles in pace Representing 62.4% of the total vehicles 85th Fercentile Speed 36.8 mph

Average Speed 30.1 cph

Vehicles > 65 MPH D. 0.0%

Page: 5

MassDOT Highway Division SPEED SUMMARY Tue 4/12/2016

Page: 6

Site Reference: 160070000780

Site ID: 110000000301

Location: ESSEX ST., SOUTH OF CAROL WAY

Direction: SOUTH

Lane: 2

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	96+	Tota

02:00	0	C	0	7	12	6-	3	Ç	0	0	0	Q.	0	C	0	28
02:00	0	C	1	6	3	2	0	1	0	Q.	0	0	0	0	0	13
03:00	0	Ū	0	2	5	1	1	C	0	0	0	0	D	0	0	9
04:00	. 0	0	2	6	6	2	1	C	a	0	Q	0	0	0	0	ÌΊ
05:00	0		4	Ĵ	10	10	1	C	0	0	0	D-	0	0	0	32
06:00	0	2	7	37	71	25	3	C	0	Û	0	0	0	0	0	L45
07:00	0	7	34	127	147	51	4	C	0	Ð	0	0·	0	0	0	370
06:00	65	63	139	262	144	14	1	0	0	0	0	D-	D	0	0	688
09:00	48	57	137	235	169	15	2		0	Ð	0	0	0	0	0	603
10:00	27	44	101	225	152	9	2	0	0	£.	0	Ð	0	Û	0	560
11:00	26	24	135	227	101	19	-C	0	0	O	0	D-	0	Ü	0	532
12:00	25	49	147	215	94	15	1		0	C.	0	9	Ů.	0	1	546
13:00	29	57	156	221	93	11	C	D	0	0	D	9	C	0	0	567
14:00	26	60	144	231	99	7	1		0	· C	0	9	Ç.	0	0	568
15:00	56	52	135	202	92	12	1	Ĵ	Q	C	0	0	C	0	0	550
16:00	77	55	142	219	105	15	2	Ĵ	0	C	Q.	0	Ð	0	0	615
17:00	73	76	169	248	117	12	0	1	Q	G	1	÷	£.	0	0	697
18:00	25	47	105	289	139	11	3	Ĵ	0	C	D	0	£	0	0	619
19:00	25	36	115	189	143	10	1	Ĵ	0	·C	D.	G	C	0	0	519
20:00	10	11	54	168	121	19	2	1	0	0	0	0	c	0	0	386
21:00	D	10	36	142	72	15	2	1	Q	0	0	0	C	0	0	208
22:00	2	5	23	94	62	10	1	0	Q	-0	O	G	Ð	0	0	197
23:00	0	0	10	61	47	6	1	0	Q	0	D.	G	-0	0	0	127
24:00	0	0	2	22	35	6	2	1	0	Ð	0	q	-0	0	0	68
DAY TOTAL	514	654	1799	3442	1979	305	35	 5	0		1		a	0	1	8734
PERCENTS	5.99		20.6%			3.4%	0.48	0.0%	D.0%		0.09	0.0%	0.0%	0.D%	D. 0%	1005

Statistical Information...

15th Percentile Speed 24.4 mph

Median Speed 31.0 mph

10 M2H Pace Speed
29 mph to 39 mph
5421 vehicles in pace
Representing 62.0% of the total vehicles

85th Percentile Speed 36.6 mph

Average Speed 30.0 mph

File: SPD-3-0102.prn

County: SPEED N&S

City: VINNIN SQUARE STUDY

MassDOT Highway Division SPEED SUMMARY Wed 4/13/2016

Page: 7

Site Reference: 160070000780

Site ID: 110000000301

Location: ESSEX ST., SOUTH OF CAROL WAY

Direction: SOUTH

Lane: 2

File: SPD-3-0102.prn City: VINNIN SQUARE STUDY

County: SPEED N&S

TIME	19	24	29	34	43	44	44	54	59	PĘ	69	74	73	35	86÷	Tota
01:00	0	G	4	В	3	4	2	0	0	0	1	Ð	0	0	D	28
02:60	0	¢	0	4	3	4	0	0	0	0	Ç	Đ.	0	0	9	11
03:00	0	3:	1	4	4	0	0	0	0	0	C	Ð	0	0	-0	9
04 : GD	0	3:	2	3	- 5	1	0	0	0	0	G	·G	0	0	- 0	12
05:60	Q	3	2	5	15	- 5	0	1	0	0	3	e	0	0	-0	28
06:CD	0	1	E	34	71	29	4	0	0	0	3	C	0	0	0	147
07:00	4	10	41	122	163	51	4	0	0	0	3	C	0	Ü	e	392
08:00	20	42	150	269	150	20	3	0	0	0	J	0	0	0	0	654
09:00	21	40	91	253	150	15	2	D	0	0	J	0	D	0	0	573
10:00	21	22	100	259	142	17	2	0	0	0	Ĵ	0	Ç-	0	G.	563
11:00	21	25	94	241	132	27	1	1	0	0	0	จ	C	0	C	542
12:00	19	29	115	249	155	17	2	D	Q-	Ú	Ú	0	C	0	C	586
13:00	59	56	120	239	121	10	1	Ū.	0	1	- O	2	C	Q	2	610
14:00	26	53	131	250	131	15	D	D	9	2	0	Q	0	0	2	512
15:00	91	74	144	206	E 9	16	1	1	1	Q	0	Q	0	0	0	623
16:00	76	42	145	233	120	21	3	C·	9	2	0	0	Ū	0	4	646
17:00	€8	54	130	278	134	23	5	C	9	0	0	2	9	0	2	696
16:00	60	80	117	225	115	17	1	2	3	D	0	0	1	0	0	637
19:00	45	42	96	196	99	19	2	C	0	0	. 0	0	3	0	ŋ	499
20:00	3	22	54	154	108	16	4	C	0	0	. 0	0	Ĵ	0	Q	373
21:00	£	9	36	124	85	14	3	C	0	0	0	0	Ĵ	0	0	271
22:00	-C	0	30	96	86	13	2	G	û	Q	0	0	Ĵ	D	0	227
23:00	3	Ţ	6	63	60	12	Ð	1	ű	D	0	0	Ĵ	0	0	152
24:00	0	0	1	38	34	10	4	0	. ¢	0	0	0	J	0	0	87
DAY TOTAL	 556	6DB	1619	3561	2182	36C	 \$6	6	1	5	1	4	0	0	10	E978
PERCENTS	€.25				26.45	4.2%	0.5%	0.0%	0.0%	0.05	0.0%	0.0%	0.00	C.D%	0.1%	100%

Statistical Information...

15th Percentile Speed 24.6 mph

Median Speed 31.4 mph

10 MPH Face Speed
28 mph to 39 mph
5743 vehicles in pace
Representing 63.9% of the total vehicles

B5th Percentile Speed 37.0 mph

Average Speed 30.4 mph

Vehicles > 65 MPH 15 0.2%

MassDOT Righway Division SPEED SUMMARY Thu 4/14/2016

Раче: 8

Site Reference: 160070000780

Site ID: 110000000301

Location: ESSEX ST., SCUTH OF CARCL WAY

Cirection: SCUTH

lane: 2

TIME 19 24 29 34 39 44 45 54 59 64 69 11.74 19 85 86+ Tota 01:00 Ú 0 2 14 н ú 0 Ð J 0 1 ũ 30 02:30 Ü 0 - 7 В 3 ú 1 0 0 Ð Ď. 0 Ľ. O. 20 03:30 2 2 1 0 ζ 0 Ç 0 ij. Ð Ò è 0 10 04:00 G - 0 2 3 :: 7 0 ij. 1 Ĵ ű. Ç C Ó 14 05:00 ¢ - 0 ē 6 2 0 ú ú 0 С 26 1.2 06:00 Ċ 103 31 55 34 2 Ũ 0 Ü ¢ C 134 07:00 Z 29 112 160 45 0 0 ō. ¢ C Ĵ -3 358 08:00 50 31 124 225 :01 30 4 Ĵ 0 0 O 0 0 0 0 EKT 09:00 15 31 113 246 156 24 Ð 8 0 0 0 Q. 0 ð. 0 593 10:00 12 80 231 163 25 0 D D O. n 535 60 DAY TOTAL PERCENTS

Statistical Information...

15th Percentile Speed 26.5 mph

Median Speed 32.7 mph

10 MPH Pace Speed
29 mph to 39 mph
1626 vehicles in pace
Representing 69.7% of the total vehicles

85th Percentile Speed 33.0 mph

Average Speed 32.1 mph

File: SPD-3-0102.prm

County: SPEEC NAS

City: VINNIH SQUARE STUDY

Vehicles > 65 KPH C 0.01

MassDOT Eighway Division SPEED SUMMARY Mon (/11/2016

Page: 9

Site Raference: 160070000479

Site ID: 110000000401

Location: SALEM ST., SOUTE OF VEHNEN ST.

Direction: 50A0 TOTAL

STA.4 TOTAL

File: SPD-4-0102.ptp City: VINNIW SQOARE STODY

County: SPEED NIS

SKIT	19	24	29	34	39	44	49	54	59	કેર્દ	69	74	73	85	36+	Tota
16:00	20	108	193	77	6	0	1	0	0	- 0	C	C-	0	Ð	Û	405
15:00	79	1.54	196	84	4	J.	0	0	0	0	C	G	0	-O	Û	517
16:00	54	L86	234	52	ī	ว	0	0	C	0	G	<u>G</u>	0	0	G-	513
17:00	33	133	189	99	9	Q.	0	0	Ç	0	0	C-	0	0	C-	463
19:00	18	96	192	116	15	0	0	0	C	0	Ū	C-	0	0	G-	437
19:00	35	109	175	87	ī	0	0	0	0:	0	0	G-	0	g	C	414
20:00	9	54	146	15	6	9	-	Q.	0	ୃ 0	0	Ċ-	0	0	G	391
21:00	3	20	97	54	7	- 0	0	0	C	0	D	C	0	0	Ç	181
22:00	2	16	40	44	٤		0	0	0	0	0	¢	Ò	Q	¢	11L
23:00	1	6	15	20	1		Ċ	0	Û	0	Û	C	0	0	G	48
24:00	3	7	8	13	4	0	0	0	0	0	a	C	0	0	C	3.5
DAY TOTAL	256	679	1489	771	74	. 2	2	Û	Ç	0	J	C	0	0	C	3425
PERCENTS	7.6%	25.75	43.5%	21.1%	5.18	0.3%	0.0%	G.0%	0.0%	0.0%	0.0%	0.6%	0.0%	0.0%	0.05	1991

Statistical Information...

15th Percentile Speed 20.5 mph

Median Speed 25.9 mph

10 MPH Pace Speed 19 mph to 29 mph 2368 vehicles in pace Representing 69.1% of the total vehicles 35th Percentile Speed 31.0 mph

Average Speed 25.2 mph

MassDOT Highway Division SPEED SUMMARY Tue 4/12/2016

Page: 10

Site Reference: 160070000479

Site ID: 110000000401

Location: SALEM ST., SOUTH OF VINNIN ST.

Direction: ROAD TOTAL

File: SPD-4-0102.prn City: VINNIK SQUARE STUDY

County: SPEED NAS

TIME	19	24	59	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	2	0		1	0	0	0	0	0	D	O	0	0	0	Ó	7
D2:00	0	1	5	2	0	D	0	0	0	0	0	0	0	0	0	8
03:00	0	Ð	1	D	1		0	0	Q.	0	0	0	0	0	0	2
04:00	0	0	Q	0	1		0	0	a	-0	0	0	0	0	0	1
05:00	2	4	6	3	2	0	0	Q	0	. 0	0	Q	0	O	0	17
06:00	0	q	15	11	2	ŋ	Ó	Ů.	0	0	0	O	0	0	0	32
37:00	4	21	56	45	5	Û	0	0	0	0	0	0	0	0	0	131
08:00	37	106	190	79	7	J	0	0	0	0	0	0	0	0	0	415
09:00	38	119	180	57	5	0	0	0	Ū	0	0	Q	O	0	0	399
10:00	27	94	173	7.3	3	1	0	0	0	0	0	0	0	0	0	371
11:00	41	104	176	42	6	0	0	0	0	0	0	0	٥	D	0	369
12:00	22	111	167	67	4	0	0	0	Û	0	0	0	0	0	0	371
13:00	50	151	228	64	5	1	0	0	Ð	0	Ů.	0	0	0	0	519
14:00	26	134	168	55	1	0	G	0	Ð	0	Ō	0	0	0	0	384
15:00	41	111	177	63	3	0	O	O	0	0	0	Ð	0	0	0	395
16:0D	17	131	182	80	11	0	0	0	0	0	0	0	0	D-	0	421
17:00	27	133	198	84	9	0	0	Q.	0	0	D	0	0	0	O	451
19:00	38	129	207	89	8	1	0	Q.	0	0	D	0	0	0	0	472
19:00	12	94	190	109	13	0	0	D-	0	0	0	0	0	0	0	418
20:00	5	76	143	68	7	0	Ü	Ü	0	0	. 0	0	0	0	0	299
21:00	6	52	95	72	Ð	1	Û	Û	0	¢	D	0	0	0	O-	234
22:00	1	17	69	43	10	0	0	0	0	Q	D	0	0	D	0	140
23:00	1	5	14	17	5	0	0	0	0	0	0	0	0		Q	42
24:00	٥	6	12	11	2	1	0	0	0	0	0	0	0	0	q	32
DAY TOTAL	397	1603	2656	1155	118	 5			0			0	0	0	0	5934
PERCENTS		27.1%			1.9%	0.01	0.0%	0.0%	0.0%	0.01	0.0%	0.0%	0.0%	0.08	0.0%	100%

Statistical Information ...

15th Percentile Speed 20.5 mph

Median Speed 25.0 mph

10 MPH Pace Speed
19 mph to 29 mph
4259 vehicles in pace
Representing 71.7% of the total vehicles

85th Percentile Speed 30.7 mph

Average Speed 25.2 mph

MassDOT Highway Division SPEED SUMMARY Wed 4/13/2016

Page: 11

Site Reference: 160070000479

Site ID: 110000000401

Location: SALEM ST., SOUTH OF VINNIN ST.

Direction: ROAD TOTAL

File: SPD-4-0102.prn City: VINNIN SQUARE STUDY

County: SPEED NAS

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
	70															
01:00	0	1	5	3	2	٥	0	0	0	0	0	0	D	a	0	11
02:00	0	0	2	2	0	٥	0	0	0	0	0	Ū	0	0	0	4
03:00	0	0	5	3	1	¢	୍ 0	0	0	0	0	0	0	Ó	0	9
04:00	Q	2	1	1	1	0	0	0	0	0	0	0	Ò	Ó	0	5
05:00	0	0	2	4	1	0	0	0	Q	0	0	0	0	0	0	7
06:00	3	2	Z1	19	2	1	0	0	Q	0	O	0	0	0	0	48
07:00	10	24	53	41	10	3	Q.	0	0	0	0	Q	D	. 0	0	141
09:00	13	89	206	112	18	2	0	0	0	0	O-	Ū	0	0	0	440
09:00	32	96	200	99	12	0	. 0	٥	0	0	Û	Ü	0	0	0	439
10:00	33	91	151	96	17	0	-0	0	0	0	Ů.	0	0	0	0	388
11:00	38	102	191	82	10	Ö	0	Ó	Q	Ď	Ď.	Ō	Ò	Ó	0	423
12:00	52	113	166	79	6	Ö	ō	ō	ō	Ď	Ď	ō	Ö	à	0	416
13:00	34	138	216	88	14	ō	ō	ò	Ö	Ď.	Ď	ñ	ð	ō	ō	490
14:00	33	135	216	104	8	Ď	0	ō	ō	ū	- 0	ō	ò	Ď	ō	496
15:00	81	160	249	89	5	ō	ō	Ó	ō	ō	ā	ō	0	0	ŏ	584
16:00	46	122	221	89	14	ŏ	Ď	ŏ	ŏ	Ö	ő	Ó	ő	Ó	0	494
17:00	47	135	215	110	11	Ď	Õ	ō	Ď	Ď	ŏ	ŏ	ň	ń	Ó	518
18:00	28	134	231	106	20	í	Õ	ő	ŏ	Ď	ň	ň	ŏ	Ď	ō	520
19:00	9	103	174	106	11	ī	ă	ŏ	Ö	ů	ñ	ŏ	ŏ	ő	ō	404
20:00	Đ	76	152	77	8	ō	Ð	ő	Ö	0	ō	ő	0	ŏ	ŏ	321
21:00	2	39	95	56	4	0	ű	ŏ	Ď	Ü	ō	ő	ő	o	Ö	196
22:00	0	19	72	36	3	0	ő	ŏ	Ď	0	'n	ŏ	ň	ñ	ŏ	130
23:00	1	8	24	24	5	4	ő	ŏ	Ö	Ö	ő	ŏ	ŏ	Ď	ŏ	66
24:00	Ô	1	7	- 8	5	i	ů.	ŏ	Ď	0	ŏ	ŏ	ŏ	Ď	ŏ	22
24:00	U	1	,	4	2	1	v			U	-0				0	
DAY TOTAL	472	1590	2875	1434	186	13	0	0	0	0	0	Q	0	0	0	6572
PERCENTS		24,28			2.8%	C.15	0.0%	0.0%	0.0%	0.0%	D.0%	0.0%	0.0%	0.0%	0.0%	100%
					2100	4110	2100	5.00	0.00	0.00	0.00	0.00	0104	0.00		100

Statistical Information...

15th Percentile Speed 20.6 mph

Median Speed 26.1 mph

10 MPH Pace Speed 19 mph to 29 mph 4465 vehicles in pace Representing 67.9% of the total vehicles B5th Percentile Speed 31.3 mph

Average Speed 25.5 mph

MassDOT Highway Division SPEED SUMMARY The 4/14/2016

Page: 12

Site Reference: 160070000479

Site ID: 1100000000401

location: SALEM ST., SCOTH OF VINNEH ST.

Cirection: RCAD TOTAL

File: SPD-4-C102.prm City: VIBNES SQUARE STUDY

County: SPEED NES

CIME	19	24	29	34	39	44	49	Sŧ	59	64	69	74	79	83	86+	Tota
01:00	С	1	5	6	2	0	0	0	j	D	C	0	ú	¹ 0	0	14
02:00	Ç	0	4	7	1	4	0	0	0	D	0	0	0.0	Û	Ů.	13
03:00	Ċ	0	2	1	C	0	0	0	0	D	Û	0	0	Ū	0	3
04:00	C	- 0	D	3	1	0	0	0	0	Ó	Ó	0	0.0	D	0	3
05:00	a	0	3	5	2	0	1	C-	0	. 0	Ū	0	0	D	C	11
06:30	5	1	23	12	3	1	0	C	0	j.	0	0	- 0	3	C	4.5
07:00	5	15	62	51	5	1	C·	C	0	j	Û	0	0	3	C	140
08:00	63	90	180	100	11	2	C	Û	. 0	- 0	Q	Ď.	0	J	1	116
.09:00	49	91	213	56	10	C	Ċ	Q	0	Q	Q	0	0	0	a	456
10:00	32	90	177	83	7	1	Ç	0	0	0	C	0	0	0	Û	390
DAY TOTAL	156	291	674	364	41	- 6	1	0	0	- 0	D	0	0	0	0	1531
PERCENTS	10.1%	19.19	44.10	23.0%	2.6%	0.3%	0.0%	0.0%	0.01	0.01	0.0%	0.0%	0.0%	0.01	0.0%	1005

Statistical Information ...

15th Percentile Speed 20.3 mph

Nedian Speed 26.4 pph

10 MPR Pace Speed **
24 mph to 34 mph
1036 vehicles in pace
Representing 67.7% of the total vehicles

85th Percentile Speed 31.5 mph

Average Speed 25.4 mph

MassDOT Highway Division SPEED SUMMARY Mon 4/11/2016

Page: 1

Site Reference: 160070000479

Site ID: 110000000401

Location: SALEM ST., SOUTH OF VINNIN ST.

Direction: NORTH

Lane: 1

STA 4 NB File: SPD-4-0102.prn City: VINNIN SQUARE STUDY

County: SPEED N&S

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
14:00	11	42	63	46	3	0	0	0	0	0	0	0	0	0	0	165
15:00	47	76	96	30	2	0	0	0	0	0	0	0	0	0	0	251
16:00	32	85	108	19	3	0	0	0	0	0	0	0	0	0	0	247
17:00	19	68	64	36	5	0	0	0	0	0	0	0	0	0	0	192
18:00	10	54	71	43	10	0	0	0	0	0	0	0	0	0	0	188
19:00	26	53	77	28	1	0	0	0	0	0	0	0	0	0	0	185
20:00	6	35	55	27	2	0	1	0	0	0	0	0	0	0	0	126
21:00	2	13	41	24	2	0	0	0	0	0	0	0	0	0	0	82
22:00	2	10	12	16	1	1	0	0	0	0	0	0	0	0	0	42
23:00	0	2	10	6	1	1	0	0	0	0	0	0	0	0	0	20
24:00	2	1	3	0	1	0	0	0	0	0	0	0	0	0	0	7
DAY TOTAL	157	439	600	275	31	2	1	0	0	0	0	0	 0	0	0	1505
PERCENTS		29.2%	39.9%	18.3%	2.0%	0.1%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 19.8 mph

Median Speed 25.3 mph

10 MPH Pace Speed
19 mph to 29 mph
1039 vehicles in pace
Representing 69.0% of the total vehicles

85th Percentile Speed 30.5 mph

Average Speed 24.4 mph

MassDOT Highway Division SPEED SUMMARY Tue 4/12/2016

Page: 2

Site Reference: 160070000479

\$it= ID: 110000000401

Location: SALEM ST., SOUTH OF VINNIN ST. Direction: NGRTH

Lanc: 1

File: SPD-4-0102.pcn City: VINNIN SQUARE STUDY

County: SPEED N&S

TIME	19	24	29	34	39	4.4	49	54	59	64	69	74	79	85	86+	Tota
01:00	1	0	2	0	0	0	0	0	0	0	0	0	C	0	0	3
02:00	0	0	2	1	-0	0	Ð	-0	0	0	Ð	0	G	0	0	3
03:00	0	-0	1	0	0	0	0	0	Q.	C C	0	0	0	0	0	1
04:00	0	0	0	0	1	0	- 0	0	Q-	0	-0	0	0	Q	0	1
05:00	1	2	3	3	2	0	0	0	0	Ċ	0	0	0	0	0	11
06:00	0	3	6	7	2	0	0	0	0	C	0	0	0	0	0	18
37:00	0	8	30	32	4	0	0	0	0	Ú	٥	0	-0	0	G	74
08:00	32	59	90		5	0	0	0	0	0	0	0	-0	Ô	0	222
09:00	32	64	85	26	3	0	0	0	0	0	0	0	-0	0	0	210
10:00	14	47	75	29	2	1	0	O	۵	Û	Ō	0	0	0	0	168
11:00	32	47	96	14	4	5	0	0	0	Ð	0	Q	0	0	0	183
12:00	13	52	72	35	2	D	0	0	0	Ð	0	0	0	0	0	174
13:00	34	80	91	36	0	1	0	0	0	Ð	0	Û	0	0	0	242
14:00	21	68	73	21	1	0	0	Ó	0	0	0	0	- 0	0	0	184
15:00	2B	64	67	26	0	J	0	0	0	-0	0	0	0	0	Q	185
16:00	14	63	74	32	5	0	0	0	D	-0	0		0	0	0	188
17:00	18	69	B4	32	3	0	0	0	D	0	0	0	0	0	0	206
18:00	27	69	86	29	1	1	0	0	D.	0	0	0	0	0	0	213
19:00	11	62	76	34	4	0	0	0	0	0	0	Ū	0	0	0	187
20:00	3	32	53	26	2	0	0	0	-0	0	0	Û	0	0	O-	116
21:00	3	30	40	26	2	0	0	0	0	0	0	0	0	0	Û	101
22:00	1	9	23	20	6	0	0	0	0	0	0	0	0	0	0	59
23:00	0	3	3	5	2	0	0	0	- 0	0	0	0	0	0	0-	13
24:OD	0	3	2	4	0	1	0	0	0	0	0	Ð	0	0	0	10
DAY TOTAL	205	834	1124	474	51	4	0	0	0	0						2772
PERCENTS				17.18	1.0%	0.1%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	1001
								_				_			_	

Statistical Information ...

15th Percentile Speed 19.8 mph

Median Speed 25.2 mph

10 MPH Face Speed 19 mph to 29 mph 1958 vehicles in pace Representing 70.6% of the total vehicles 85th Percentile Speed 30.2 mph

Average Speed 24.3 mph

MassDOT Highway Division SPEED SUMMARY Wed 4/13/2016

Page: 3

Site Reference: 160070000479

Site ID: 110000000401

Location: SALEM ST., SOUTH OF VINNIN ST.

Direction: NORTH

Lane: 1

File: SPD-4-0102.prn City: VINNIN SQUARE STUDY

County: SPEED Nes

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	D	_	2	2	0	0	0	0	0	0	0	0	0	0	0	5
D2:00	D		_	2	0	0	0	0	0	D	0	0	0	0	0	3
03:00	D	0	-	1	0	0	0	0	0	0	0	0	-0	0	0	5
04:00	U	Ţ	0	1	1	0	0	0	0	0	0	0	0	0	0	3
05:00	0	0		3	1	0	0	0	0	0	0	0	Ð	.0	0	5
06:00		٥		11	2	ő	0	0	0	0	0	0	Ð	0	0	28
07:00	2			28		2	0	0	0	0	0	0	Ð	O.	0	78
08:00	12			51	10	Ţ	0	0	0	0	0	0	0		0	227
09:00	26			43	8	o.	0	0	0	0	. 0	0	0	D	0	219
10:00	27	45		44	7	0	0	0	0	0	0	0	0	D	0	186
11:00	23		-	40	3	0	0	0	Ů.	0	0	0	Ó	0	D	197
12:00	15		_	28	1	0	0	0	C	0	0	0	٥	0	0	188
13:00	23	64		41	7	0	0	0.	Ü	0	0	C	0	0	0	220
14:00	14	69		51	6	0	0	0	ç	0	0	i i	٥	0	0	232
15:00	54	69		37	2	0	Ð	0	C	0	0	C	Ò	0	0	276
16:00	39			37	6	0	0	0	0	0	0	0	0	0	0	243
17:00	34	67		34	3	0	-0	0	C	0	0	0	Ō	a	0	229
18:00	19			45	7	1	0	0	0	0	0	0	0	Ō	0	246
19:00	8	59		36	2	1	0	0	0	0	0	0	0	0	0	183
20:00	5	36		34	2	0	0	0	0	0	Ū	0	0	0	0	140
21:00	2	20	_	20	3	0	0	0	0	0	Ó	0	0	0	0	82
22:00	Ü	10		12	Ţ	0	0	0	0	٥	0	0	0	0	0	50
23:00	Ţ	3		11	3	3	0	0	0	0	0	0	0	0	0	31
24:00	C	1	2	0	2	1	0	0	0	0	0	0	0	0	0	6
DAY TOTAL	305	815	1257	612	84	9	0		0	0	0	0	0	0	0	3082
PERCENTS				19.9%	2.78	0.29	0.0%	0.08	0.0%	0.00	0.08	0.00	0.0%	0.0%	0.0%	100%
Ferrinbian!	T = 60.00															

Statistical Information...

15th Percentile Speed 20.0 mph

Median Speed 25.7 mph

10 MPK Pace Speed 19 mph to 29 mph 2072 vehicles in pace Representing 67.2% of the total vehicles 85th Percentile Speed 31.0 mph

Average Speed 24.0 mph

Site Reference: 160070000479

Site ID: 1100000000401

Location: SALEM SO., SOUTH OF VINHIN ST.

Direction: NORTH

Care: 1

File: SP3-4-0102.prm City: VINNIN SQUARE STUDY

County: SPEED NAS

719E	19	24	29	34	39	4.4	49	Sś	59	64	69	74	79	85	36+	Tota
~~~																
02:00	0	1	0	4	2	9	0	0	0	0	0	Ð	C C	Q	0	7
02:09	0	J	2	1	1	1	D	0	IJ	0	0	0	¢	0	0	7
03:00	0	- 0	Ç.	0	0	C	0	0	j j	0	0	ij.	ū	0	0	Ċ
04:00	0	0	C	1	0	C	a	0	Ĵ	0	0	0	3	0	0	3
05:00	0	- 0	2	4	2	C	J	0	-Ĵ	0	0	0	3	0	0	8
06:40	1	0	13	6	3	1	)	3	0	0	Q	0	J	0	0	24
07:00	-	7	31	35	2	1	Ĵ	9	- 0	0	0	0	Ð	0	0	77
06:00	48	54	87	41	1	1	0	0	0	0	0	0	·J	0	0	234
09:00	37	45	101	31	3	Ĵ	- J	O	0	0	0	0	0	0	0	217
10:00	21	40	73	41	1	)	0	0	0	0	₀ 0	0	J	0	0	176
<b>5</b>																
DAY TOTAL	108	147	309	158	27	4	- 0	0	0	0	0		0	0	- 0	753
PERCENTS	14.45	19.6%	41.0%	22.35	2.2%	0.5%	0.01	0.0%	0.0%	92.0	0.0%	0.0%	0.0%	J.0%	0.0%	1004

Statistical Information ...

15th Percentile Speed 19.2 mpt

Median Speed 26.0 mph

10 MFH Pace Speed 26 mph to 34 mph 477 vehicles in pace Representing 63.3% of the total vehicles

85th Percentile Speed 31.3 mph

Average Speed 24.5 ægh

#### NassOOT Highway Division SEEEC SUMMARY Mon 4/11/2016

Page: 5

Site Reference: 160070000479

Site ID: 1100000000401

Location: SAMEN ST., SCOTH OF VINNIN ST.

Direction: SOUTH

Lane: 2

STA.453

File: SFC-4-0102.prg

City: VINNIH SQUARE STUDY

County: SPEED NAS

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	65	86∻	Tota
14:00	9	6-6	130	31	3	0	1	C-	0	0	a	Û	¢	Ď.	0	240
15:00	32	78	100	54	5	0	0	C	G	0	g	0	Ċ	Ĵ	0	266
16:00	22	81	126	33	g	0	0	C	0	Q	J.	C-	Q	O.	0	266
17:00	14	65	125	63	ą.	0	Q.	Ū	0	O	- 0	C	0	Ĵ	Q	271
18:00	B:	42	121	73	5	0	0	Û	0	0	0	C	0	0	Ò	249
19:00	19	55	59	55	6	- 0	0	0	0	0	0	J	ŋ	0	0	229
20:30	3	29	91	48	d	0	่ อ	Ü	0	C·	0	3	Ĵ	0	0	175
21:30	1	7	56	3.0	5	0	0	J	J	0	0	ĵ	ĵ	0	Ç	99
22:00	Ū-	- 6	23	28	7	0	0	Ĵ	0	D.	0	Ð	j	0	0	69
23:00	1	4	9	14	J	0	0	Ĵ	ŋ	Ç-	0	Ð	Đ,	0	C	26
24:00	1	6	5	13	3	D	0	J	0	C	0	Ð	Ð	0	C	28
DAY TOTAL	101	440	699	446	43	Ç	:	Q.	0	0	0	g	Ð	0	9	1920
PERCENTS	5.3%	23.0%	46.31	23.2%	2.2%	6.0%	3.0%	0.1ŧ	0.0%	0.0%	0.0%	D. 8%	Q. C%	C.0%	0.0%	1004

Statistical Information...

15th Percentile Speed 21.1 mph

Median Speed 26.4 mph

10 MFH Page Speed
24 mph to 34 mph
1335 vehicles in page
Representing 69.5% of the total vehicles

85th Fercentile Speed 31.3 mph

Average Speed 25.9 mph

Site Reference: 160070000479

Site ID: 110000000401

Location: SALEM ST., SOUTH OF VINNIN ST.

Direction: SOUTH

Lane: 2

File: SPD-4-0102.prn City: VINNIN SQUARE STUDY

County: SPEED N&S

TIME	19	24	29	34	46	44	19	54	59	64	69	74	79	85	E6+	Tota
01:00	1	0	2	1	c	D	0	0	0	0	Ð	0	0	0	0	4
02:00	ā		3	ī	Č	ō	Č	Ď	Õ	ő	0	ō	Ď	ŏ	-0	5
03.00	จ	9	ō	- C	í	Ď	Ď	Ď	0	Ď	ē	Ö	Ď	č	Õ	ī
04:00	จ	9	0	-0	0	Ď	Ď	Ď	0	Ö	ē	ŏ	Ď	Č	ō	0
05:00	1	2	3	0	1	Ď	Ċ-	D	Ö	Ö	Č.	Ď	Ď	č	Ð	6
06:00	0	1	9	4	3	Ů.	£	Ď	Ŏ	Ö	C	D	Ď	Č	จ	14
07:30	4	13	26	13	1	G	C	D	Ō	0	Č	Ċ	D	€	- J	57
08:00	5		100	43	2	0	£	0	0	Ö	Ċ	Ô	Ď	J	a a	197
09:00	-6		95	31	2	D	G	0	0	Ď	Ó	Ċ	Ō	ŧ	จ	189
10:00	13		98	44	1	-0	-0	Ð	Ö	Ď	Ð	Ċ	D	่ ปี	3	263
11:00	9		90	28	2	-0	0	- 0	0	D	đ	Ĉ-	. 0	g	0	186
12:00	9	5.9	95	32	2	0	0	0	0	0	- J	-C·	0	. 0	0	197
13:00	16	71	137	49	5	Ð	()	0	0	0	3	£	0	0	0	277
14:00	5	66	93	34	0	0	0	C	0	0	3	-C	C.	0	0	200
15:00	13	47	110	37	3	C	a a	C	0	0	Ú	-0	·C	0	0	210
16:00	3	69	108	48	6	C	Ð	C	0	0	0	-0	C	0	0	233
17:00	9	6 ፋ	114	52	-6	C	0	6	0	Ç.	0	-0	0	0	0	245
18:00	11	60	121	60	7	-0	0	-0	0	0	- 0	-0	-0	0	0	259
19:00	4	32	114	75	9	0	0	่ ป	. 0	0	0	Ð	0	0	0	231
20:00	2	44	90	42	5	Ĵ	, 0	J	0	C	Ó	a)	Ĵ	0	0	183
21:00	3	22	55	4.6	6	1	- 0	J	0	· · · C	0	จ	J	0	0	133
22:00	0	8	46	23	4	Ĵ	0	- 0	0	C	5	3	Ĵ	0	0	81
23:00	1	2	11	12	3	Ĵ	0	0	0	C	0	- O	0	0	0	29
24:00	0	3	10	1	2	่ ป	0	. 0	0	С	0	0	0	0	0	22
DAY TOTAL	112	769	1532	681	67	<b>1</b>		0			٠	0	0	0	0	3162
PERCENTS		24.4%			2.19	0.0%	0.0%	0.0%	0.04	0.0%	0.0%	0.0%	0.01	0.0%	0.09	100%
Statistica)	Infor	mation.														

Statistical Information ...

15th Fercentile Speed 21.4 mph

Median Speed 26.3 mph

10 HPH Pace Speed in 19 mph to 29 mph 2301 vehicles in pace Representing 72.7% of the total vehicles 85th Percentile Speed 31.0 mph

Average Speed 26.0 mph

#### MassDDT Highway Division SPEED SUMMARY Wed 4/13/2016

Page: 7

Site Reference: 160070000479

Site ID: 110000000401

Location: SALEM ST., SOUTH OF VINNIN ST.

Direction; SOUTH

Lane: 2

TIME 86+ Tota 01:00O Ð 02:00 ũ Ð П Ō Ō O. 03:00 O Q Û Ü 04:00 Û Δ n. Û Û 05:00 0 0 Û Ů. ũ O 06:00 C Ď 07:C0 В Ū · 13 Ů. Ð Ü C 08:00 £12 В П 09:00 4.7 D. J Ð O 10:00 4.6 Π Û П O П 11:00 Q. Ð 12:00 Q n Ô Q. 13:00 D ß Û Û ø Q 14:00 D Ð Ð Ď П 15:00 Ō D D Ō Ō ø D Ū 16:00 -8 D D Ð Ð ö D 17:00 Û Ð 18:00 Ö þ Ð a .....C 19:00 Đ Ð 20:00 б O 21:00 à Û ß Đ. 22:00 Ð û 23:00 -0 24:00 В Ð 

4 0

C

O-

Statistical Information...

DAY TOTAL

15th Percentile Speed 21.3 mph

Median Speed 26.5 mph

10 MPH Pace Speed
24 mph to 34 mph
2440 vehicles in pace
Representing 69.9% of the total vehicles

775 1618 822 104

85th Percentile Speed 31.5 mph

0 0 0 0 0 3490

Average Speed 26.1 mph

File: SPD-4-0102.prn

County: SPEED N&S

City: VINNIM SQUARE STUDY

### MassCOT Highway Division SPEED SCHMARY 7hu 4/14/2016

Page: 8

Site Reference: 160070000479

Site ID: 110003300401 Location: SALEM SP., SOUTH OF VINKIN ST.

Direction: SOUTH

Lane: 2

File: SPD-4-0102.pm City: VINNIN SQUARE STUDY County: SPEED N4S

TINE	13	21	29	34	39	44	49	54	59	64	69	34	79	85	E 6+	īcta
												*****				
01:00	Ð	0	5	2	0	C	-0	0	0	0	o	c	0	0	0	1
02:00	0	0	2	4	0	C	ū	0	0	Ö	ō	Ċ	Ď	ō	ō	6
03:00	Û	0	2	1	0	g	0	0	0	0	Ó	Ó	5	ō	ō	3
04:00	C	0	0	0	0	J	ŋ	0	0	0	0	Ď	j	ō	Ö	Ď
05:00	¢	Ç	4	-	0	J	1	0	0	0	ŋ	D	j	0	0	3
06:D0	€	1	10	ซ์	0	J	0	D	0	0	0	0	0	0	- 0	21
07:00	€	9	31	15	3	0	0	D	0	C	ŋ	ú	0	0	0	63
06:00	15	36	93	59	8		0	C-	0	Ç	0	0	0	0	Ç	212
09:00	12	48	117	-55	7	0	0	C·	0	0	0	Q	0	0	D	249
10:30	11	50	104	42	6	1	0	Ç.	0	C	0	0	0	0	0	214
									·							
JAY TOTAL	45	144	365	196	24	2	1	8	Ç-	Ç.	0	0	Q	Ç.	C	778
PERCENTS	6.01	18.6%	47.09	25.15	3.0%	0.2%	9.1%	0.3%	0.0%	0.0%	J.09	0.0%	0.0%	0.35	0.09	100%
	_															

Statistical Information...

15th Percentile Speed 21.5 ⊃ph

Median Speed 26.7 mph

10 MPH Pace Speed 24 mph to 34 mph 561 vehicles in pace Representing 72.1% of the total vehicles 85th Percentile Speed 31.7 mph

Average Speed 26.2 ლინ

#### MassDOT Highway Division SPEED SUMMARY Mon 4/11/2016

STA. 5

TOTAL

Site ID: 110000000501

Location: TEDESCO ST., WEST OF WEST SP.

Sirection: ROAD TOTAL

Site Reference: 160070000758

File: SPD-5-G102.prm City: VIRNIN SQUARE STUDY

County: SPEED N&S

TIME	19	24	29	34	39	44	19	54	59	64	69	74	T9	85	96÷	Tota
16:00	13	19	141	502	310	41	3	3	0	0	c	3	j j	9	0	1329
15:00	4	45	205	573	231	18	J	3	0	0	C	Û	Ü	0	0	1090
16:00	17	46	252	591	318	32	1	1	0	0	0	0	J	0	O.	1256
17:00	2	26	220	590	336	26	J	Ð	0	0	0	Ç	Ð	0	0	1192
19:00	23	39	201	613	327	44	3	Ð	0	0	0	C	Ð	0	0	1250
19:00	15	41	177	478	331	37	2	1	0	0	0	C	Ð	0	0	1082
20:00	J	6	100	399	233	27	3	Ð	0	Q.	G-	Ç	Ð	Û	D	758
21:00	2	6	73	193	155	26	3	0	0	0	G-	C	Ð	0	Ç.	474
22:00	2	€	34	114	125	16	4	0	0	0	G	0	Ð	0	C	299
23:00	0	9	7	52	36	11	3	0	0	0	G-	D	Ð	¢.	C	111
24:00	ū	3	5	21	27	1	3	1	0	0	C	Đ	0	Û	Ç.	58
DAY TOTAL	 78	234	1415	4116	2442	261	30	 3		0	0	3	0	 C	C	8599
PERCENTS	1.0%		16.5%			3.2%	0.35	0.0%	0.0%	0.09	0.6%	0.36	0.0%	G.0%	0.04	1004

Statistical Information...

15th Percentile Speed 27.5 mph

Median Speed 32.1 mph

10 MPH Page Speed 29 mph to 39 mph 6553 vehicles in page Representing 76.2% of the total vehicles 85th Percentile Speed 37.0 mph

Average Speed 32.0 mpb

Vehicles > 65 MPH 0.0%

Fage: 9

#### MaasDOT Highway Division SPEED SUMMARY Tue 4/12/2016

Page: 10

Site Reference: 160070000758

Sice ID: 110000000501

Location: TEDESCO ST., WEST OF WEST ST. Direction: ROAD TOTAL

File: SP0-5-0102.prn City: VINNIN SQUARE STUDY

County: SPEED N&S

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	0	1	2	8	11	5	0	0	0	0	Q.	0	۵	0	0	_
02:00	0	0	1	5	_	Q	1	1	. ·	0	0	0	0	0	. 0	
03:00	0	0	1	- 9	4	1	0	0	, o	0	0	0	D	0	0	
34:00	0	0	0	. 0	. 5	3	0	0	Ŷ.	0	0	Q.	0	0	0	6
05:00	0	2	3	12	12	. 9	5	1	0	0	0	Q	0	0	0	
06:00	1	2	16	48	71	37	- 6	0	0	0	0	0	0	0	0	
07:00	6	16	83	236	285	60	7	0	0	C	0	0	0	0	0	693
08:00	4	30	182	650	303	43	1	0	0	C	0	0	0	0	0	1213
09:00	7	49	210	571	290	28	1	0	0	C	0	0	0	0	0	1156
10:00	2	13	136	471	309	29	2	Ð	0	C	0	0	0	Ō	0	962
11:00	6	39	191	423	257	34	2	0	0	0	0	0	0	0	0	952
12:00	2	16	143	451	269	40	3	O	0	D	0	0	0	0	0	944
13:00	4	34	165	473	283	32	3	Ó	Û.	Q	O	0	0	0	0	994
14:00	4	23	129	411	296	45	4	Û	0	Ó	Ó	0	0	0	0	912
15:00	- 6	34	194	483	278	27	1	1	Q	0	0	0	O	0	0	1024
16:00	- 6	32	208	593	262	25	4	0	0	Û	0	Q.	0	Q	Q	1130
17:00	12	29	259	632	281	18	. 3	0	0	-0	0	0	0	Q	0	1234
18:00	. 7	34	254	653	299	22	0	0	0	-0	0	0	0	0	0	1269
19:00	9	46	215	498	329	38	6	0	D-	0	0	Ů-	0	0	0	1141
20:00	1	17	89	438	204	22	0	0	0	0	0	0	Ó	0	0	771
21:00	D	11	51	255	164	23	2	2	Ò.	ō	ò	Ō	ō	0	0	508
22:00	Ċ	4	30	120	127	15	ī	ō	ò	Ó	ò	Ď	ò	ō	Ō	297
23:00	Č	3	10	42	51	14	. 0	ō	0	0	ō	0	ō	Ö	Ď	120
24:00	¢	2	7	17	23	10	2	1	۵	0	Q	٥	0	. 0	۵	62
DAY TOTAL	77	437	2579	7494	4435	 580	 54	6	o	0	0	0		0	п	15662
PERCENTS	0.5%		16.5%			3.7%	0.3%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	

Statistical Information...

15th Percentile Speed 27.6 mph

Median Speed 32.2 mph

10 MPH Pace Speed 29 mph to 39 mph 11929 vehicles in pace Representing 76.1% of the total vehicles . 85th Percentile Speed 37.1 mph

Average Speed 32.1 mph

#### MassDOT Highway Division SPEED SUMMARY Wed 4/13/2016

Page: 11

Site Reference: 160070000758

Site ID: 11000000501

Location: TEDESCO ST., WEST OF WEST ST.

Direction: ROAD TOTAL

File: SPD-5-0102.prn City: VINNIN SQUARE STUDY

County: SPEED N&S

TIME	19	24	29	34	39	44	49	54	59	54	69	74	79	85	85+	Tota
			377													
01:33	0	0	2	9	12	5	D	C	1	۵	C	0	0	Ç.	0	29
02:00	0	0	0		5	C	2	¢	Q	0	Ç	0	0	C	0	9
03:00	0	e	0	1	2	1	D	Ç	D.	0	C	0	0	Ç.	0	4
04:00	Û	0	0	1	2	3	1	C	0	0	C	0	0	C	Q	7
05:00	0	3	3	8	14	В	2		0	0	C	0	0	C	0	36
05:00	0	4	8	42	86	37	7	1	0	0	G	0	0	C	0	185
00:00	2	5	51	238	305	61	6	C	0	0	C	0	0	Ç	0	674
08:00	3	1€	136	567	393	65	2	C	0	0	C	0	0	C	0	1161
09:00	9	24	129	563	346	23	2	1	Ç	0	G	0	0	C	0	1097
10:00	5	11	128	458	310	41	1	0	0	0	C	0	0	C	0	954
11:00	4	23	107	507	285	25	3.		0	0	C	0	0	C	0	954
12:00	1	22	140	524	303	47	0	3	0	0	C	Q.	0	Ç	Q	1037
13:00	9	16	144	538	319	34	2	1	0	0	C	0	0	C	0	1062
14:00	17	68	258	543	206	33	0	1	1	0	C	0	0	С	0	1127
15:00	13	26	200	585	304	29	1	3	0	0	0	0	0	C	Q	1152
16:00	5	29	212	536	350	31	4	· 3	0	0	0	0	0	C	0	1167
17:00	11	48	255	630	321	22	2	J	0	0	0	0	0	C	0	1289
16:00	15	39	278	103	304	22	2	1	Ç.	0	0	Ç.	0	C	0	1364
19:00	8	€3	223	532	327	29	2	0	Ç.	0	0	C.	0	C	- 0	1164
20:00	6	27	150	403	220	26	2	g.	Ç.	0	0	Ç.	Ú	C	0	834
21:00	2	15	134	275	143	9	1	Ĵ	Ç.	0	0	Ç	0	C	0	579
22:00	0	7	34	130	120	20	D	ŋ	D.	0	0	0	0	¢	0	311
23:00	0	1	11	56	62	20	1	ŋ	¢	0	0	¢	Ú	0	0	151
24:00	0	2	6	15	29	11	1	ŋ	C	0	0	C	0	0	0	66
DAY TOTAL	110	417	2617	7866	4773	662	44		 2	0	 0		0	 ე	0	16435
PERCENTS	0.76			47.9%		3.6%	0.2%	0.3%	0.0%	0.0%	0.0%	€.0%	80.0	0.0%	D.0%	1005

Statistical Information ...

15th Percentile Speed 27.7 mph

Hedian Speed 32.2 mph

10 MFH Pace Speed
29 mph to 39 mph
12639 vehicles in pace
Representing 76.9% of the total vehicles

85th Percentile Speed 37.1 mph

Average Speed 32.2 mph

#### MassDOT Highway Division SPEED SUMMARY Thu 4/14/2016

Page: 12

Site Reference: 160070000758

Site ID: 110000000001

Location: TEDESCO ST., WEST OF WEST ST.

Direction: ROAS POTAL

File: SPS-5-0102.prm City: VINNIN SQUARE STUDY

County: SPEED N&S

EMIT	19	24	29	34	35	14	49	54	59	64	69	74	79	85	∌6+	Tota
01:00	0:	Ù	1	14	12	4	1	Ç	C	Ò	0	0	0	g	0	32
02:00	Û	0	2	6	8	3	0	C	C	0	0	0	0	0	0	17
03:00	Û	0	2	3	:	0	0	¢	Ç	0	0	0	0	0	0	- 6
04:00	0-	0	C	3	ő	Q	2	C	J	0	0	0	0	0	0	:1
05:00	D:	5	7	8	25	5	2	C	3	0	Ç.	0	0	0	0	52
06:00	D-	1	7	35	82	35	5	C	Ĵ	0	Ç.	0	0	0	0	165
07:00	1	7	54	179	301	56	4.	1	J	0	Ç.	0	0	0	0	613
08:00	Ε	14	161	585	392	43	. 3	Q	3	0	Ç.	0	0	0	0	1204
09:00	1	25	164	572	302	25	1	0	0	0	Ç.	0	0	Ú	0	1030
10:00	12	30	153	435	252	41	3	0	Ð	Ç	C	0	0	0	0	926
PERCENTS	2C 0.5%	92 2.0%	551 13.45	1840 44.8%		220 5.3%	21 0.5%	0.0%	მ 0.0%	0.01	0 Ծ.Եֆ	0 90.0	0 10.0	0 0.0%	0.0%	4116 100%

Statistical Information...

15th Percentile Speed 28.7 mph

Median Speed 32.8 mph

10 MFH Pace Speed 29 mph to 39 mph 3221 vehicles in pace Representing 73.2% of the total vehicles

85th Percentile Speed 37.7 mph

Average Speed 32.8 ਛੜੂੀ

Vehicles > 55 MPH 9 3.0%

#### MassDOT Highway Division SEEED SOMMARY Mon 4/11/2016

Page: 1 0

STA.5 NB

Site Reference: 160070000758

Site ID: 1100000003501

Bodation: TEDESCO ST., WEST OF WEST ST.

Direction: NORTH

Eane: 1

File: SPO-5-0102.prn City: VINNIN SQUARE STUDY

County: SPEED NAS

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	65	86+	Tota
14:00	6	14	79	297	112	17	0	3	g	0	g	Ð	0	0	C	525
15:CO	1	29	114	261	93	12	0	Ĵ	Ĵ	0	Ĵ	-J	Ĵ	0	C	500
16:00	4	25	145	322	132	9	0	J	J	0	J	Ð	จ	0	C	637
17:00	1	19	109	263	135	17	0	Ĵ	â	0	Ĵ	Ð	J	0	C	542
18:00	3	19	117	265	120	11	1	j j	3	0	g	-1	-3	0	C	535
19:00	Ę.	5	70	211	107	7	0	Ĵ	Ĵ	0	Ĵ	-J	t)	0	C	404
20:00	0	4	49	159	77	9	0	Ĵ	Ĵ	0	J	Ð	จ	0	Ċ	297
21:33	1	2	34	63	54	ধ	3	-3	J	ŋ	J	Ð	จ	0	C	165
22:00	2	2	21	59	4.4	4	Ç	J	Ĵ	0	Ĵ	Ð	จ	0	C	132
23:00	0	0	6	23	24	- (	Ţ	-3	3	0	Ĵ	Ð	Ú,	0	0	48
24:00	0	0	S	1:3	9	0	1	0	·J	0	Ð	Ð	Ĵ	0	0	55
neo maria		112	7	1002	207						2					3017
DAY TOTAL PERCENTS	22 0.6%	117 3.11	744 19.5%	1953 51.1%	987 23.1%	98 2.5%	6 C.3%	C.0%	0.3%	0 C.J%	0 0.0%	0.0%	0.0% 0.0%	0.0%	0.0%	3627 1004

Statistical Information...

15th Percentile Speed 26.9 mph

Median Speed 31.6 mph

16 M9H Page Speed
 29 mph to 39 mph
 2840 vehicles in page
 Representing 74.2% of the total vehicles

95th Percentile Speed 36.4 mph

Average Speed 31.5 ⊋ph

Vehicles > 55 MPK C 0.0%

#### MassDOT Highway Division SPEED SUMMARY Tue 4/12/2016

Page: 2

Site Reference: 160070000758

Site ID: 110000000501

Location: TEDESCO ST., WEST OF WEST ST.

Direction: NORTH

Lane: 1

TIME 19 24 29 34 39 44 49 54 59 64 69 74 79 85 B6+ Tota

01:00	0	1	1	3	6	2	0	0	0	Q	D-	Q	0	Q.	0	13
02:00	. 0	0	1	3	0	Ū	0	0	0	D	Û	0	0	D.	D	4
03:00	0	0	1	1	2	1	0	0	0	D.	0	0	0	D.	0	5
04:00	. 0	0	0	0	0	1	0	Ð	0	0	Ů.	0	0	O	D	1
05:00	0	2	1	5	7	5	2	· O	. 0	0	0	Q	0	O	D	22
05:00	1	2	13	26	50	19	3	D	0	0	Q-	0	0	0	0	114
07:00	1	3.4	64	164	200	44	5	Ð	0	0	Q.	0	-0	0	0	492
08:00	Q.	18	111	451	188	11	1	0	0	0	Q-	0	0	0	0	780
09:00	3	32	140	339	153	7	0	0	0	0	0	0	0	0	D	674
10:00	1	10	90	278	168	11	0	0	0	0	0	0	0	0	0	558
11:00	3	31	130	237	137	8	0	0	0	0	0	0	0	D	D	546
12:00	2	11	85	256	129	18	0	· O	0	0	0	0	0	0	0	501
13:00	2	14	90	258	119	9	0	Ð	0	0	0	0	Ð	Q.	0	492
14:00	2	13	90	207	109	8	2	0	0	Q	Q.	0	- 0	0	0	431
15:00	1	19	104	255	107	12	0	1	0	Q	0	0	0	0	0	499
16:00	3	10	115	322	117	6	0	0	0	0	0	0	-0	0	0	573
17:00	1	14	105	302	125	6	2	0	0	0	0	0	-Q	0	0	555
18:00	2	13	93	261	132	- 6	0	0	0	Q	0	0	Q	0	0	507
19:00	5	16	73	192	139	24	1	Đ.	Q.	Q.	0	0	Ð	0	0	450
20:00	0	6	34	182	85	8	0	Û	0	Q.	0	0	Ð	0	0	315
21:00	0	3	31	123	63	5	1	2	0	0	0	0	0	0	0	228
22:00	0	3	20	49	4.0	5	O.	0	Q	0	0	Ó	0	0	0	117
23:00	Ó	0	- 6	21	20	4	O	Ū.	0	Û	0	Ô	0	0	0	51
24:00	0	2	2	9	10	4	2	D.	0	Û	0	0	0	0	0	29
DAY TOTAL	27					224	19	3	0	0	0	. 0	Ð	0	0	7957
PERCENTS	0.4%	3.0%	17.6%	49.6%	26.48	2.8%	0.28	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information ...

15th Percentile Speed 27.3 mph

Median Speed 31.9 mph

10 MPH Pace Speed 29 mph to 39 mph 6050 vehicles in pace Representing 76.0% of the total vehicles \$5th Percentile Speed 36.8 mph

Average Speed 31.9 mph

File: \$PD-5-0102.prn

County: SPEED N&S

City: VINNIN SQUARE STUDY

### MassDOT Highway Division SPEED SUMMARY Wed 4/13/2016

Page: 3

Site Reference: 160070000758

Site ID: 110000000501

Location: TEDESCO ST., WEST OF WEST ST.

Direction: NORTH

Lane: 1

File: SPD-5-0102.prn City: VINNIN SQUARE STUDY

County: SPEED NAS

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	Ð	0	1	4	4	2	D	0	1	0	0	0	0	0	0	12
02:00	Û	0	0	0		0	2	0	· O	0	-0	0	0	-0	C	2
03:00	0	0	0	0	_	0	0	D	O.	0	0	0	0	0	C	0
04:00	0	0	0	0	_	0	0	0	0	0	0	0	0	0	C	2
05:00	0	3	3	4	5	5	1	0	0	0	0	۵	0	0	0	21
06:00	0	- 4	6	27	56	24	5	0	0	0	0	0	O	0	0	122
07:00	2	4	46	156	-	4.5	5	0	0	Ů.	: 0	0	0	0	0	489
08:00	1	7	82	339		37	1	0	0	0	0	Ü	0	0	Q	710
09:00	4	13	84	350		9	0	0	Q	0-	0	Û	0	0	0	656
10:00	1	2	78	276		26	0	0	0	0	0	Ò	0	- 0	D	544
11:00	1	14	66	295		12	1	0	0	Ø	0	0	0	Ó	- 0	541
12:00	O	7	83	255	156	22	0	O.	Ó	0	0	0	0	0	0	523
13:00	3	8	73	268	169	14	1	0	0	0	0	0	0	0	0	536
14:00	3	29	108	252	117	20	C	1	1	0	Û	0	Ð	0	0	531
15:00	13	10	98	272	143	15	1	0	0	Û	0	0	Ð	0	0	552
16:00	5	17	100	280	160	14	0	0	0	0	0	0	0	Ô	0	576
17:00	8	22	138	316	141	7	0	G	0	0	0	0	0	0	0	632
18:00	6	19	137	310	129	8	1	C	0	O	0	0	0	0	0	610
19:00	3	8	105	217	131	9	Ū	C	0	0	0	0	0	0	0	473
20:00	5	11	63	151	70	11	0	Û	0	0	0	0	0	0	0	308
21:00	1	6	45	122	53	3	0	Ü	0	0	0	0	0	0	Ó	230
22:00	0	6	16	49	4.7	7	0	0	0	0	0	0	0	0	0	125
23:00	0	1	6	26	23	10	1	Q	0	0	0	Ċ	Ó	0	0	67
24:00	0	Û	3	10	7	5	1	۵	0	0	0	C	0	0	0	26
DAY TOTAL	53	191	1341	3979	2397	304	20	<u>1</u>	2	0	0		0	0		8288
PERCENTS	0.70		16.2%			3.6%	0.2%	0.08	0.00	0.0%	0.0%	0.0%	0.0%	0.0%	0.01	100%

Statistical Information...

15th Percentile Speed 27.7 mph

Median Speed

10 MPH Pace Speed

32.2 mph

29 mph to 39 mph

6376 vehicles in pace Representing 76.9% of the total vehicles

85th Percentile Speed 37.1 mph

Average Speed 32.2 mph

Vehicles > 65 MPH 0

0.0%

#### Nass 201 Bighway Division SPEED SCHOURRY Tho 4/14/2016

Fage: 4

Site Reference: 160070000758

Site ID: 1100000000501

Location: TEDESCO ST., WEST OF WEST ST. Direction: NORTH

Lane: 1

File: SPD-5-0102.prm City: VINNIN SQUARE STUDY

County: SPEED NES

TINE	:9	24	29	34	39	44	49	54	59	64	69	74	79	85	864	iota
													~= n = + a			
G1:00	3	0	0	6	- 5	3		С	C	0	a a	9	0	Ð	J	16
62:00	j	. 0	2	3	1		0	C	C	0	1	0	0	Ð	0	7
03:00	- O	. 0	1	1	1.	0	0	C	G	0	j j	Û	0	Ð	0	3
04:00	0	C-	D	D.	2	0	1	C	C	0	9	3	Ò	- 0	·J	3
05:00	0	4	4	5	6	ڌ	i	C	Q	0	0	ŋ	0	- 0	0	23
06:00	0	1	E	27	56	22	4	Ċ	Û	0	g.	0	0	Ð	J	116
07:00	-	4	36	115	237	47	3	1	Ū	0	ŋ	J	0	0	- 3	444
09:00	2	±1	63	345	247	26	2	Q	0	0.5	. 0	g	0	Ð	J	716
09:00	0	13	105	347	161	16	0	G	0	0	9	Ĵ	0	Ð	3	612
10:00	4	6	85	293	137	12	ī	۵	0	· · · · · ·	Ĵ	3	0	Ð	Ĵ	536
224 50221			511			120					n					2526
PERCENTS	J. 31	1.69	322 12.9%			130 5.11	13 0.51	0.0%	0.0%	0 10.0	0 0.0%	0.0%	0.01	0.3%	0.01	2508 1001

Statistical Information...

15th Percentile Speed 29.0 mph

Median Speed 32.9 πp'n

10 MPH Page Speed 29 mgh to 39 mgh. 1995 vehicles in pace Representing 79.5% of the total vehicles

35th Percentile Speed 37.7 mpt

Average Speed 33.0 aph

#### MassDOT Highway Division SPEED SUMMARY Mon. 4/11/2016

STA. 55B

Site Reference: 160070900758

Site IG: 113300086531

location: TECESCO ST., WEST OF WEST ST. Direction: SCUTH

Lane: 2

File: SFD-5-0102.prm City: VINNIR SQUARE STUDY

County: SPEED Mas

SHIT	15	24	29	34	39	44	45	54	59	64	69	74	79	95	86~	Tota
14.00				205	169		3	r.			6	÷ 0			0	604
14:00 15:00	2	5 20		205 312		2 £	0	C	0	0	0	0	0	0	C	50¢ 580
15:00	13	19		269		23	1	1	0	0	0	0	0	o o	ď	619
13:00	1.1	13	112	-		11	Ď	Ċ	0	Ď	Ö	ő	Ö	Ů,	u 0	
-	23	21	94	346		33	2	Ġ	0	0	0	ő	0	0	_	650
16:00		36		267			- 4	1	_	_	_	_	_		ŋ	715
19:00	11				224	30	- 4	Ţ	0	C	0	0	C	. 0	Ĵ	678
20:00	Ĵ	2	52	240		18		Ü	0	C	L	0	L a	0	0	471
21:00	Ţ	- 5		110		18		0	0	C-	2	0	i,	0	11.3	289
22:00	0	2		55		12	- 4	J	D.	C-	E	Q.	Ç	0	3	167
23:00	0	9	_			7	2	j j	C.	C	Ç	0	C	0	J	63
24:00	0	3	3	11	18	1	2	1	C·	C	Ç	0	Q	0	-1	35
CAY TOTAL	 56	117	671	2163	1555	183	24	3	 C	C		0	0	0	 О	4772
PERCENTS	1.2%		14.19			3.8₺	0.5%	0.0%	0.00	0.0%	0.6%	0.0%	0.0%	0.09	0.0%	100%

Statistical Information...

15th Percentile Speed 26.1 mpt.

Median Speed 32.6 rsph

10 MPR Pace Speed 29 aigh to 39 aigh 3718 vehicles in pace Representing 77.9% of the total vehicles 85th Percentile Speed այր և ունե

Average Speed 32.4 mpt.

Vehicles > 65 HPH 0 0.0%

Page: 5

#### MassDOT Highway Division SPEED SUMMARY Tue 4/12/2016

Page: 6

Site Reference: 160070000758

Site ID: 110000000501

Location: TEDESCO ST., WEST OF WEST ST.

Direction: SOUTH

Lane: 2

19 29 34 39 44 55 54 59 64 69 74 79 65 96+ Tota TIME 24 01:00 02:00 1 - 0 0 2 2 П 1 1 C 0 0· b 0 Ω J 1 O. O. 5 03:00 D. 3 2 П C D. C ŋ. Ð D ũ ð. 1 0 Q. Q 5. 0 C 0 0 0 Û 04:00 2 € Ĵ J 2 7 22 05:00 :) -0 5 4 3 1 C ũ 0· 1 0 ũ ď. 06:00 Ð 0 3 22 21 18 3 0 C 0 ð. a D 0 a 67 07:00 5 2 19 72 95 16 0 Q. Ð 0 0 201 199 115 C С Π <u>ስ</u> 0 08:00 Ł 1.2 71 32 0 a. D 433 70 232 09:00 4 17 137 21 1 0 Ç 0 g. ð 0 0 482 10:00 1 3 46 193 141 18 2 0 Ç 0 Œ. O. Q 0 404 6 186 120 26 C 0 ů. a 0 406 11:00 3 61 2 0 0 0 5 195 22 D 0 12:00 58 160 Ç. g. 443 2 20 164 C 13:00 75 215 23 3 C 0 a 0 D 0 -0 502 2 10 39 204 187 37 2 C П ũ ũ 0 D. Ω 0 481 14:00 5 15 90 22B 171 Ç. Û 0 Ü O. 0 0 15:00 15 1 525 16:00 3 22 33 · 273 145 19 Ġ ¢ Û 0 0 ů. 0 0 û 557 330 156 Ò a D. 0 679 15 12 Ç. n 0 17:00 11 154 1 18:00 5 21 161 392 167 16 C Ð 762 306 190 Ò Û 4 -0 0 19:00 4 - 30 142 14 5 ¢ П 691 11 55 256 119 ·J ¢ Û 0 0 Ú Ð Ò 456 20:00 14 à 8 0 20 132 101 19 ŋ 0 0 0 0 0 280 21:00 10 71 Ď. ŋ n a n 0 n 180 22:00 0 1 87 13 1 21 23:00 0 3 4 31 10 ú a Ò Û 0 £. Ď 69 24:00 5 33 50 203 1179 3550 2329 356 35 DAY TOTAL 3 0 0 0 0 6 0 0 7705 0.7% 2.7% 15.4% 46.0% 30.2% 4.6% 0.4% 0.0% D.C% D.C% D.C% 0.0% 0.0% 0.0% C.C% 0.0% 190% PERCENTS

Statistical Information...

15th Percentile Speed 27.0 mph

Median Speed 32.4 mph

10 MPH Pace Speed
29 mph to 39 mph
5879 vehicles in pace
Representing 76.3% of the total vehicles

85th Percentile Speed 37.4 mph

Average Speed 32.4 mph

File: SPD-5-0102.prn

County: SPEED N&S

City: VINNIN SQUARE STUDY

#### MassDOT Highway Division SPEED SUMMARY Wed 4/13/2016

Page: 7

5:to Reference: 160070000758

Site ID: 110000000501

Location; TEDESCO ST., WEST OF WEST ST.

Direction: SOUTH

Lane: 2

File: SPD-5-0102.prn City: VINNIN SQUARE STUDY

County: SPEED N&S

TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	0	0	1	5		3	-0	0	0	0	0	0	0	0	0	13
02:00	0	Q	0	2	5	D.	0	0	0	0	Q	0	0	0	0	7
03:GQ	0	۵	0	3	2	3	0	0	0	D	Q.	0	0	0	0	4
04:60	0	Û	Q	1	Ō	3	1	0	0	-0	0	0	0	0	0	
05:00	O·	Q	O	4	9	3	1	0	. 0	0	0	0	0	. 0	0	13
06:00	0	Ū	2	15	30	13	2	1	0	0	0	0	0	Ġ	G.	6
07:00	D	1	11	82	74	16	1	0	Ō	0	0	0	0	0	0	13:
08:00	2	3	54	228	155	29	1	0	0	0	0	0	0	D	0	471
09:00	5	11	45	213	149	15	2	1	D	0	0	0	0	0	0	441
10:00	4	9	50	182	149	15	1	0	0	0	0	0	0	0	0	410
11:00	3	9	41	212	133	13	2	0	Q	0	0	Q	0	0	0	413
12:00	1	15	57	269	147	25	0	0	0	0	0	0	0	0	0	514
13:00	6	Ð	71	270	150	20	1	0	Ð	0	O.	0	0	0	0	526
14:00	14	39	150	291	89	13	0	0	Ð	0	0	0	0	0	0	598
15:00	0	10	102	313	161	14	0	O	0	0	0	0	0	0	0	600
16:00	-0	12	112	256	190	17	4	0	0	0	D	D	0	0	0	591
17:00	3	26	117	314	180	15	- 2	0	0	0	D	0	0	0	0	657
18:00	9	20	141	393	175	14	1	1	0	0	0	D	0	D	0	754
19:00	5	35	118	315	196	20	2	.0	0	0	0	- 0	0	Ū.	0	691
20:00	4	16	87	252	150	15	2	0	0	0	0	-0	0	0	0	524
21:00	1	9	89	153	90	6	1	0	0	0	0	0	0	0	Ü	345
22:00	0	1	19	81	73	13	0	D	0		Ü	0	0	0	Û	186
23:00	0	0	5	30	39	10	0	. 0	O.	0	0	0	0	o' 0	0	84
24:00	0	2	5	5	22	6	0	ם	. 0	0	0	0	D	0	0	40
DAY TOTAL	57	226	1276	3887	2376	298	 24	3			 D	0		0	0	8147
PERCENTS	0.7%			47.8%		3.6%	0.2%	G.01	0.0%	0.06	0.0%	0.0%	0.08	0.0%	0.00	1008

Statistical Information...

15th Percentile Speed 27.7 mph

Median Speed 32.2 mph

10 MPH Pace Speed
29 mph to 39 mph
6263 vehicles in pace
Representing 76.8% of the total vehicles

85th Percentile Speed 37.1 mph

Average Speed 32.2 mph

#### MassECT Highway Division SPEED SUMMARY Thu 4/14/2016

Page: 8

Lane: 2

Site Reference: 160070000758 Sice ID: 110000000501 File: SPD-5-ClO2.prn Caty: VINHIN SQUARE STUDY Location: TECESCO ST., WEST OF WEST ST. County: SPEED N&S Direction: SCUTH

TIME	19	24	29	34	39	44	19	54	59	64	69	74	79	35	86+	Tota
01:00	0	0	:	8	á	1	0	3	a	c	C	o.	- 0	e	0	16
02:30	Ģ.	Ċ	ō			- 3	ò	ū	j	Č.	Č	ŏ	ő	Č	Ö	10
03:00	G	C	i	2	0	ò	á	0	- -	Č	Ċ	ő	ő	Ğ	Č	7
04:00	Ç	¢	Ö	3		Ō	1	ō	- 1	Č	Ď	ŏ	ő	9	Č.	F
05:00	C	1	3	3	19	2	1	0	Ð	0	G.	ō	á	ä	Č	29
06:00	3	Q	Ţ	6	26	13	1	- 0	ŧ	0	Ŭ:	ō	ò	ō	G	45
07:00	c	3	16	64	54	19	2	0	Ú	3	0	0	ō	ō	ď	159
08:00	¢.	3	76	240	145	10	1	0	, J	0	Ç-	ō	Ö	ò	j	468
09:00	1	12	59	225	141	9	1	Ç	0	J	C	0	0	0	0	4.48
10:00	9	24	-58	142	115	29	2	C	0	0	0	0	C	0	Û	368
************						_=										
DAY TOTAL	13	43	229	698	527	90	Ē	0	0	0	3	0	0	0	0	1508
PERCENTS	0.97	2.75			32.7%	5.5%	0.4%	0.0%	0.09	0.3%	0.0%	0.09	0.0%	0.0%	0.0%	1001

Statistical Information...

15th Percentile Speed 28.1 aph

Median Speed 32.7 aph

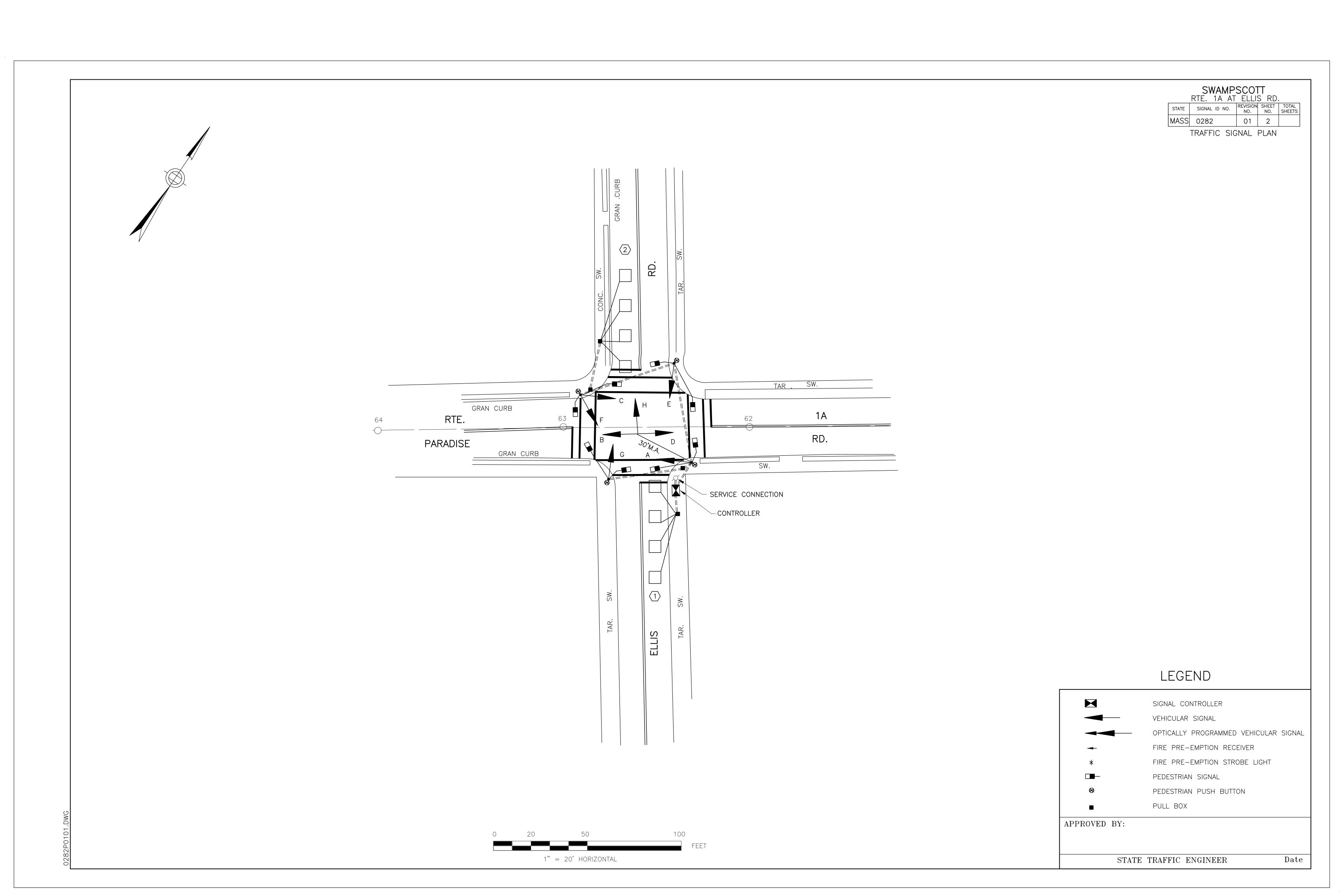
10 MPH Page Speed 29 mph to 39 mph 1225 vehicles in pace Representing 76.1% of the total vehicles 65th Fercentile Speed 37.7 mph

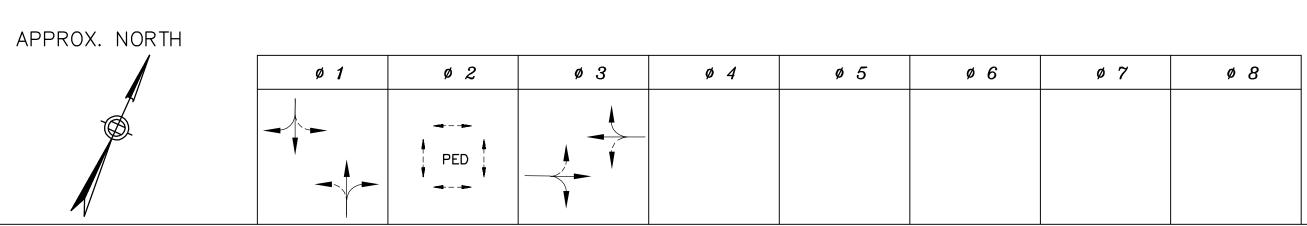
Average Speed 32.6 ±ça

Vehicles > 65 MPR Ō. 0.0%

# **APPENDIX C**

**Traffic Signal Timing and Layout Information** 





STREET	DIRECTION	HOUSINGS	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	FLASE OPER.
PARADISE ROAD (RTE. 1A)	NB	A,B	G	Υ	R	R	R	R	R	R	R																FY
PARADISE ROAD (RTE. 1A)	SB	C,D	G	Υ	R	R	R	R	R	R	R																FY
ELLIS RD.	EB	G,H	R	R	R	R	R	R	G	Υ	R																FR
ELLIS RD.	WB	E,F	R	R	R	R	R	R	G	Y	R																FR
PEDESTRIAN	ALL	ALL	DW	DW	DW	W	FDW	DW	DW	DW	DW																OFF
					<i>T1</i> .	MIN	G IN	SE	CONL	is .																	
MINIMUM GREEN (INITIAL)			30						6																		
PASSAGE TIME (VECHICLE)			1						3																		
MAXIMUM 1			30						16																		
MAXIMUM 2			0						0																		<i>[2]</i>
YELLOW CLEARANCE				4						4																	MERCENCY ONLY
RED CLEARANCE					2						2																ZZ ZZ
WALK (W)						10																					ME O
PEDESTRIAN CLEARANCE							8																				$E_I$
RECALL				OFF																							
MEMORY			NON-	<u> </u>	KING I	NON-	<u> </u>	KING	NON-		KING																

QUANTITY	ITEM
1	CONTROLLER TYPE 3W, CAB.& FDN.
1	SERVICE CONNECTION, TYPE OVERHEAD
3	10' SIGNAL POLE, BASE, & FDN.
1	30 FT MAST ARM ASSEMBLY, BASE & FDN. TY.
1	3 WAY, 3 SECTION, SIGNAL HOUSING (12" LENS)
1	2 WAY, 3 SECTION, SIGNAL HOUSING (12" LENS)
3	1 WAY, 3 SECTION, SIGNAL HOUSING (12" LENS)
8	PEDESTRIAN HOUSING (TYPE INCANDESCENT)
4	PEDESTRIAN PUSH BUTTON, SIGN & SADDLES
2	DUAL CHANNEL LOOP DETECTOR AMPLIFIER
8	ROADWAY LOOP DETECTOR
4	12"X12" PULL BOX
	Necessary duct, cable, labor, miscellaneous
	material and equipment to complete the installation.

MAJOR ITEMS REQUIRED

# SWAMPSCOTT RTE. 1A AT ELLIS RD. STATE SIGNAL ID NO. REVISION SHEET TOTAL NO. NO. SHEETS MASS 0282 01 3

TRAFFIC SIGNAL DATA

NOTES:

SEQUENCE AND TIMING NOTES:

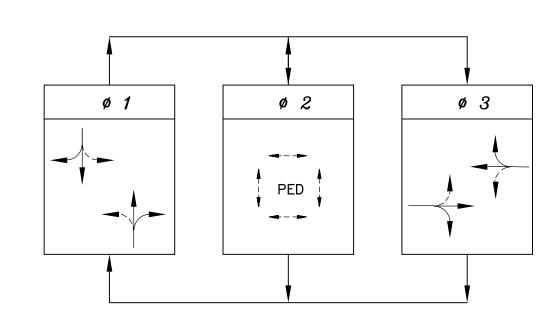
### NEMA DUAL RING PHASING NOTES:

- 1. PHASES ASSOCIATED BY A SOLID LINE SHALL NOT OPERATE CONCURRENTLY.
- 2. PHASES ASSOCIATED BY A DASHED LINE MAY OPERATE CONCURRENTLY.
- 3. THROUGH MOVEMENTS MAY INCLUDE RIGHT TURNS.
- 4. IF THE ASSIGNED RIGHT OF WAY FOR ANY TRAFFIC MOVEMENT IS TO REMAIN IN EFFECT DURING THE NEXT CALLED PHASE, THE SIGNAL INDICATIONS FOR THAT TRAFFIC MOVEMENT SHALL NOT CHANGE DURING THE CHANGE INTERVAL(S) UNLESS OTHERWISE

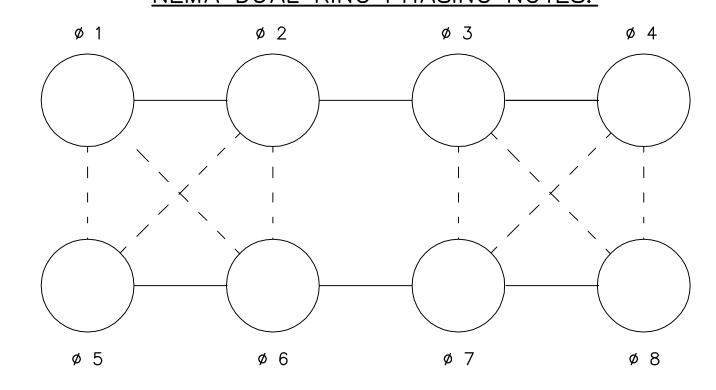
### LOOP DETECTOR NOTES:

- SEE LOOP DETECTOR DETAIL SHEET FROM DESIGN DOCUMENT FOR SPLICE PATTERN AND OTHER INFORMATION.
- 2. DELAY AND EXTENSION TIMES ARE IN SECONDS.
- 3. DELAY TIME SHALL BE EFFECTIVE ONLY DURING THE RED PORTION OF THE PHASE THAT IS CALLED BY THE DETECTOR.

# PREFERENTIAL PHASING SEQUENCE



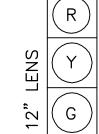
# NEMA DUAL RING PHASING NOTES:



# LOOP DETECTOR DATA

DETECTOR NUMBER	NUMBER OF SEGMENTS	LOOP SIZE	NUM. OF TURNS	ø CALLED	ø EXT.	MODE PULSE PRESENCE	DELAY TIME	EXT. TIME
1	4	6'X6'		Ø ₃	Ø ₃	PRESENCE		
2	4	6'X6'		Ø ₃	Ø ₃	PRESENCE		

# SIGNAL IDENTIFICATION





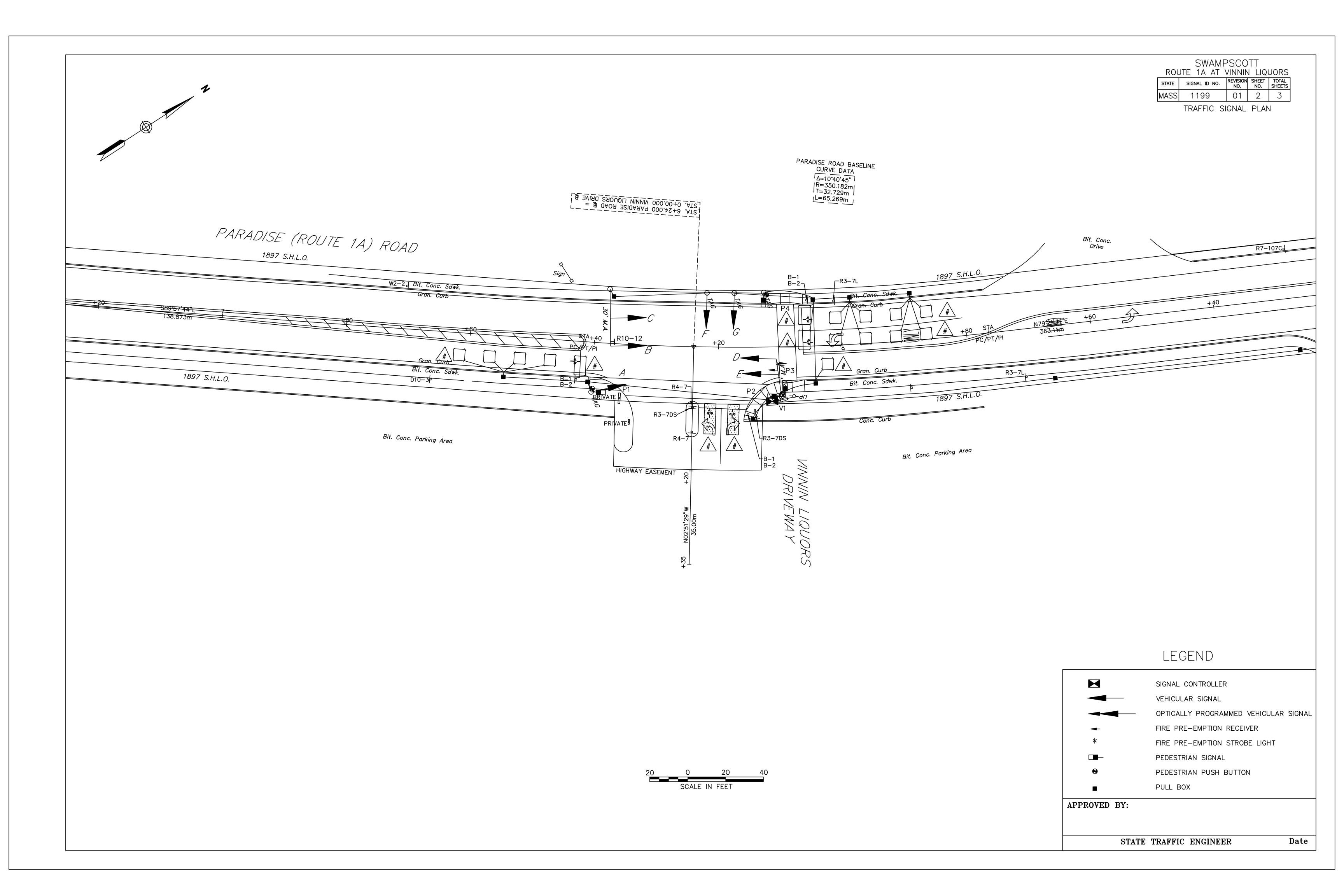
WALK	
12" INCANDESCENT	

CONTROLLER MAKE & MODEL:	EAGLE DP 300
UTILITY POLE No.	MECO 38, NET&T 38/8
METER No.	95 059 822
EMERGENCY PRE-EMPTION (TYPE	E): NONE

APPROVED BY:

STATE TRAFFIC ENGINEER

Date



							SEQU	JENCE	AND	TIMIN	G							
APPROACH	DIRECTION	HOUSING	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	FLASHING
MINIMUM INTERVAL			6			6			4			6						OPERATIO
VEHICLE EXTENSION			2			2			2			2						
MAXIMUM 1			16			45			25			45						
MAXIMUM 2			16			45			25			45						
YELLOW CLEARANCE				4			4			4			4					
RED CLEARANCE					1			1			1			1				
PEDESTRIAN INTERVAL						7/18			7/12									
PARADISE RD	EB	D,E	R	R	R	G	Y	R	R	R	R	R	R	R				FY
PARADISE RD	WB	A,B	(G− _R	(-Y-R)	R	R	R	R	R	R	R	G	Υ	R				FY
PARADISE RD	WB	С	R	R	R	R	R	R	R	R	R	G	Υ	R				FY
DRIVEWAY	NB	F	R	R	R	R	R	R	G	Υ	R	R	R	R				FR
DRIVEWAY	NB	G	R _G	$R_{-Y}$	R	R	R	R	G	Υ	R	R	R	R				FR
PEDESTRIAN X-ING	EB-WB	P1-P2	DW	DW	DW	W/FDW	DW	DW	DW	DW	DW	DW	DW	DW				OUT
PEDESTRIAN X-ING	NB-SB	P3-P4	DW	DW	DW	DW	DW	DW	W/FDW	DW	DW	DW	DW	DW				OUT
DETECTOR			N	ON-LO	CK	NO	ON-LC	OCK	NC	N-LC	)CK	N	ON-LC	CK				
RECALL				OFF			SOFT			OFF			SOFT					
<u>S:</u>				ø1			ø2			ø4			ø6		ø3,ø	5,ø7	& ø8	
JTOMATIC FLASHING OPE U.T.C.D. SECTION 4D.12. NORMALLY DW, W/FDW U USH BUTTON ACTUATION L = OVERLAP ERM = PERMISSIVE				<b>4</b> گ	) OL	7	<b>←</b> — –			<b>R</b> #	†		PI	<b>₹</b> ERM	N	OT US	SED	

COORDINATION DATA (ALL ENTRIES IN SECONDS)

	PLAN 1	PLAN 2	PLAN 3
CYCLE LENGTH	85 SEC	90 SEC	100 SEC
OFFSET	78	81	87
SPLIT Ø1&Ø6	12	12	15
SPLIT ø2&ø6	49	51	55
SPLIT Ø4	24	27	30
COORDINATED PHASE	ø2&ø6	ø2&ø6	ø2&ø6

NOTES: 1. Ø2&Ø6 "CALL NOT ACTUATED" DURING COORDINATION. 2. OFFSET: BEGINNING OF \$2&\$6 GREEN.

3. FLOATING FORCE OFF SHALL BE IN EFFECT.

4. SPLIT TIMES EQUAL GREEN PLUS CLEARANCES.

5. INHIBIT MAX TERMINATION SHALL BE IN EFFECT DURING COORDINATION.

### SEQUENCE & TIMING NOTES:

- 1. IF THE ASSIGNED RIGHT OF WAY FOR ANY TRAFFIC MOVEMENT IS TO REMAIN IN EFFECT DURING THE NEXT CALLED PHASE. THE SIGNAL INDICATIONS FOR THAT TRAFFIC MOVEMENT WILL NOT CHANGE DURING THE CLEARANCE INTERVAL.
- 2. THE RIGHT OF WAY MAY BE ASSIGNED TO ANY PHASE OR ANY COMBINATION OF NON-CONFLICTING PHASES.
- 3. IF CALLS EXIST ON ALL PHASES, THE ASSIGNMENT OF RIGHT OF WAY SHALL BE IN ACCORDANCE WITH THE PREFERENTIAL PHASE SEQUENCE.
- 4. IF THE ASSIGNED RIGHT-OF-WAY FOR ANY TRAFFIC MOVEMENT IS TO CHANGE DURING THE NEXT CALLED PHASE, THE SIGNAL INDICATION FOR THAT MOVEMENT WILL DISPLAY THE APPROPRIATE CLEARANCE INTERVALS.

## DAILY & WEEKLY COORDINATION PROGRAM

	MONDAY THRU FRIDAY	SATURDAY	SUNDAY
PLAN 1 85" CYCLE	0700–1100	_	_
PLAN 2 90" CYCLE	1100-1900	_	_
PLAN 3 100" CYCLE		1000-1800	_
FREE OPERATION	0000-0700 1900-2400	0000-1000 1800-2400	0000-2400
FLASH OPERATION	_	_	_

SWAMPSCOTT ROUTE 1A AT VINNIN LIQUORS STATE SIGNAL ID NO. REVISION SHEET TOTAL NO. NO. SHEETS 1199 01 3 3

TRAFFIC SIGNAL DATA

PRE-EMPTION PHASING & PRIORITY												
DETECTOR & PRIORITY	PRE-EMPT PHASE ASSIGNMENT	MOVEMENT	VEHICLE PHASE ASSIGNMENT									
D1	1	Į,	ø1&ø6									
D2	2	7	ø2									

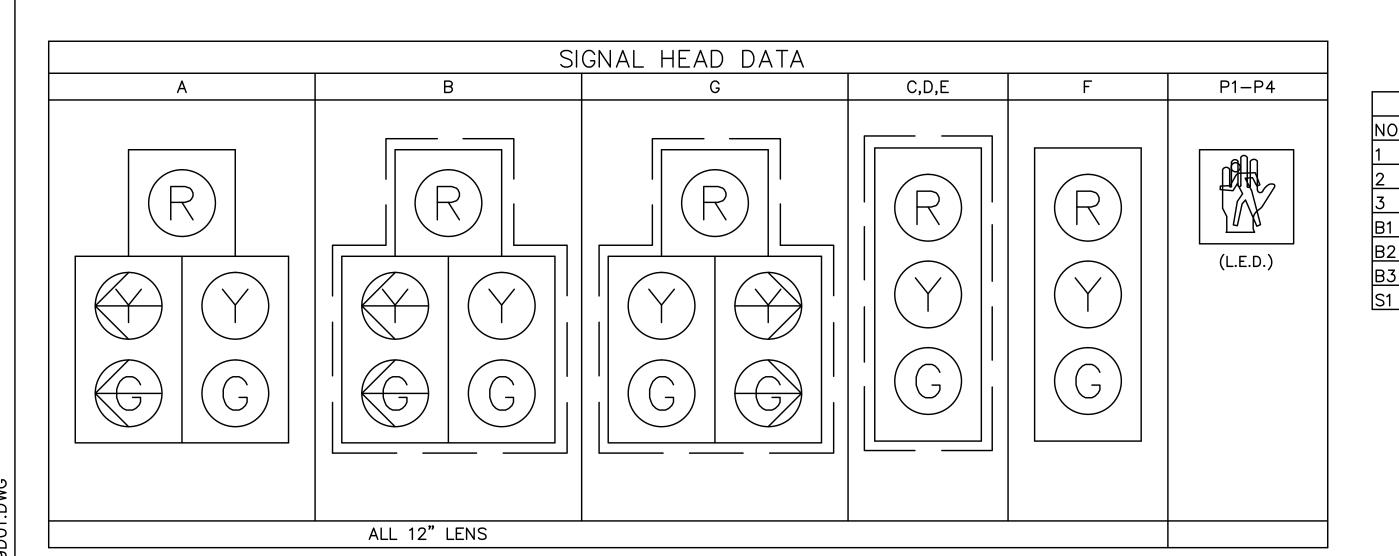
### EMERGENCY VEHICLE PRE-EMPTION OPERATION.

- 1. EMERGENCY VEHICLE PRE-EMPTION SIGNALS SHALL BE OPTICALLY TRANSMITTED BY OPTICAL EMITTERS MOUNTED IN EMERGENCY VEHICLES AND RECEIVED BY OPTICAL DETECTORS LOCATED AT EACH INTERSECTION.
- 2. PRE-EMPTION SIGNALS SHALL BE SERVICED ON A FIRST COME, FIRST SERVE BASIS.
- 3. IN RESPONSE TO A PRE-EMPTION SIGNAL RECEIVED AT AN INTERSECTION BY OPTICAL DETECTOR D1 (OR D2) THE CONTROLLER SHALL HOLD OR ADVANCE TO AND HOLD IN EMERGENCY VEHICLE PRE-EMPTION PHASE #1 (OR #2) GREEN FOR A MINIMUM OF TEN (10) SECONDS OR UNTIL PRE-EMPTION SIGNAL CEASES. THE CONTROLLER SHALL THEN TIME PRE-EMPTION PHASE CLEARANCES FOR THE ASSOCIATED PHASE(S) AS SHOWN IN THE SEQUENCE AND TIMING CHART AND SERVICE SUBSEQUENT EMERGENCY VEHICLE PRE-EMPTION PHASES AS NECESSARY.
- 4. UNLESS OTHERWISE STATED, ONCE A PRE-EMPTION CALL HAS BEEN RECEIVED BY THE TRAFFIC SIGNAL CONTROLLER AND THE PRE-EMPTION PHASE IS BEING SERVICED, IT SHALL REMAIN IN THAT PHASE AS LONG AS THE CALL IS PRESENT.
- 5. MINIMUM GREEN AND NORMAL VEHICLE CLEARANCE SHALL BE PROVIDED ON PHASES THAT ARE TO BE TERMINATED BY PRE-EMPTION DEMAND.
- 6. PRE-EMPTION STROBE SHALL BE ILLUMINATED WHENEVER ANY EMERGENCY VEHICLE PRE-EMPTION GREEN IS ON.
- 7. EMERGENCY VEHICLE PRE-EMPTION SHALL OVERRIDE COORDINATION.

ITEM 816.07 TRAFFIC SIGNAL RECONSTRUCTION PARADISE ROAD @ VINNIN LIQUOR STORE LIST OF MAJOR ITEMS REQUIRED

	LIST OF MITCORY FIELDS TREAGOTIVED
QUANTITY	DESCRIPTION
1	MODIFY EXIST TS PEEK 3000E CONTROLLER & CABINET TO PROPOSED TIMINGS SHOWN
3	PEDESTRIAN PUSH BUTTON W/R10-3f AND SIGN SADDLE
1	TROUBLE-SHOOT & REESTABLISH COMMUNICATION LINK TO MASTER
7	12" CIRCULAR YELLOW L.E.D. MODULES (ALL)
2	12" YELLOW LEFT ARROW L.E.D. MODULES (A,B)
1	12" YELLOW RIGHT ARROW L.E.D. MODULES (A,B)
1	TYPE C, 2-CHANNEL CARD RACK LOOP DETECTOR AMPLIFIER
1	EMERGENCY PRE-EMPTION STROBE (WHITE LENS)
	DILIC NECESSARY DUCT CARLE LARGE MISCELLANEOUS MATERIAL AND

PLUS NECESSARY DUCT, CABLE, LABOR, MISCELLANEOUS MATERIAL AND EQUIPMENT TO COMPLETE THE INSTALLATION AND PROVIDE AN OPERATING TRAFFIC CONTROL SIGNAL.



# DETECTOR SCHEDULE

	DE ⁻	TECTOR		AMP	LIFIER	PHASE PHASE		DELAY/		L(	OOPS		DETECTION	
۷O.	STREET	DIRECTION	LANE	CHANNEL	SETTING	CALLED	EXTENDED	EXTENSION	SIZE (FT)	SEGMENTS	TURNS	CONNECTIONS	MODE	
1	PARADISE RD	SB	LEFT	1	PRESENCE	1	1	4	6 X 6	4	EXISTING	EXISTING	PRESENCE	
2	PARADISE RD	SB	THROUGH	2	PRESENCE	6	6		6 X 6	4	EXISTING	EXISTING	PRESENCE	
3	PARADISE RD	NB	RIGHT/THROUGH	3	PRESENCE	2	2		6 X 6	4	EXISTING	EXISTING	PRESENCE	
31	PARADISE RD	SB	LEFT	6	PRESENCE	1	1	4	6 X 10	1	EXISTING	EXISTING	PRESENCE	
32	PARADISE RD	SB	THROUGH	7	PRESENCE	6	6		6 X 10	1	EXISTING	EXISTING	PRESENCE	
33	PARADISE RD	NB	RIGHT/THROUGH	8	PRESENCE	2	2	6	6 X 10	1	EXISTING	EXISTING	PRESENCE	
S1	PARADISE RD	NB	THROUGH	11	PULSE	_			6 X 6	1	EXISTING	EXISTING	SYSTEM	
•			-			•		•		•				

DETECTOR DATA									
DETECTOR NO.	ZONE SIZE	CAMERA	DELAY /EXT	CALL PHASE	AMPLIFIER CHANNEL				
4	_	V1	4 SEC DELAY	ø4	16				
5	_	V1	6 SEC DELAY	ø4	16				

NOTE: DELAY AND EXTENSION TIMINGS SHALL BE PROGRAMMED IN THE CONTROLLER ONLY

UTILITY POLE No. 138, BELL ATLANTIC 1/138 35492239  EMERGENCY PRE-EMPTION (TYPE): OPTICOM	CONTROLLER MAKE & M	ODEL:	PEEK 3000E
	UTILITY POLE No.		138, BELL ATLANTIC 1/138
EMERGENCY PRE-EMPTION (TYPE): OPTICOM			35492239
	EMERGENCY PRE-EMPTI	ON (TYPE	E): OPTICOM

STATE TRAFFIC ENGINEER

Date

6. MAXIMUM 1 = NORMAL OPERATION

8. STOP AND GO OPERATION FOR 24 HOURS PER DAY.

* UPON PEDESTRIAN PUSH BUTTON ACTUATION

PREFERENTIAL PHASE SEQUENCE

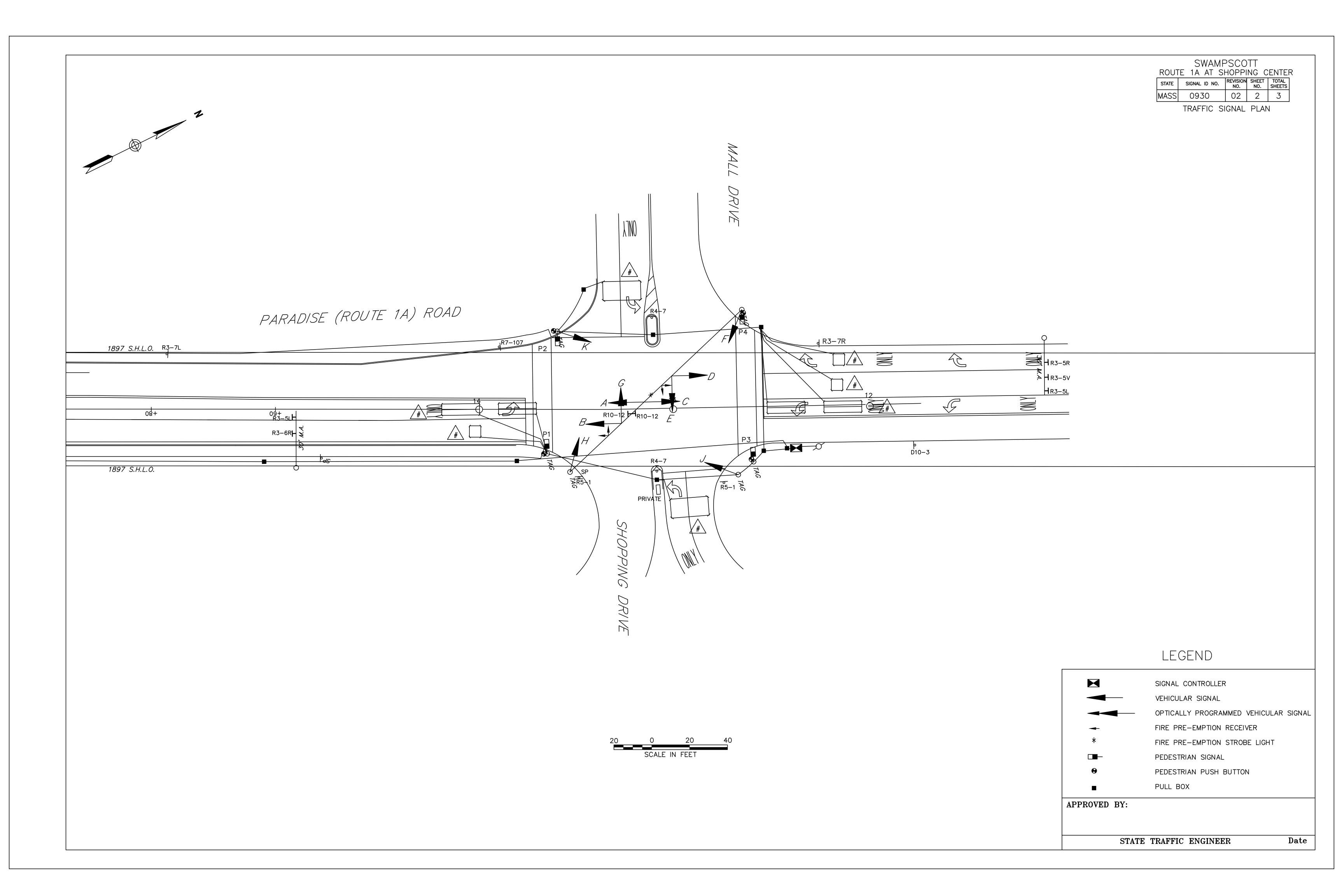
FLASHING OPERATION FOR EMERGENCY ONLY.

9. DURING PEDESTRIAN INTERVAL, FDW THROUGH YELLOW OPERATION SHALL BE IN EFFECT.

10. INHIBIT MAX TERMINATION SHALL BE IN EFFECT

7. MAXIMUM 2 = NOT USED

DURING COORDINATION.



									SEQ	UENCE	E AND	MIT (	IG														
APPROACH	DIRECTION	HOUSING	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	FLASHING
MINIMUM INTERVAL			6			10						8			6			10			8						OPERATION
VEHICLE EXTENSION			1			3						2			1			2			2						
MAXIMUM 1			15			35						25			15			35			15						
MAXIMUM 2			15			25						25			15			25			15						
YELLOW CLEARANCE				4			4						4			4			4			4					
RED CLEARANCE					1			1						1			1			1			1				
PEDESTRIAN INTERVAL									7	16	1																
PARADISE RD	NB	A	R	R	R	G	Y	R	R	R	R	R	R	R	⊬G− _R	<del>(</del> Y- _R	R	R	R	R	R	R	R				FY
PARADISE RD	NB	B,J	R	R	R	G	Υ	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R				FY
PARADISE RD	SB	С	(G-R	(Y-R)	R	R	R	R	R	R	R	R	R	R	R	R	R	G	Υ	R	R	R	R				FY
PARADISE RD	SB	D,K	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	G	Υ	R	R	R	R				FY
SHOPPING DRIVE	EB	E,F	R	R	R	R	R	R	R	R	R	G	Y	R	R	R	R	R	R	R	R	R	R				FR
MALL DR	WB	G,H	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	G	Υ	R				FR
PEDESTRIAN X-ING	ALL	ALL	DW	DW	DW	DW	DW	DW	W	FDW	DW	DW	DW	DW	DW	DW	DW	DW	DW	DW	DW	DW	DW				OUT
DETECTOR			NO	L DN-LC	L OCK		LOCK		NO	L DN-LC	L DCK	N	L DN-LC	L OCK	NO	L DN-LC	L DCK		LOCK		N	l ON-LO	L DCK				
RECALL				OFF			SOFT			OFF			OFF			OFF			SOFT			OFF					-
<u>S:</u>				ø1			ø2			ø3*			ø4			ø5			ø6			ø7			ø8		1
TOMATIC FLASHING OPE J.T.C.D. SECTION 4D.12.		? <del>.</del>		<u> </u>			]\			<u> </u>			11			]\			]\			]\					1

**SEQUENCE & TIMING NOTES:** 

- 1. IF THE ASSIGNED RIGHT OF WAY FOR ANY TRAFFIC MOVEMENT IS TO REMAIN IN EFFECT DURING THE NEXT CALLED PHASE, THE SIGNAL INDICATIONS FOR THAT TRAFFIC MOVEMENT WILL NOT CHANGE DURING THE CLEARANCE INTERVAL.
- 2. THE RIGHT OF WAY MAY BE ASSIGNED TO ANY PHASE OR ANY COMBINATION OF NON-CONFLICTING PHASES.
- 3. IF CALLS EXIST ON ALL PHASES, THE ASSIGNMENT OF RIGHT OF WAY SHALL BE IN ACCORDANCE WITH THE PREFERENTIAL PHASE SEQUENCE.
- 4. IF THE ASSIGNED RIGHT-OF-WAY FOR ANY TRAFFIC MOVEMENT IS TO CHANGE DURING THE NEXT CALLED PHASE, THE SIGNAL INDICATION FOR THAT MOVEMENT WILL DISPLAY THE APPROPRIATE CLEARANCE INTERVALS.

SWAMPSCOTT ROUTE 1A AT SHOPPING CENTER SIGNAL ID NO. REVISION SHEET TOTAL NO. NO. SHEETS 0930 02 3 3

TRAFFIC SIGNAL DATA

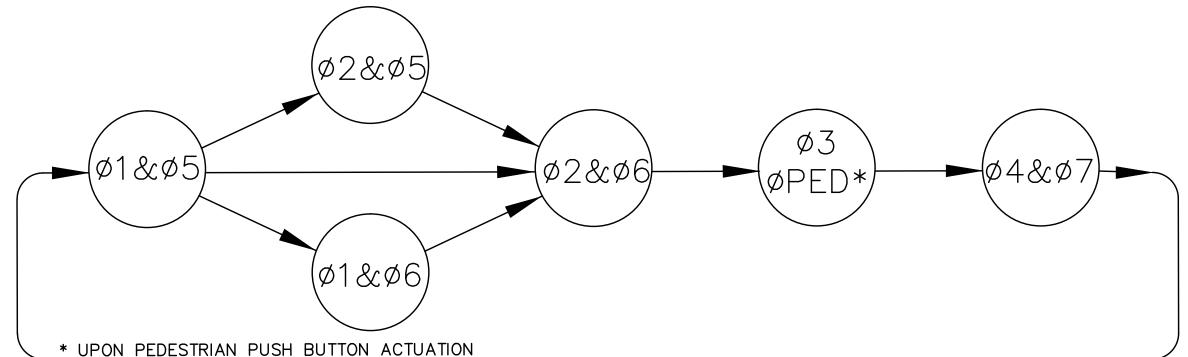
PRE-EMPTION									
PHASING & PRIORITY									
DETECTOR & PRIORITY	PRE-EMPT PHASE ASSIGNMENT	MOVEMENT	VEHICLE PHASE ASSIGNMENT						
D1	1	$\Rightarrow$	ø2&ø5						
D2	2	JIL	ø1&ø6						
D3	3	4	ø4						
D4	4	71	ø7						

### DAILY & WEEKLY COORDINATION PROGRAM

		<u> </u>	
	MONDAY THRU FRIDAY	SATURDAY	SUNDAY
PLAN 1 85" CYCLE	0700–1100	-	
PLAN 2 90" CYCLE	1100-1900	-	ı
PLAN 3 100" CYCLE		1000–1800	ı
FREE OPERATION	0000-0700 1900-2400	0000-1000 1800-2400	0000-2400
FLASH OPERATION	_	_	_

EMERGENCY VEHICLE PRE-EMPTION OPERATION.

- 1. EMERGENCY VEHICLE PRE-EMPTION SIGNALS SHALL BE OPTICALLY TRANSMITTED BY OPTICAL EMITTERS MOUNTED IN EMERGENCY VEHICLES AND RECEIVED BY OPTICAL DETECTORS LOCATED AT EACH INTERSECTION.
- 2. PRE-EMPTION SIGNALS SHALL BE SERVICED ON A FIRST COME, FIRST SERVE BASIS.
- 3. IN RESPONSE TO A PRE-EMPTION SIGNAL RECEIVED AT AN INTERSECTION BY OPTICAL DETECTOR D1 (OR D2, D3, D4) THE CONTROLLER SHALL HOLD OR ADVANCE TO AND HOLD IN EMERGENCY VEHICLE PRE-EMPTION PHASE #1 (OR #2, #3, #4) GREEN FOR A MINIMUM OF TEN (10) SECONDS OR UNTIL PRE-EMPTION SIGNAL CEASES. THE CONTROLLER SHALL THEN TIME PRE-EMPTION PHASE CLEARANCES FOR THE ASSOCIATED PHASE(S) AS SHOWN IN THE SEQUENCE AND TIMING CHART AND SERVICE SUBSEQUENT EMERGENCY VEHICLE PRE-EMPTION PHASES AS NECESSARY.
- 4. UNLESS OTHERWISE STATED, ONCE A PRE-EMPTION CALL HAS BEEN RECEIVED BY THE TRAFFIC SIGNAL CONTROLLER AND THE PRE-EMPTION PHASE IS BEING SERVICED, IT SHALL REMAIN IN THAT PHASE AS LONG AS THE CALL IS PRESENT.
- 5. MINIMUM GREEN AND NORMAL VEHICLE CLEARANCE SHALL BE PROVIDED ON PHASES THAT ARE TO BE TERMINATED BY PRE-EMPTION DEMAND.
- 6. PRE-EMPTION STROBE SHALL BE ILLUMINATED WHENEVER ANY
- EMERGENCY VEHICLE PRE-EMPTION GREEN IS ON. 7. EMERGENCY VEHICLE PRE-EMPTION SHALL OVERRIDE COORDINATION.



PREFERENTIAL PHASE SEQUENCE

PERM 🗦

## COORDINATION DATA (ALL ENTRIES IN SECONDS)

	PLAN 1	PLAN 2	PLAN 3
CYCLE LENGTH	85 SEC	90 SEC	100 SEC
OFFSET	0	0	0
SPLIT ø1&ø5	12 (12)	12 (12)	12 (12)
SPLIT ø2&ø6	59 (34)	54 (29)	53 (28)
SPLIT Ø3 PED	- (25)	- (25)	- (25)
SPLIT Ø4&Ø7	14 (14)	24 (24)	35 (35)
COORDINATED PHASE	ø2&ø6	ø2&ø6	ø2&ø6

NOT USED

NOTES: 1. Ø2&Ø6 "CALL NOT ACTUATED" DURING COORDINATION.

- 2. OFFSET: BEGINNING OF \$2&\$6 GREEN.
- 3. FLOATING FORCE OFF SHALL BE IN EFFECT.
- 4. SPLIT TIMES EQUAL GREEN PLUS CLEARANCES. 5. () = SPLIT TIMES WITH PEDESTRIAN PHASE ACTUATED.
- 6. INHIBIT MAX TERMINATION SHALL BE IN EFFECT
- DURING COORDINATION.
- 7. PERMISSIVE MODE SHALL BE IN EFFECT.

### ITEM 816.06 TRAFFIC SIGNAL RECONSTRUCTION PARADISE ROAD @ SWAMPSCOTT MALL/SHOPPING CENTER

	LIST OF MAJOR ITEMS REQUIRED								
QUANTITY	DESCRIPTION								
1	MODIFY EXIST TS PEEK 3000E CONTROLLER & CABINET TO PROPOSED PHASING & TIMINGS SHOWN								
3	1-SECTION PEDESTRIAN SIGNAL HEAD (L.E.D.)								
4	PEDESTRIAN PUSH BUTTON W/R10-3f AND SIGN SADDLE								
9	WIRE LOOP DETECTOR								
1	EMERGENCY PRE-EMPTION OPTICAL DETECTORS (MODEL 722) & DETECTOR CABLE								
10	12" CIRCULAR YELLOW L.E.D. MODULES (ALL)								
8	12" CIRCULAR GREEN L.E.D. MODULES								
3	SIGNAL HEAD VISORS (TUNNEL)								
1	REPAIR DAMAGED PB								

PLUS NECESSARY DUCT, CABLE, LABOR, MISCELLANEOUS MATERIAL AND EQUIPMENT TO COMPLETE THE INSTALLATION AND PROVIDE AN OPERATING TRAFFIC CONTROL SIGNAL.

# DETECTOR SCHEDULE

	DET	ECTOR			AMPLIFI	ER	PHASE	PHASE	DELAY/		L	00PS		DETECTION
NO.	STREET	DIRECTION	LANE	NO.	CHANNEL	SETTING	CALLED	EXTENDED	EXTENSION	SIZE (FT)	SEGMENTS	TURNS	CONNECTIONS	MODE
1	PARADISE RD	NB	THROUGH	1	1	PRESENCE	2	2		6 X 6	1	EXISTING	1	PRESENCE
2	PARADISE RD	NB	LEFT	1	2	PRESENCE	5	5		6 X 20	2	EXISTING	EXISTING	PRESENCE
3	PARADISE RD	SB	RIGHT	2	1	PRESENCE	6	6		6 X 6	1	EXISTING	1	PRESENCE
4	PARADISE RD	SB	THROUGH	2	2	PRESENCE	6	6		6 X 6	1	EXISTING	-	PRESENCE
5	PARADISE RD	SB	LEFT	3	1	PRESENCE	1	1		6 X 20	2	EXISTING	EXISTING	PRESENCE
6	SHOPPING DRIVE	WB	ALL	3	2	PRESENCE	7	7		10 X 20	1	EXISTING	_	PRESENCE
7	MALL DRIVE	EB	ALL	4	1	PRESENCE	4	4		10 X 20	1	EXISTING	_	PRESENCE

CONTROLLER MAKE & MODEL:	PEEK 3000E
UTILITY POLE No.	NET&T 156, BELL ATLANTIC 1/156
METER No.	NO METER
EMERGENCY PRE-EMPTION (T)	YPE): OPTICOM
APPROVED BY:	

Date

(L.E.D.) BI-MODAL LENS-ALL 12" LENS

SIGNAL HEAD DATA

P1-P4

2. * UPON PEDESTRIAN PUSH BUTTON ACTUATION

5. MAXIMUM 2 = 11:00AM - 7:00PM, SUN-SAT

7. STOP AND GO OPERATION FOR 24 HOURS PER DAY.

FLASHING OPERATION FOR EMERGENCY ONLY.

8. DURING PEDESTRIAN INTERVAL, FDW THROUGH

9. INHIBIT MAX TERMINATION SHALL BE IN EFFECT

YELLOW OPERATION SHALL BE IN EFFECT.

3. PERM = PERMISSIVE

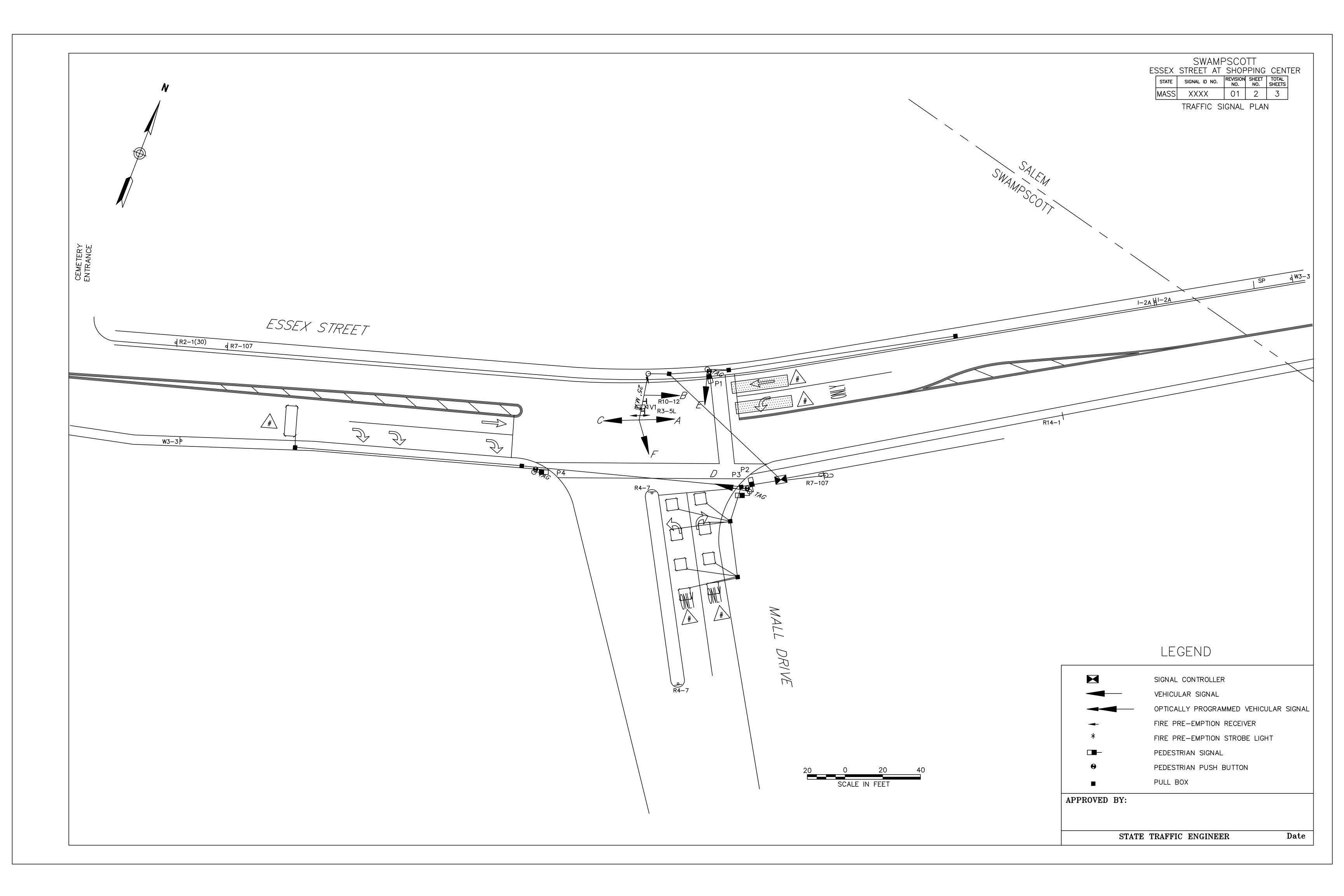
4. MAXIMUM 1 = ALL OTHER TIMES

6. Ø2 OMITS Ø5; Ø6 OMITS Ø1

DURING COORDINATION.

B,D,E,F,G,H,J,K

STATE TRAFFIC ENGINEER



							SEQU	JENCE	AND	TIMIN	G										
APPROACH	DIRECTION	HOUSING	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	FLASHING
MINIMUM INTERVAL			6			10						6			10						OPERATIO
VEHICLE EXTENSION			2			4						2			4						
MAXIMUM 1			10			35						20			35						
MAXIMUM 2			15			50						25			50						
YELLOW CLEARANCE				4			4						4			4					
RED CLEARANCE					1			1						1			1				
PEDESTRIAN INTERVAL									7	27	1										
ESSEX ST	EB	C,D	R	R	R	G	Υ	R	R	R	R	R	R	R	R	R	R				FY
ESSEX ST	WB	Α	←G-R	(-Y− _R	R	R	R	R	R	R	R	R	R	R	G	Υ	R				FY
ESSEX ST	WB	В	R	R	R	R	R	R	R	R	R	R	R	R	G	Υ	R				FY
MALL DRIVE	NB	E	$R_{-G}$	$R_{-Y}$	R	R	R	R	R	R	R	G	Υ	R	R	R	R				FR
MALL DRIVE	NB	F	R	R	R	R	R	R	R	R	R	G	Υ	R	R	R	R				FR
PEDESTRIAN X-ING	ALL	ALL	DW	DW	DW	DW	DW	DW	W	FDW	DW	DW	DW	DW	DW	DW	DW				OUT
DETECTOR			NO	L DN-LC	L DCK		LOCK		N	L DN-LC	L CK		LOCK		N	L ON-LO	L DCK				
RECALL				OFF			SOFT			OFF			OFF			SOFT	-				
<u>S:</u>				ø1			ø2			ø3			ø4			ø6		ø5,	ø7 &	ø8	
TOMATIC FLASHING OPE J.T.C.D. SECTION 4D.12. UPON PEDESTRIAN PUSI = OVERLAP RM = PERMISSIVE XIMUM 1 = NORMAL OF XIMUM 2 = 3PM - 7PI	H BUTTON A		17	1	) OLA	<b>→</b>	ጎሶ	Λ	1/	•	<b>†</b>	17	\ <b>\</b>	[ DLA	17	F 1/	<b>₽</b> ERM		OT US		

SEQUENCE & TIMING NOTES:

SIGNAL HEAD DATA

- 1. IF THE ASSIGNED RIGHT OF WAY FOR ANY TRAFFIC MOVEMENT IS TO REMAIN IN EFFECT DURING THE NEXT CALLED PHASE, THE SIGNAL INDICATIONS FOR THAT TRAFFIC MOVEMENT WILL NOT CHANGE DURING THE CLEARANCE INTERVAL.
- 2. THE RIGHT OF WAY MAY BE ASSIGNED TO ANY PHASE OR ANY COMBINATION OF NON-CONFLICTING PHASES.
- 3. IF CALLS EXIST ON ALL PHASES, THE ASSIGNMENT OF RIGHT OF WAY SHALL BE IN ACCORDANCE WITH THE PREFERENTIAL PHASE SEQUENCE.
- 4. IF THE ASSIGNED RIGHT-OF-WAY FOR ANY TRAFFIC MOVEMENT IS TO CHANGE DURING THE NEXT CALLED PHASE, THE SIGNAL INDICATION FOR THAT MOVEMENT WILL DISPLAY THE APPROPRIATE CLEARANCE INTERVALS.

SWAMPSCOTT ESSEX STREET AT SHOPPING CENTER STATE SIGNAL ID NO. REVISION SHEET TOTAL SHEETS MASS XXXX 01 3

TRAFFIC SIGNAL DATA

	PRE-E	MPTION									
PHASING & PRIORITY											
DETECTOR & PRIORITY	PRE-EMPT PHASE ASSIGNMENT	MOVEMENT	VEHICLE PHASE ASSIGNMENT								
D1	1	11	ø2								
D2	2	Ц	ø1&ø6								
D3	3	71	ø4								

#### EMERGENCY VEHICLE PRE-EMPTION OPERATION.

- 1. EMERGENCY VEHICLE PRE-EMPTION SIGNALS SHALL BE OPTICALLY TRANSMITTED BY OPTICAL EMITTERS MOUNTED IN EMERGENCY VEHICLES AND RECEIVED BY OPTICAL DETECTORS LOCATED AT EACH INTERSECTION.
- 2. PRE-EMPTION SIGNALS SHALL BE SERVICED ON A FIRST COME, FIRST SERVE BASIS.
- 3. IN RESPONSE TO A PRE-EMPTION SIGNAL RECEIVED AT AN INTERSECTION BY OPTICAL DETECTOR D1 (OR D2, D3) THE CONTROLLER SHALL HOLD OR ADVANCE TO AND HOLD IN EMERGENCY VEHICLE PRE-EMPTION PHASE #1 (OR #2, #3) GREEN FOR A MINIMUM OF TEN (10) SECONDS OR UNTIL PRE-EMPTION SIGNAL CEASES. THE CONTROLLER SHALL THEN TIME PRE-EMPTION PHASE CLEARANCES FOR THE ASSOCIATED PHASE(S) AS SHOWN IN THE SEQUENCE AND TIMING CHART AND SERVICE SUBSEQUENT EMERGENCY VEHICLE PRE-EMPTION PHASES AS NECESSARY.
- 4. UNLESS OTHERWISE STATED, ONCE A PRE-EMPTION CALL HAS BEEN RECEIVED BY THE TRAFFIC SIGNAL CONTROLLER AND THE PRE-EMPTION PHASE IS BEING SERVICED, IT SHALL REMAIN IN THAT PHASE AS LONG AS THE CALL IS PRESENT.
- 5. MINIMUM GREEN AND NORMAL VEHICLE CLEARANCE SHALL BE PROVIDED ON PHASES THAT ARE TO BE TERMINATED BY PRE-EMPTION DEMAND.
- 6. PRE-EMPTION STROBE SHALL BE ILLUMINATED WHENEVER ANY EMERGENCY VEHICLE PRE-EMPTION GREEN IS ON.

R	R Y G	(L.E.D.)	QUAN 1 3 2
			1

P1-P4

# PREFERENTIAL PHASE SEQUENCE

* UPON PEDESTRIAN PUSH BUTTON ACTUATION

7. STOP AND GO OPERATION FOR 24 HOURS PER DAY.

FLASHING OPERATION FOR EMERGENCY ONLY.

8. DURING PEDESTRIAN INTERVAL, FDW THROUGH

YELLOW OPERATION SHALL BE IN EFFECT.

	DETECTOR DATA										
DETECTOR NO.	ZONE SIZE	CAMERA	DELAY /EXT	CALL PHASE							
1	TO BE FIELD ADJUSTED	V1	0	ø1&ø6							
2	TO BE FIELD ADJUSTED	V1	0	ø6							

NOTE: DELAY AND EXTENSION TIMINGS SHALL BE PROGRAMMED IN THE CONTROLLER ONLY

	ALL 12" LENS	
	ALL IZ LLINS	

ITEM 816.05
TRAFFIC SIGNAL RECONSTRUCTION ESSEX STREET @ MALL DRIVEWAY

	LIST OF MAJOR ITEMS REQUIRED
QUANTITY	
1	MODIFY EXISTING TS PEEK 3000E CONTROLLER AND CABINET TO PROPOSED TIMINGS SHOWN
3	PEDESTRIAN PUSH BUTTON W/R10-3f AND SIGN SADDLE
2	AUDIBLE PED DEVICE
1	PULL BOX-12"x12" - FRAME & COVER
1	VIDEO DETECTION SYSTEM (1 CAMERA. VDP & CABLES)
6	12" CIRCULAR YELLOW L.E.D. MODULES (ALL)
1	12" YELLOW LEFT ARROW L.E.D. MODULES (A)
1	12" YELLOW RIGHT ARROW L.E.D. MODULES (E)

PLUS NECESSARY DUCT, CABLE, LABOR, MISCÉLLANEOUS MATERIAL AND EQUIPMENT TO COMPLETE THE INSTALLATION AND PROVIDE AN OPERATING TRAFFIC CONTROL SIGNABESCRIPTION

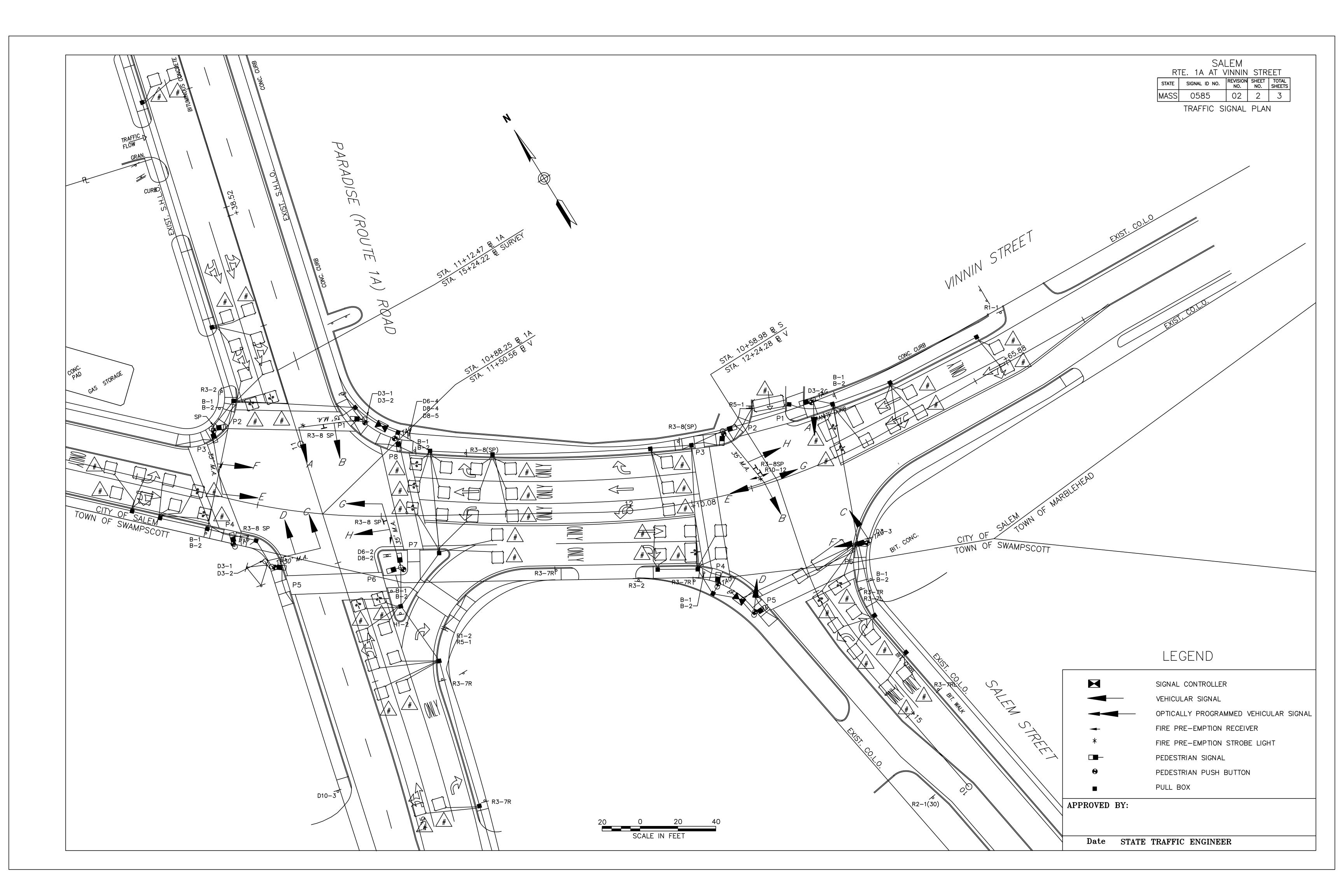
CONTROLLER MAKE & MODEL: P	PEEK 3000E
UTILITY POLE No.	IET&T 17, 135
METER No. 8	86 400 280
EMERGENCY PRE-EMPTION (TYPE)	: OPTICOM
APPROVED BY:	

### DETECTOR SCHEDULE

	DET	ECTOR			AMPLIF	IER	PHASE	PHASE	DELAY/		L(	OOPS		DETECTION
NO.	STREET	DIRECTION	LANE	NO.	CHANNEL	SETTING	CALLED	EXTENDED	EXTENSION	SIZE (FT)	SEGMENTS	TURNS	CONNECTIONS	MODE
3	ESSEX STREET	EB	THROUGH	2	1	PRESENCE	2	2		6 X 16	1	EXISTING	1	PRESENCE
4	MALL DRIVE	NB	LEFT	2	2	PRESENCE	4	4		6 X 6	4	EXISTING	EXISTING	PRESENCE
5	MALL DRIVE	NB	RIGHT	3	1	PRESENCE	7	4&7		6 X 6	4	EXISTING	EXISTING	PRESENCE

STATE TRAFFIC ENGINEER

Date



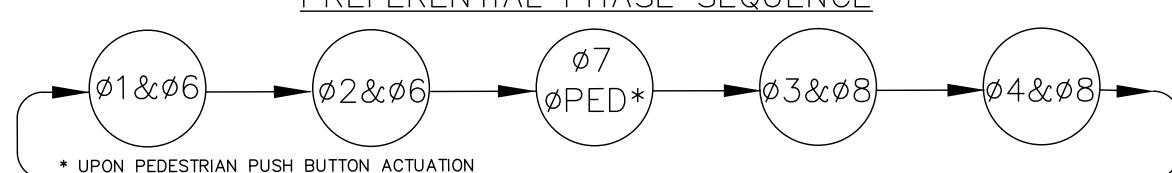
										SEQUE	NCE	AND 1	ΓIMING													_	
APPROACH	DIRECTION	HOUSING	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	FLASHING
MINIMUM INTERVAL			6			10			6			8			10						8						OPERATION
VEHICLE EXTENSION			1			2			1			3			2						2						
MAXIMUM 1			12			25			15			35			25						35						
MAXIMUM 2			14			25			15			40			25						40						
YELLOW CLEARANCE				4			4			4			4			4			3			4					
RED CLEARANCE					1			1			1			1			1						1				
PEDESTRIAN INTERVAL																		7	15	0							
PARADISE RD	NB	A,B	R	R	R	G	Υ	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R				FY
PARADISE RD	SB	С	$\leftarrow$ G $-$ R	$\leftarrow$ Y $-$ R	R	G	Υ	R	R	R	R	R	R	R	G	Υ	R	R	R	R	R	R	R				FY
PARADISE RD	SB	D	R	R	R	G	Υ	R	R	R	R	R	R	R	G	Υ	R	R	R	R	R	R	R				FY
VINNIN ST	EB	G,H	R	R	R	R	R	R	R	R	R	G	Υ	R	R	R	R	R	R	R	R	R	R				FR
VINNIN ST	WB	E	R	R	R	R	R	R	←G- _R	(Y-R)	R	R	R	R	R	R	R	R	R	R	G	Y	R				FR
VINNIN ST	WB	F	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	G	Y	R				FR
PEDESTRIAN X-ING	ALL	ALL	DW	DW	DW	DW	DW	DW	DW	DW	DW	DW	DW	DW	DW	DW	DW	W	FDW	DW	DW	DW	DW				OUT
DETECTOR			NO	DN-LO	CK	N	ON-LC	OCK	NO	ON-LO	CK	NC	ON-LC	CK	NO	ON-LC	OCK	N	ON-LC	CK	NO	ON-LC	OCK				
RECALL				OFF			SOFT	,		OFF			OFF			SOFT			OFF			OFF					
<u>.                                    </u>				ø1			ø2			ø3			ø4			ø6			ø7*			ø8		ø	5 & 9	<b>ø</b> 7	
FOMATIC FLASHING OPE L.T.C.D. SECTION 4D.12.	RATION PER	2														<u> </u>											1

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### 1. AU

- M.U.T.C.D. SECTION 4D.12. 2. * UPON PEDESTRIAN PUSH BUTTON ACTUATION
- 3. PERM = PERMISSIVE
- 4. Y = YIELD CONTROL
- 5. Ø4 & Ø8 DUAL ENTRY
- 6. MAXIMUM 1 = ALL OTHER TIMES
- 7. MAXIMUM 2 = 11:00AM 7:00PM, SUN-SAT
- 8. STOP AND GO OPERATION FOR 24 HOURS PER DAY. FLASHING OPERATION FOR EMERGENCY ONLY.
- 9. DURING PEDESTRIAN INTERVAL, FDW THROUGH YELLOW OPERATION SHALL BE IN EFFECT.
- 10. INHIBIT MAX TERMINATION SHALL BE IN EFFECT DURING COORDINATION.

### PREFERENTIAL PHASE SEQUENCE



#### **SEQUENCE & TIMING NOTES:**

1. IF THE ASSIGNED RIGHT OF WAY FOR ANY TRAFFIC MOVEMENT IS TO REMAIN IN EFFECT DURING THE NEXT CALLED PHASE, THE SIGNAL INDICATIONS FOR THAT TRAFFIC MOVEMENT WILL NOT CHANGE DURING THE CLEARANCE INTERVAL.

PERM

NOT USED

- 2. THE RIGHT OF WAY MAY BE ASSIGNED TO ANY PHASE OR ANY COMBINATION OF NON-CONFLICTING PHASES.
- 3. IF CALLS EXIST ON ALL PHASES, THE ASSIGNMENT OF RIGHT OF WAY SHALL BE IN ACCORDANCE WITH THE PREFERENTIAL PHASE SEQUENCE.
- 4. IF THE ASSIGNED RIGHT-OF-WAY FOR ANY TRAFFIC MOVEMENT IS TO CHANGE DURING THE NEXT CALLED PHASE. THE SIGNAL INDICATION FOR THAT MOVEMENT WILL DISPLAY THE APPROPRIATE CLEARANCE INTERVALS.

### DETECTOR SCHEDULE

	DE.	TECTOR		AMPL	IFIER	PHASE	PHASE	DELAY/		L	OOPS		DETECTION
NO.	STREET	DIRECTION	LANE	CHANNEL	SETTING	CALLED	EXTENDED	EXTENSION	SIZE (FT)	SEGMENTS	TURNS	CONNECTIONS	MODE
1	PARADISE RD	NB	LEFT/THROUGH	1	PRESENCE	2	2		6 X 6	3	3	SERIES	PRESENCE
2	PARADISE RD	NB	LEFT/THROUGH	2	PRESENCE	ı	_		6 X 6	1	3	_	SYSTEM
3	PARADISE RD	NB	THROUGH	3	PRESENCE	ı	_		6 X 6	1	3	_	SYSTEM
4	PARADISE RD	NB	THROUGH	4	PRESENCE	2	2		6 X 6	3	3	SERIES	PRESENCE
5	PARADISE RD	SB	LEFT/THROUGH	5	PRESENCE	1&6	1&6		6 X 6	3	3	SERIES	PRESENCE
6	PARADISE RD	SB	LEFT/THROUGH	6	PRESENCE	ı	_		6 X 6	1	3	_	SYSTEM
7	PARADISE RD	SB	RIGHT/THROUGH	7	PRESENCE	1	_		6 X 6	1	3	_	SYSTEM
8	PARADISE RD	SB	RIGHT/THROUGH	8	PRESENCE	6	6		6 X 6	3	3	SERIES	PRESENCE
9	VINNIN STREET	EB	LEFT	9	PRESENCE	4	4		6 X 6	3	3	SERIES	PRESENCE
10	VINNIN STREET	EB	RIGHT/THROUGH	10	PRESENCE	4	4		6 X 6	3	3	SERIES	PRESENCE
11	VINNIN STREET	EB	LEFT	11	PRESENCE	1	_		6 X 6	1	3	_	SYSTEM
12	VINNIN STREET	EB	THROUGH	12	PRESENCE	-	_		6 X 6	1	3	_	SYSTEM
13	VINNIN STREET	WB	LEFT	13	PRESENCE	3	3		6 X 6	3	3	SERIES	PRESENCE
14	VINNIN STREET	WB	THROUGH	14	PRESENCE	8	8		6 X 6	3	3	SERIES	PRESENCE
15	VINNIN STREET	WB	RIGHT	15	PRESENCE	8	8		6 X 6	3	3	SERIES	PRESENCE
16	VINNIN STREET	WB	LEFT	16	PRESENCE	_	_		6 X 6	1	3	_	SYSTEM
17	VINNIN STREET	WB	THROUGH	17	PRESENCE	-	_		6 X 6	1	3	_	SYSTEM
18	VINNIN STREET	WB	RIGHT	18	PRESENCE	1	_		6 X 6	1	3	_	SYSTEM
B1	PARADISE RD	NB	LEFT/THROUGH	19	PRESENCE	2	2		6 X 6	1	4	_	BICYCLE
B4	PARADISE RD	NB	THROUGH	20	PRESENCE	2	2		6 X 6	1	4	_	BICYCLE
B5	PARADISE RD	SB	LEFT/THROUGH	21	PRESENCE	1	1		6 X 6	1	4	_	BICYCLE
B8	PARADISE RD	SB	RIGHT/THROUGH	22	PRESENCE	6	6		6 X 6	1	4	_	BICYCLE
В9	VINNIN STREET	EB	LEFT	23	PRESENCE	4	4		6 X 6	1	4	_	BICYCLE
B10	VINNIN STREET	EB	RIGHT/THROUGH	24	PRESENCE	4	4		6 X 6	1	4	_	BICYCLE
B13	VINNIN STREET	WB	LEFT	25	PRESENCE	3	3		6 X 6	1	4	_	BICYCLE
B14	VINNIN STREET	WB	THROUGH	26	PRESENCE	8	8		6 X 6	1	4	_	BICYCLE
B15	VINNIN STREET	WB	RIGHT	27	PRESENCE	8	8		6 X 6	1	4	_	BICYCLE

#### EMERGENCY VEHICLE PRE-EMPTION OPERATION.

DAILY & WEEKLY COORDINATION PROGRAM

SATURDAY

1000-1800

0000-1000 1800-2400

MONDAY

FRIDAY

0700-1100

1100-1900

0000-0700

OPERATION | 1900-2400

100" CYCLE

PLAN 2

100" CYCLE

PLAN 3

90" CYCLE

FLASH OPERATION

- 1. EMERGENCY VEHICLE PRE-EMPTION SIGNALS SHALL BE OPTICALLY TRANSMITTED BY OPTICAL EMITTERS MOUNTED IN EMERGENCY VEHICLES AND RECEIVED BY OPTICAL DETECTORS LOCATED AT EACH INTERSECTION.
- 2. PRE-EMPTION SIGNALS SHALL BE SERVICED ON A FIRST COME, FIRST SERVE BASIS.
- 3. IN RESPONSE TO A PRE-EMPTION SIGNAL RECEIVED AT AN INTERSECTION BY OPTICAL DETECTOR D1 (OR D2, D3) THE CONTROLLER SHALL HOLD OR ADVANCE TO AND HOLD IN EMERGENCY VEHICLE PRE-EMPTION PHASE #1 (OR #2, #3) GREEN FOR A MINIMUM OF TEN (10) SECONDS OR UNTIL PRE-EMPTION SIGNAL CEASES. THE CONTROLLER SHALL THEN TIME PRE-EMPTION PHASE CLEARANCES FOR THE ASSOCIATED PHASE(S) AS SHOWN IN THE SEQUENCE AND TIMING CHART AND SERVICE SUBSEQUENT EMERGENCY VEHICLE PRE-EMPTION PHASES AS NECESSARY.
- 4. UNLESS OTHERWISE STATED, ONCE A PRE-EMPTION CALL HAS BEEN RECEIVED BY THE TRAFFIC SIGNAL CONTROLLER AND THE PRE-EMPTION PHASE IS BEING SERVICED, IT SHALL REMAIN IN THAT PHASE AS LONG AS THE CALL IS PRESENT.
- 5. MINIMUM GREEN AND NORMAL VEHICLE CLEARANCE SHALL BE PROVIDED ON PHASES THAT ARE TO BE TERMINATED BY PRE-EMPTION DEMAND.
- 6. PRE-EMPTION STROBE SHALL BE ILLUMINATED WHENEVER ANY EMERGENCY VEHICLE PRE-EMPTION GREEN IS ON.
- 7. EMERGENCY VEHICLE PRE-EMPTION SHALL OVERRIDE COORDINATION.

SUNDAY

0000-2400

			SA	L	EM		
ROL	JTE	1A	ΑT	\	/INNIN	STR	EET
STATE	SIG	SNAL II	D NO.		REVISION NO.	SHEET NO.	TOTA SHEE
MASS		058	5		02	3	

TRAFFIC SIGNAL DATA

PRE-EMPTION										
PHASING & PRIORITY										
TECTOR & RIORITY	PRE-EMPT PHASE ASSIGNMENT	MOVEMENT	VEHICLE PHASE ASSIGNMENT							
D1	1	<del>†</del>	ø2							
D2	2	<b> →</b>	ø1&ø6							
D3	3	4	ø4							
		]][	ø3&ø8							

### COORDINATION DATA (ALL ENTRIES IN SECONDS)

	PLAN 1	PLAN 2	PLAN 3
CYCLE LENGTH	100 SEC	100 SEC	90 SEC
OFFSET	0	0	0
SPLIT Ø1&Ø6	12 (12)	12 (12)	12 (12)
SPLIT Ø2&Ø6	40 (16)	42 (18)	40 (16)
SPLIT Ø7 PED	- (24)	- (24)	- (24)
SPLIT Ø3&Ø8	12 (12)	12 (12)	12 (12)
SPLIT Ø4&Ø8	36 (36)	34 (34)	26 (26)
COORDINATED PHASE	ø2&ø6	ø2&ø6	ø2&ø6

### NOTES: 1. \( \phi 2 & \phi 6 \) "CALL NOT ACTUATED" DURING COORDINATION.

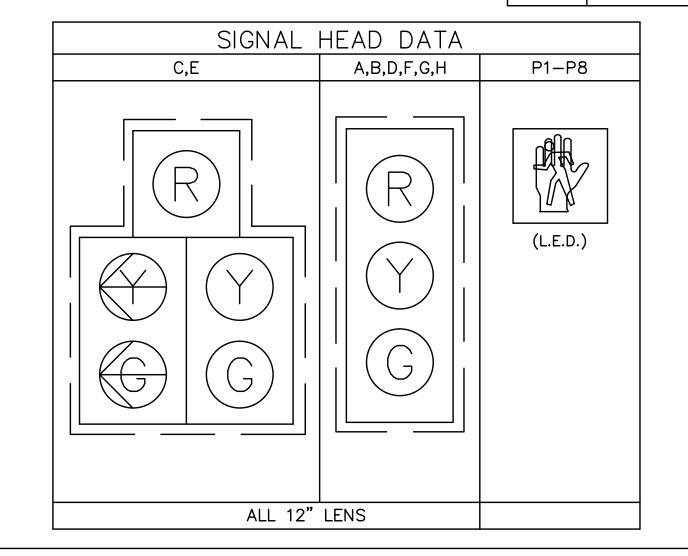
- 2. OFFSET: BEGINNING OF \$\psi 2\&\psi 6\$ GREEN.
- 3. FLOATING FORCE OFF SHALL BE IN EFFECT. 4. SPLIT TIMES EQUAL GREEN PLUS CLEARANCES.
- 5. () = SPLIT TIMES WITH PEDESTRIAN PHASE ACTUATED.
- 6. INHIBIT MAX TERMINATION SHALL BE IN EFFECT DURING COORDINATION.
- 7. PERMISSIVE MODE SHALL BE IN EFFECT.

ITEM 816.01 TRAFFIC SIGNAL RECONSTRUCTION PARADISE ROAD @ VINNIN STREET LIST OF MAJOR ITEMS REQUIRED

QUANTITY	DESCRIPTION
1	MODIFY EXISTING TS PEEK 3000E CONTROLLER & CABINET TO PROPOSED TIMINGS SHOWN
1	AUDIBLE PEDESTRIAN DEVICE
8	12" CIRCULAR YELLOW L.E.D. MODULES (ALL)
2	12" YELLOW LEFT ARROW L.E.D. MODULES (C,E)
1	3-SECTION SIGNAL HEAD ASTROBRAC
2	PEDESTRIAN SIGNAL HEAD VISOR (1-SECTION)

PLUS NECESSARY DUCT, CABLE, LABOR, MISCELLANEOUS MATERIAL AND EQUIPMENT TO COMPLETE THE INSTALLATION AND PROVIDE AN OPERATING TRAFFIC CONTROL SIGNAL.

MODIFY EXIST TS PEEK M3000E SYSTEM MASTER CONTROLLER AND MODEM TO PROPOSED TIMINGS SHOWN (INCLUDED UNDER ITEM 815.923)

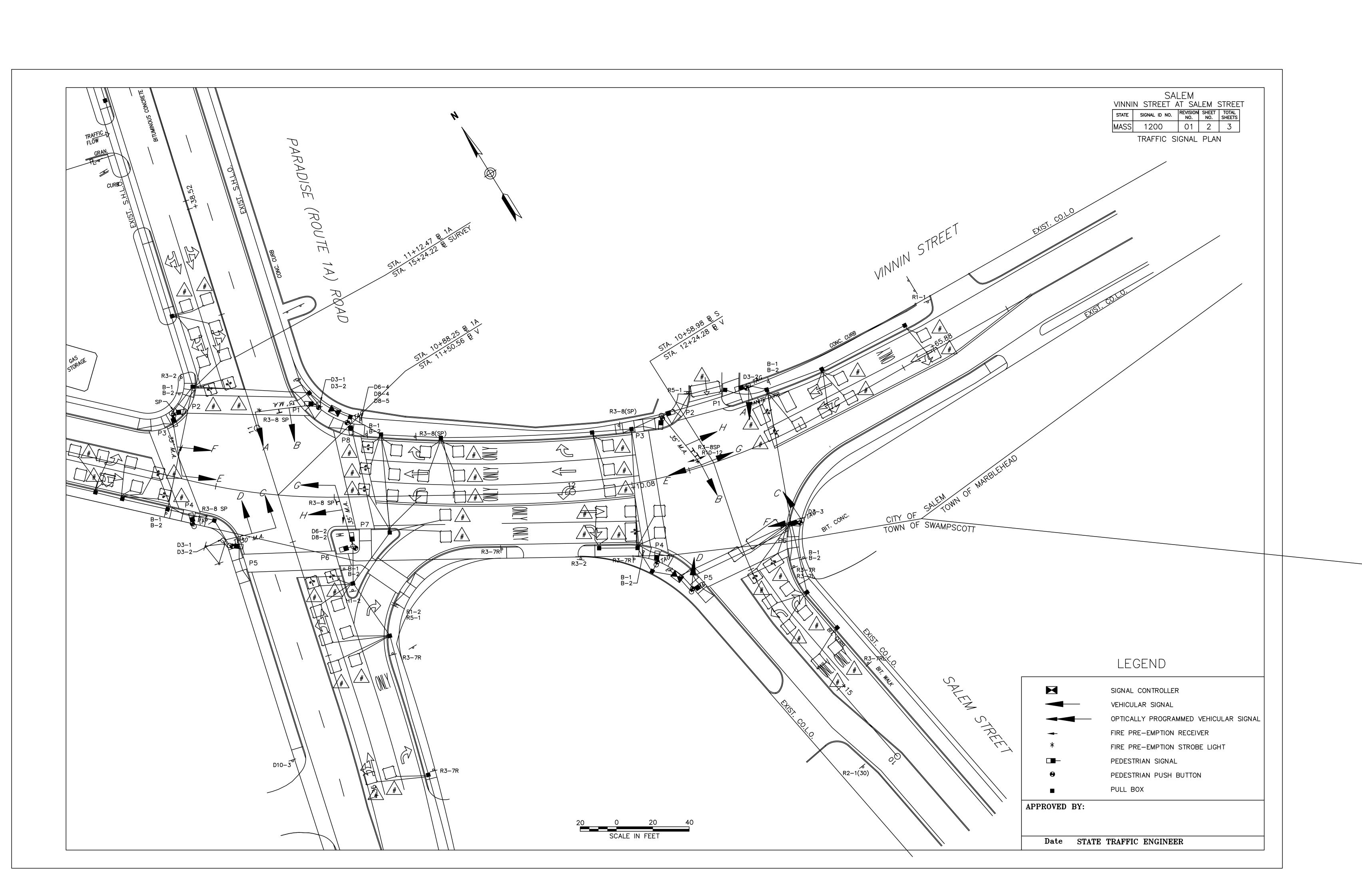


CONTROLLER MAKE & MODEL:	PEEK 3000E
UTILITY POLE No.	MECO 3872
METER No.	05073193
EMERGENCY PRE-EMPTION (TYP	E): OPTICOM

APPROVED BY:

STATE TRAFFIC ENGINEER

Date



									SEO	LIENICE	- AND	TIMIN	IC											
ADDDOACH	DIRECTION	HOUSING	T 1	l 2	3		5	₆	3EQ   7	B S	1			10	17	1 1	15	16	17	18	19	20	21	T. A.C.IIINIO
APPROACH	DIRECTION	HUUSING	6	2	3	10	3	6	/	0	9	10	11	12	13	14	15	16	17	10	19	20	21	FLASHING OPERATION
MINIMUM INTERVAL			1			1						6			10			6						
VEHICLE EXTENSION	1		1 1			2						0.5			2			05						
MAXIMUM 1			15			35						25			35			25						
MAXIMUM 2			16			45						30			45			30						
YELLOW CLEARANCE	1			4			4			3			4			4			4					
RED CLEARANCE					1			1			1			1			1			1				
PEDESTRIAN INTERVAL									7	17														
VINNIN ST	EB	 E,F	R	R	R	G	Y	R	R	R	R	R	 R	R	R	R	R	R	R	R				FY
VINNIN ST	WB	G	1,	(-Y− _R	R	R	R	R	R	R	R	R	R	R	G	Y	R	R	R	R				FY
VINNIN ST	WB		R	R	R	R	R	R	R	R	R	R	R	R	G	Y	R	R	R	R				FY
SALEM ST	NB	A,B	R	R	R	R	R	R	R	R	R	G	Y	R	R	R	R	R	R	R				FR
STAPLES DR	SB	C,D	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	G	Υ	R				FR
PEDESTRIAN X-ING			DW	DW	DW	DW	DW	DW	w	FDW	DW	DW	DW	DW	DW	DW	DW	DW	DW	DW				OUT
FLULSTRIAN X-ING					D **			DW	**		DW		D **		DW	D **								001
DETECTOR			N	L ON-LC	CK	N	ON-LO	DCK	N(	DN-LC	CK	NO	N-LC	)CK	N(	L ON-LO	DCK	N(	ON-LO	DCK				
RECALL				OFF			SOFT			OFF		OFF			SOFT				OFF					-
<u>S:</u>				ø1			ø2			ø3*			ø4			ø6			ø7		ø!	5 & Ø	.8	_
<u>J.</u> JTOMATIC FLASHING OPE U.T.C.D. SECTION 4D.12.	?																	<b>→</b>		, ,	<u> </u>	<u> </u>	-	
UPON PEDESTRIAN PUS ERM = PERMISSIVE 4 & Ø7 DUAL ENTRY	ACTUATION			7	$  \Rightarrow$				<b>←</b> — ¬						Р	PERM				N	OT US	SED		
	MUM 1 = ALL OTHER TIMES MUM 2 = $11:00AM - 7:00PM$ , SUN-SA						10	<b>&gt;</b>		1	<u> </u>		1			1/	<b>&gt;</b>		10	<i>&gt;</i>				
TOP AND GO OPERATION			DAY.																					

PREFERENTIAL PHASE SEQUENCE

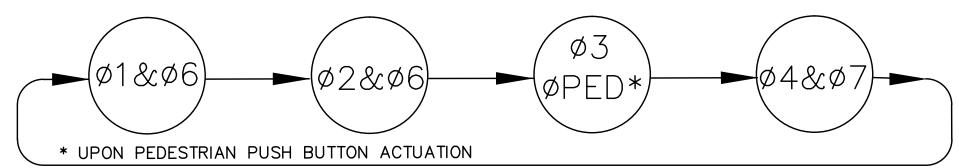
FLASHING OPERATION FOR EMERGENCY ONLY.

8. DURING PEDESTRIAN INTERVAL, FDW THROUGH

9. INHIBIT MAX TERMINATION SHALL BE IN EFFECT

YELLOW OPERATION SHALL BE IN EFFECT.

DURING COORDINATION.



ITEM 816.02 TRAFFIC SIGNAL RECONSTRUCTION VINNIN STREET @ SALEM STREET

	LIST OF MAJOR ITEMS REQUIRED
QUANTITY	DESCRIPTION
1	MODIFY EXISTING TS PEEK 3000E CONTROLLER & CABINET TO PROPOSED TIMINGS SHOWN
1	PEDESTRIAN PUSH BUTTON W/R10-3f AND SIGN SADDLE
1	AUDIBLE PEDESTRIAN DEVICE
8	12" CIRCULAR YELLOW L.E.D. MODULES (ALL)
1	12" YELLOW LEFT ARROW L.E.D. MODULES (G)

PLUS NECESSARY DUCT, CABLE, LABOR, MISCELLANEOUS MATERIAL AND EQUIPMENT TO COMPLETE THE INSTALLATION AND PROVIDE AN OPERATING TRAFFIC CONTROL SIGNAL.

### DETECTOR SCHEDULE

	DE	TECTOR		AMPL	JFIER	PHASE	PHASE	DELAY/		LC	OOPS		DETECTION
NO.	STREET	DIRECTION	LANE	CHANNEL	SETTING	CALLED	EXTENDED	EXTENSION	SIZE (FT)	SEGMENTS	TURNS	CONNECTIONS	MODE
1	SALEM STREET	NB	LEFT	1	PRESENCE	4	4		6 X 6	3	3	SERIES	PRESENCE
2	SALEM STREET	NB	RIGHT	2	PRESENCE	4	4		*	3	3	SERIES	PRESENCE
3	SALEM STREET	NB	LEFT	3	PRESENCE	4	_		6 X 6	1	3	_	SYSTEM
4	SALEM STREET	NB	RIGHT	4	PRESENCE	4	_		6 X 6	1	3	_	SYSTEM
5	VINNIN STREET	WB	LEFT/THROUGH	5	PRESENCE	1&6	1&6		6 X 6	3	3	SERIES	PRESENCE
6	VINNIN STREET	WB	THROUGH	6	PRESENCE	6	6		**	3	3	SERIES	PRESENCE
7	VINNIN STREET	WB	LEFT/THROUGH	7	PRESENCE	_	_		6 X 6	1	3	_	SYSTEM
8	VINNIN STREET	WB	THROUGH	8	PRESENCE	_	_		6 X 6	1	3	_	SYSTEM
9	VINNIN STREET	EB	THROUGH	9	PRESENCE	2	2		6 X 6	3	3	SERIES	PRESENCE
10	VINNIN STREET	EB	RIGHT	10	PRESENCE	2	2		6 X 6	2	3	SERIES	PRESENCE
11	VINNIN STREET	EB	THROUGH	11	PRESENCE	ı	_		6 X 6	1	3	_	SYSTEM
12	VINNIN STREET	EB	RIGHT	12	PRESENCE	-	_		6 X 6	1	3	_	SYSTEM
13	STAPLES EXIT	SB	ALL	13	PRESENCE	7	7		6 X 10	1	3	_	PRESENCE
B1	SALEM STREET	NB	LEFT	14	PRESENCE	4	4		6 X 6	1	4	_	BICYCLE
B2	SALEM STREET	NB	RIGHT	15	PRESENCE	4	4		6 X 12	1	4	_	BICYCLE
B5	VINNIN STREET	WB	LEFT/THROUGH	16	PRESENCE	1&6	1&6		6 X 6	1	4	_	BICYCLE
В6	VINNIN STREET	WB	THROUGH	17	PRESENCE	6	6		6 X 12	1	4	_	BICYCLE
B10	VINNIN STREET	EB	RIGHT	18	PRESENCE	2	2		6 X 9	1	4	_	BICYCLE

**SEQUENCE & TIMING NOTES:** 

- 1. IF THE ASSIGNED RIGHT OF WAY FOR ANY TRAFFIC MOVEMENT IS TO REMAIN IN EFFECT DURING THE NEXT CALLED PHASE, THE SIGNAL INDICATIONS FOR THAT TRAFFIC MOVEMENT WILL NOT CHANGE DURING THE CLEARANCE INTERVAL.
- 2. THE RIGHT OF WAY MAY BE ASSIGNED TO ANY PHASE OR ANY COMBINATION OF NON-CONFLICTING PHASES.
- 3. IF CALLS EXIST ON ALL PHASES, THE ASSIGNMENT OF RIGHT OF WAY SHALL BE IN ACCORDANCE WITH THE PREFERENTIAL PHASE SEQUENCE.
- 4. IF THE ASSIGNED RIGHT-OF-WAY FOR ANY TRAFFIC MOVEMENT IS TO CHANGE DURING THE NEXT CALLED PHASE, THE SIGNAL INDICATION FOR THAT MOVEMENT WILL DISPLAY THE APPROPRIATE CLEARANCE INTERVALS.

VINNIN STREET AT SALEM STREET STATE SIGNAL ID NO. REVISION SHEET TOTAL NO. NO. SHEETS MASS 1200 01 3 3

TRAFFIC SIGNAL DATA

P	PRE-E HASING 8	MPTION & PRIOR	ITY
DETECTOR & PRIORITY	PRE-EMPT PHASE ASSIGNMENT	MOVEMENT	VEHICLE PHASE ASSIGNMENT
D1	1	77	ø4
D2	2	<u> </u>	ø2
D3	3	<b> </b>	ø1&ø6

### DAILY & WEEKLY COORDINATION PROGRAM

	MONDAY THRU FRIDAY	SATURDAY	SUNDAY
PLAN 1 100" CYCLE	0700–1100	ı	_
PLAN 2 100" CYCLE	1100–1900	-	_
PLAN 3 90" CYCLE		1000-1800	_
FREE OPERATION	0000-0700 1900-2400	0000-1000 1800-2400	0000-2400
FLASH OPERATION	_	_	_

COORDINATION DATA

(ALL ENTRIES IN SECONDS)

COORDINATED PHASE | ø2&ø6 | ø2&ø6 | ø2&ø6

5. ( ) = SPLIT TIMES WITH PEDESTRIAN PHASE ACTUATED.

NOTES: 1. \( \phi 2 \& \phi 6 \) "CALL NOT ACTUATED" DURING COORDINATION.

3. FLOATING FORCE OFF SHALL BE IN EFFECT.

4. SPLIT TIMES EQUAL GREEN PLUS CLEARANCES.

6. INHIBIT MAX TERMINATION SHALL BE IN EFFECT

2. OFFSET: BEGINNING OF \$2\&\$6\$ GREEN.

7. PERMISSIVE MODE SHALL BE IN EFFECT.

CYCLE LENGTH

SPLIT Ø1&Ø6

SPLIT Ø2&Ø6

SPLIT Ø3 PED

SPLIT Ø4&Ø7

DURING COORDINATION.

PLAN 1 | PLAN 2 | PLAN 3

49

- (26)

100 SEC | 100 SEC | 90 SEC

12 (12) | 12 (12) | 12 (12)

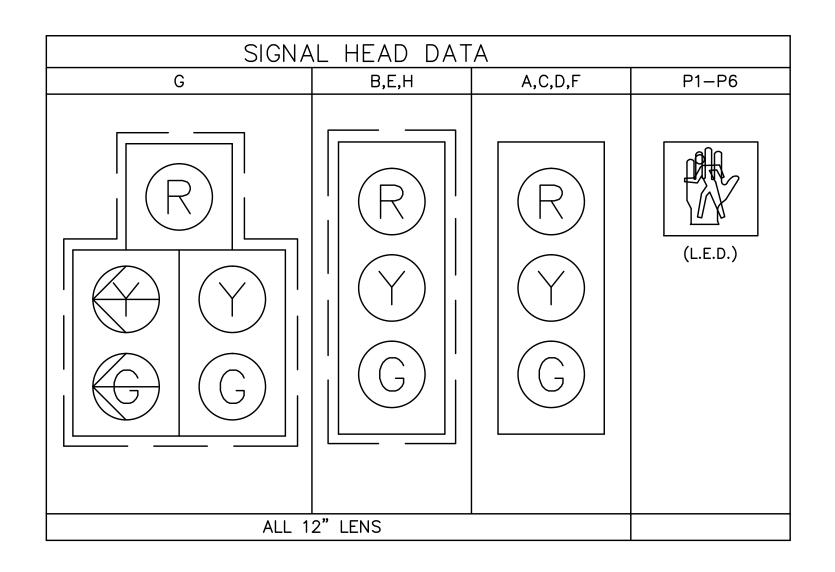
62 (36) | 64 (38) | 59 (33)

26 (26) | 24 (24) | 19 (19)

- (26) - (26)

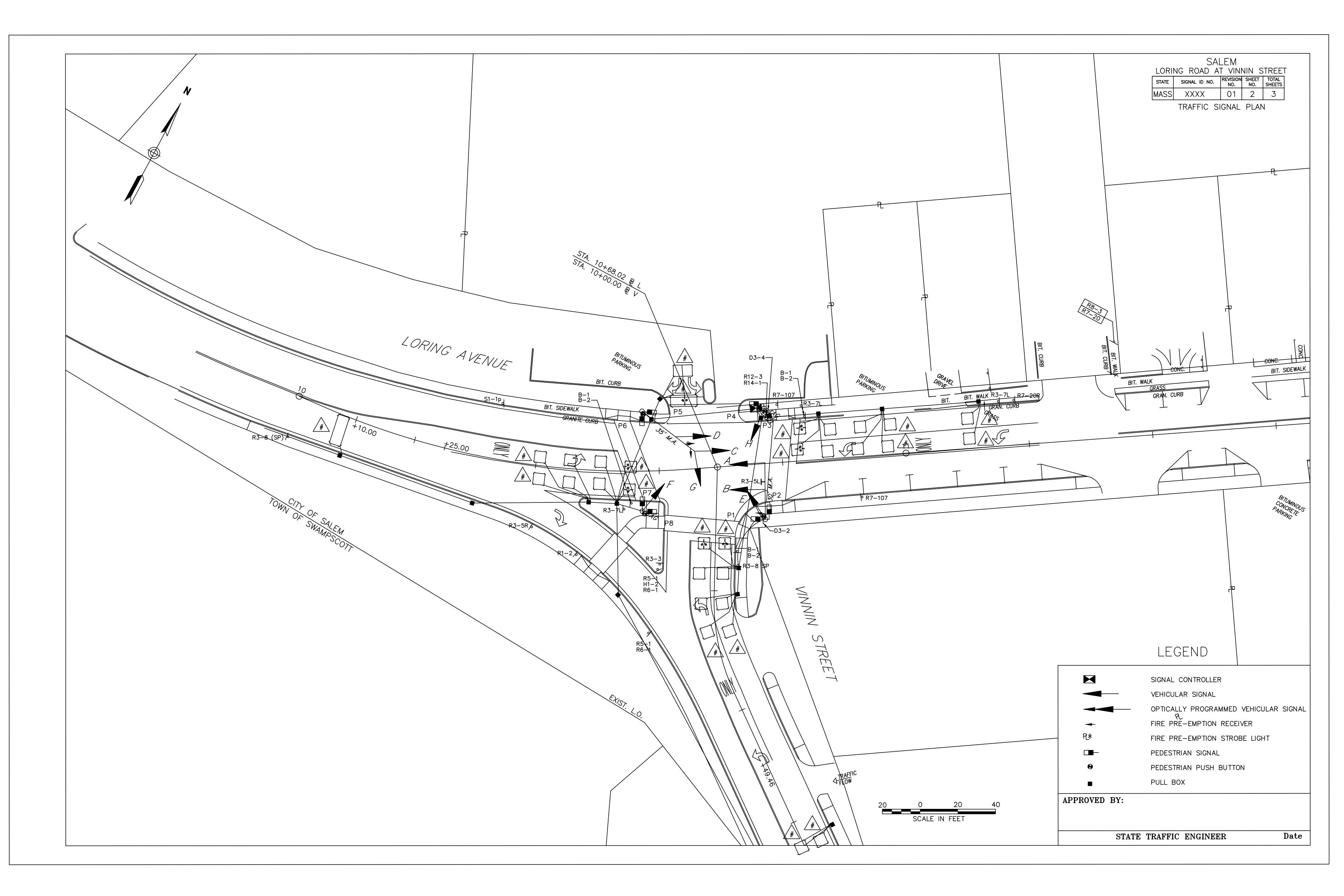
#### EMERGENCY VEHICLE PRE-EMPTION OPERATION.

- 1. EMERGENCY VEHICLE PRE-EMPTION SIGNALS SHALL BE OPTICALLY TRANSMITTED BY OPTICAL EMITTERS MOUNTED IN EMERGENCY VEHICLES AND RECEIVED BY OPTICAL DETECTORS LOCATED AT EACH INTERSECTION.
- 2. PRE-EMPTION SIGNALS SHALL BE SERVICED ON A FIRST COME, FIRST SERVE BASIS.
- 3. IN RESPONSE TO A PRE-EMPTION SIGNAL RECEIVED AT AN INTERSECTION BY OPTICAL DETECTOR D1 (OR D2, D3) THE CONTROLLER SHALL HOLD OR ADVANCE TO AND HOLD IN EMERGENCY VEHICLE PRE-EMPTION PHASE #1 (OR #2, #3) GREEN FOR A MINIMUM OF TEN (10) SECONDS OR ÜNTİL PRE-EMPTION SIGNAL CEASES. THE CONTROLLER SHALL THEN TIME PRE-EMPTION PHASE CLEARANCES FOR THE ASSOCIATED PHASE(S) AS SHOWN IN THE SEQUENCE AND TIMING CHART AND SERVICE SUBSEQUENT EMERGENCY VEHICLE PRE-EMPTION PHASES AS NECESSARY.
- 4. UNLESS OTHERWISE STATED, ONCE A PRE-EMPTION CALL HAS BEEN RECEIVED BY THE TRAFFIC SIGNAL CONTROLLER AND THE PRE-EMPTION PHASE IS BEING SERVICED, IT SHALL REMAIN IN THAT PHASE AS LONG AS THE CALL IS PRESENT.
- 5. MINIMUM GREEN AND NORMAL VEHICLE CLEARANCE SHALL BE PROVIDED ON PHASES THAT ARE TO BE TERMINATED BY PRE-EMPTION DEMAND.
- 6. PRE-EMPTION STROBE SHALL BE ILLUMINATED WHENEVER ANY
- EMERGENCY VEHICLE PRE-EMPTION GREEN IS ON. 7. EMERGENCY VEHICLE PRE-EMPTION SHALL OVERRIDE COORDINATION.



CONTROLLER MAKE & MODELS		
UTILITY POLE No. METER No.	NET&T 029, MECO 65 021 878 487	
EMERGENCY PRE-EMPTION (T	YPE): OPTICOM	
APPROVED BY:		
STATE TRAFFIC	ENGINEER	Date

*  $2 - 6 \times 6$ ,  $1 - 6 \times 20$  **  $1 - 6 \times 6$ ,  $1 - 6 \times 10$ 



			1						UENC	E AND	IIMII	1			<del></del>						1		1	<u> </u>
APPROACH	DIRECTION	HOUSING	<u> </u>	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	FLASHINO OPERATIO
MINIMUM INTERVAL			6			10						8			10			8						OPERATIO
VEHICLE EXTENSION			1			2						2			2			2						
MAXIMUM 1			10			30						30			30			30						
MAXIMUM 2			10			40						35			40			35						
YELLOW CLEARANCE				4			4			3			4			4			4					
RED CLEARANCE					1			1			1			1			1			1				
PEDESTRIAN INTERVAL									7	12														
LORING AVE	NB	A,B	R	R	R	G	Y	R	R	R	R	R	R	R	R	R	R	R	R	R				FY
LORING AVE	SB	С	(-G-R	(Y-R)	R	R	R	R	R	R	R	R	R	R	G	Υ	R	R	R	R				FY
LORING AVE	SB	D	R	R	R	R	R	R	R	R	R	R	R	R	G	Υ	R	R	R	R				FY
VINNIN ST	WB	G,H	R	R	R	R	R	R	R	R	R	G	Υ	R	R	R	R	R	R	R				FR
DRIVEWAY	EB	E,F	R	R	R	R	R	R	R	R	R	R	R	R	R	R	R	G	Υ	R				FR
PEDESTRIAN X-ING	ALL	ALL	DW	DW	DW	DW	DW	DW	W	FDW	DW	DW	DW	DW	DW	DW	DW	DW	DW	DW				OUT
DETECTOR			NO	L DN-LC	CK	N	L DN-LC	I DCK	NO	L DN-LC	CK	NC	N-LC	CK	NC	DN-LC	)CK	NC	N—LC	l CK				
RECALL				OFF			OFF			OFF			SOFT			OFF			OFF					
• •				ø1			ø2			ø3*			ø4			ø6			ø7		Ø	5 & 9	#8	
TOMATIC FLASHING OPE I.T.C.D. SECTION 4D.12. JPON PEDESTRIAN PUSI RM = PERMISSIVE				1		<b>*</b>	1	1	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	 	1	\[\]	1	<u> </u>	<b>1</b>	] PI	<b>₹</b> ERM	· ነ[	<b>*</b>	<u> </u>		IOT US		
	ELD JM 1 = ALL OTHER TIMES JM 2 = 11:00AM - 7:00PM, SUN-SAT					Ŷ	1		Y	<b>←</b> – →		Ŷ	71	•	Ŷ	1]		Ť	1					

PREFERENTIAL PHASE SEQUENCE

* UPON PEDESTRIAN PUSH BUTTON ACTUATION

ITEM 816.04 TRAFFIC SIGNAL RECONSTRUCTION

8. STOP AND GO OPERATION FOR 24 HOURS PER DAY.

FLASHING OPERATION FOR EMERGENCY ONLY.

9. DURING PEDESTRIAN INTERVAL, FDW THROUGH YELLOW OPERATION SHALL BE IN EFFECT.

10. INHIBIT MAX TERMINATION SHALL BE IN EFFECT

DURING COORDINATION.

	VINNIN STREET @ LORING AVENUE LIST OF MAJOR ITEMS REQUIRED
QUANTITY	DESCRIPTION
1	MODIFY EXIST TS PEEK 3000E CONTROLLER & CABINET TO PROPOSED SEQUENCE & TIMINGS SHOWN
2	PEDESTRIAN PUSH BUTTON W/R10-3f AND SIGN SADDLE
1	DISCONNECT SWITCH
8	12" CIRCULAR YELLOW L.E.D. MODULES (ALL)
1	12" YELLOW LEFT ARROW L.E.D. MODULES (C)
3	SIGNAL HEAD ASTROBRAC

PLUS NECESSARY DUCT, CABLE, LABOR, MISCELLANEOUS MATERIAL AND EQUIPMENT TO COMPLETE THE INSTALLATION AND PROVIDE AN OPERATING TRAFFIC CONTROL SIGNAL.

COORDINATED PHASE | ø4&ø7 | ø4&ø7 | ø4&ø7 NOTES: 1. Ø4&Ø7 "CALL NOT ACTUATED" DURING COORDINATION. 2. OFFSET: BEGINNING OF Ø4&Ø7 GREEN. 3. FLOATING FORCE OFF SHALL BE IN EFFECT.

CYCLE LENGTH

SPLIT Ø1&Ø6

SPLIT Ø2&Ø6

SPLIT Ø4&Ø7

SPLIT Ø3 PED

OFFSET

4. SPLIT TIMES EQUAL GREEN PLUS CLEARANCES. 5. ( ) = SPLIT TIMES WITH PEDESTRIAN PHASE ACTUATED.

6. INHIBIT MAX TERMINATION SHALL BE IN EFFECT DURING COORDINATION.

COORDINATION DATA

(ALL ENTRIES IN SECONDS)

45

PLAN 1 PLAN 2 PLAN 3

100 SEC | 100 SEC | 90 SEC

46

12 (12) | 13 (13) | 12 (12)

33 (33) | 30 (30) | 25 (25)

55 (34) 57 (36) 53 (32)

- (21) | - (21) | - (21)

41

DETECTOR SCHEDULE 7 PERMISSIVE MODE SHALL BE IN FEFECT

				L		'N 301			/	Т.			
	DET	ECTOR		AMP	LIFIER	PHASE	PHASE	DELAY/		L	00PS		DETECTION
NO.	STREET	DIRECTION	LANE	CHANNEL	SETTING	CALLED	EXTENDED	EXTENSION	SIZE (FT)	SEGMENTS	TURNS	CONNECTIONS	MODE
1	LORING AVENUE	NB	LEFT	1	PRESENCE	2	2		6 X 6	3	3	S/P	PRESENCE
2	LORING AVENUE	NB	ALL	2	PRESENCE	ı	_		6 X 16	1	3	_	SYSTEM
3	LORING AVENUE	NB	THROUGH	3	PRESENCE	2	2		6 X 6	3	3	S/P	PRESENCE
4	LORING AVENUE	SB	LEFT	4	PRESENCE	1&6	1&6		6 X 6	3	3	S/P	PRESENCE
5	LORING AVENUE	SB	LEFT	5	PRESENCE	1	_		6 X 6	1	3	_	SYSTEM
6	LORING AVENUE	SB	RIGHT/THROUGH	6	PRESENCE	1	_		6 X 6	1	3	_	SYSTEM
7	LORING AVENUE	SB	RIGHT/THROUGH	7	PRESENCE	6	6		6 X 6	3	3	S/P	PRESENCE
8	VINNIN STREET	WB	LEFT	8	PRESENCE	4	4		6 X 6	3	3	SERIES	PRESENCE
9	VINNIN STREET	WB	RIGHT/THROUGH	9	PRESENCE	4	4		6 X 6	3	3	SERIES	PRESENCE
10	VINNIN STREET	WB	LEFT	10	PRESENCE	1	_		6 X 6	1	3	_	SYSTEM
11	VINNIN STREET	WB	RIGHT/THROUGH	11	PRESENCE	ı	_		6 X 6	1	3	_	SYSTEM
12	SITE DRIVE	EB	ALL	12	PRESENCE	7	7		6 X 10	2	3	SERIES	PRESENCE
B1	LORING AVENUE	NB	LEFT	13	PRESENCE	2	2		6 X 6	1	4	_	BICYCLE
В3	LORING AVENUE	NB	THROUGH	14	PRESENCE	2	2		6 X 6	1	4	_	BICYCLE
В4	LORING AVENUE	SB	LEFT	15	PRESENCE	1&6	1&6		6 X 6	1	4	_	BICYCLE
В7	LORING AVENUE	SB	RIGHT/THROUGH	16	PRESENCE	6	6		6 X 6	1	4	_	BICYCLE
В8	VINNIN STREET	WB	LEFT	17	PRESENCE	4	4		6 X 6	1	4	_	BICYCLE
В9	VINNIN STREET	WB	RIGHT/THROUGH	18	PRESENCE	4	4		6 X 6	1	4	_	BICYCLE
B12	SITE DRIVE	EB	ALL	19	PRESENCE	7	7		6 X 10	1	4	_	BICYCLE

1.	IF THE ASSIGNED RIGHT OF WAY FOR ANY TRAFFIC
	MOVEMENT IS TO REMAIN IN EFFECT DURING THE
	NEXT CALLED PHASE, THE SIGNAL INDICATIONS FOR
	THAT TRAFFIC MOVEMENT WILL NOT CHANGE DURING
	THE CLEARANCE INTERVAL.

- 2. THE RIGHT OF WAY MAY BE ASSIGNED TO ANY PHASE OR ANY COMBINATION OF NON-CONFLICTING PHASES.
- 3. IF CALLS EXIST ON ALL PHASES, THE ASSIGNMENT OF RIGHT OF WAY SHALL BE IN ACCORDANCE WITH THE PREFERENTIAL PHASE SEQUENCE.
- 4. IF THE ASSIGNED RIGHT-OF-WAY FOR ANY TRAFFIC MOVEMENT IS TO CHANGE DURING THE NEXT CALLED PHASE, THE SIGNAL INDICATION FOR THAT MOVEMENT WILL DISPLAY THE APPROPRIATE CLEARANCE INTERVALS.

#### DAILY & WEEKLY COORDINATION PROGRAM

	MONDAY THRU FRIDAY	SATURDAY	SUNDAY
PLAN 1 100" CYCLE	0700-1100	_	_
PLAN 2 100" CYCLE	1100–1900	-	_
PLAN 3 90" CYCLE	_	1000-1800	_
FREE OPERATION	0000-0700 1900-2400	0000-1000 1800-2400	0000-2400
FLASH OPERATION	_	_	_

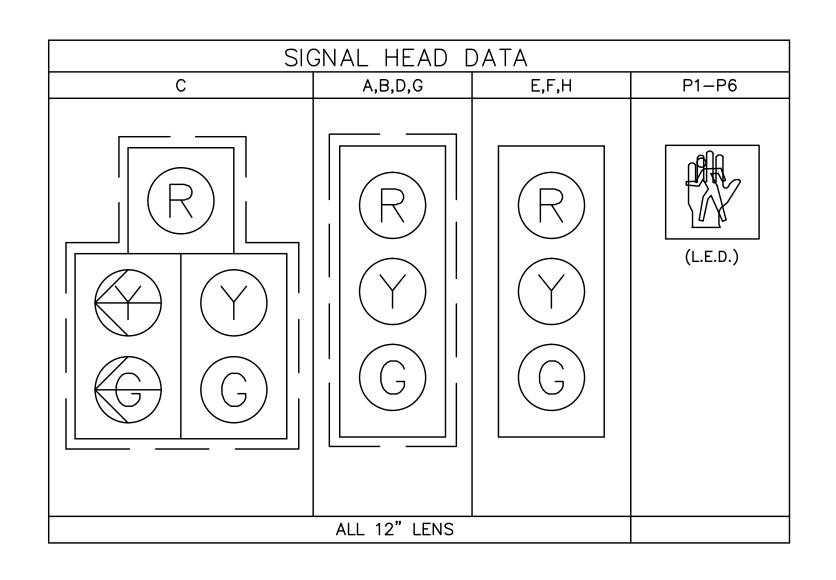
		_		EM		
LORIN	IG F	ROAD	ΑT	VINN	IN S	REET
STATE	SIG	NAL ID N	١٥.	REVISION NO.	SHEET NO.	TOTAL SHEETS
MASS		XXXX		01	3	

TRAFFIC SIGNAL DATA

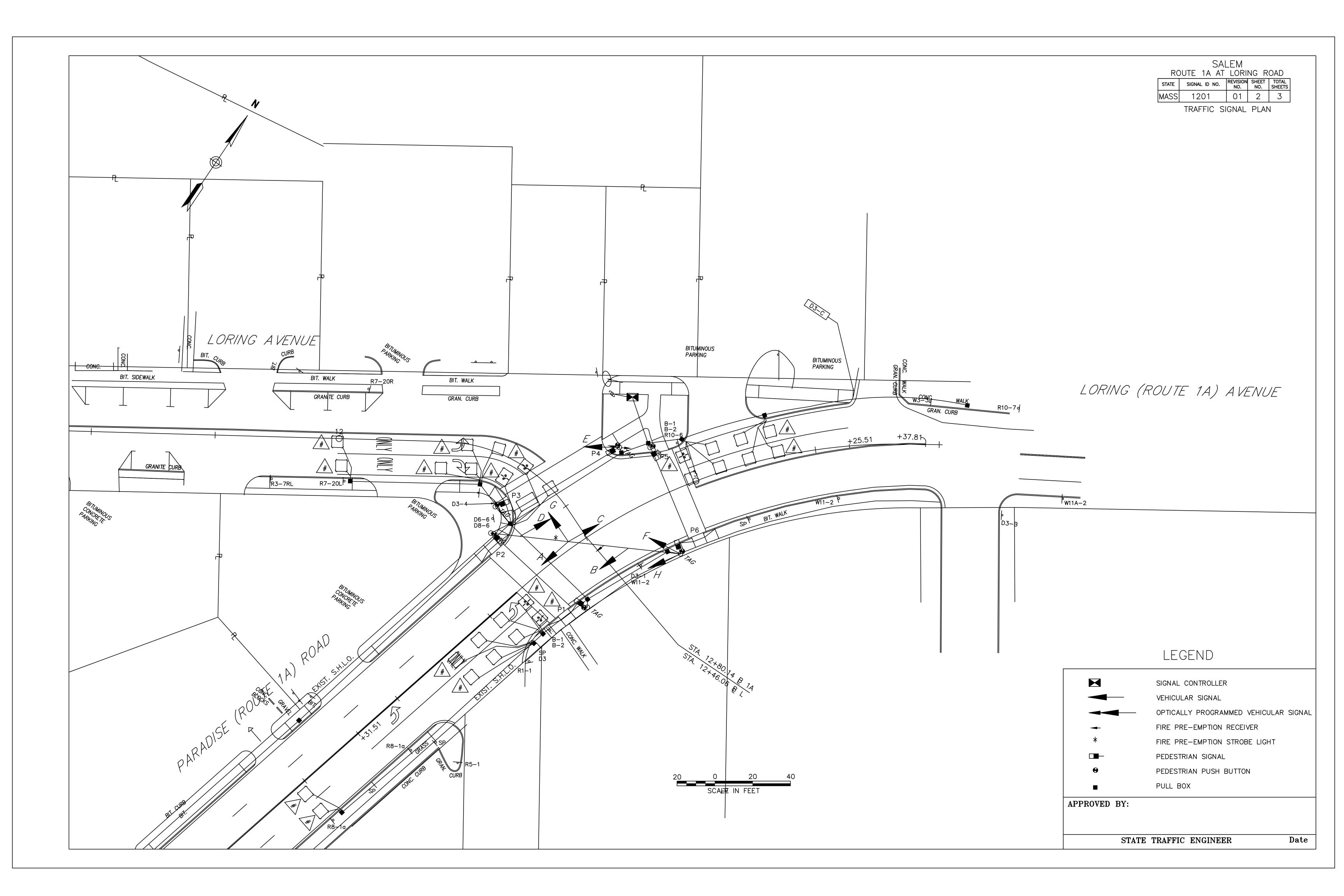
Р	PRE-E HASING 8	MPTION & PRIORI	ITY
DETECTOR & PRIORITY	PRE-EMPT PHASE ASSIGNMENT	MOVEMENT	VEHICLE PHASE ASSIGNMENT
D1	1	11	ø2
D2	2	JI.	ø1&ø6
D3	3	71	ø4

#### EMERGENCY VEHICLE PRE-EMPTION OPERATION.

- 1. EMERGENCY VEHICLE PRE-EMPTION SIGNALS SHALL BE OPTICALLY TRANSMITTED BY OPTICAL EMITTERS MOUNTED IN EMERGENCY VEHICLES AND RECEIVED BY OPTICAL DETECTORS LOCATED AT EACH INTERSECTION.
- 2. PRE-EMPTION SIGNALS SHALL BE SERVICED ON A FIRST COME, FIRST SERVE BASIS.
- 3. IN RESPONSE TO A PRE-EMPTION SIGNAL RECEIVED AT AN INTERSECTION BY OPTICAL DETECTOR D1 (OR D2, D3) THE CONTROLLER SHALL HOLD OR ADVANCE TO AND HOLD IN EMERGENCY VEHICLE PRE-EMPTION PHASE #1 (OR #2, #3) GREEN FOR A MINIMUM OF TEN (10) SECONDS OR UNTIL PRE-EMPTION SIGNAL CEASES. THE CONTROLLER SHALL THEN TIME PRE-EMPTION PHASE CLEARANCES FOR THE ASSOCIATED PHASE(S) AS SHOWN IN THE SEQUENCE AND TIMING CHART AND SERVICE SUBSEQUENT EMERGENCY VEHICLE PRE-EMPTION PHASES AS NECESSARY.
- 4. UNLESS OTHERWISE STATED, ONCE A PRE-EMPTION CALL HAS BEEN RECEIVED BY THE TRAFFIC SIGNAL CONTROLLER AND THE PRE-EMPTION PHASE IS BEING SERVICED, IT SHALL REMAIN IN THAT PHASE AS LONG AS THE CALL IS PRESENT.
- 5. MINIMUM GREEN AND NORMAL VEHICLE CLEARANCE SHALL BE PROVIDED ON PHASES THAT ARE TO BE TERMINATED BY PRE-EMPTION DEMAND.
- 6. PRE-EMPTION STROBE SHALL BE ILLUMINATED WHENEVER ANY
- EMERGENCY VEHICLE PRE-EMPTION GREEN IS ON.
- 7. EMERGENCY VEHICLE PRE-EMPTION SHALL OVERRIDE COORDINATION.



CONTROLLER MAKE & MODEL:	PEEK 3000E	
UTILITY POLE No.	NYNEX 1/92, MECO 264	6
METER No.	12 261 930	
EMERGENCY PRE-EMPTION (TYPE	E): OPTICOM	
APPROVED BY:		
STATE TRAFFIC EN	IGINEER	Date



							SE	EQUEN	CE AN	ND TIM	IING										
APPROACH	DIRECTION	HOUSING	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	FLASHING
MINIMUM INTERVAL			6			10						8			10						OPERATIO
VEHICLE EXTENSION			1			2						3			2						
MAXIMUM 1			10			30						25			30						
MAXIMUM 2			10			30						25			30						
YELLOW CLEARANCE				4			4			3			4			4					
RED CLEARANCE					1			1			1			1			1				
PEDESTRIAN INTERVAL									7	15											
PARADISE RD	NB	A	<b>←</b> G-	<del>(</del> Y-	<del> </del> R-	<del></del>	<del>                                     </del>	<del>-</del> R−	<del>-</del> R-	<del>                                     </del>	<del>-</del> R−	<del> </del> R-	<del>                                      </del>	<u></u> -R−	<del> </del> -R-	<del> </del>	<del> </del> R-				←FR—
PARADISE RD	NB	В,Н	R	R	R	R	R	R	R	R	R	R	R	R	G	Y	R				FY
LORING AVE	SB	C,D	R	R	R	G	Υ	R	R	R	R	R	R	R	R	R	R				FY
LORING AVE	EB	E,F,G	R	R	R	R	R	R	R	R	R	G	Υ	R	R	R	R				FR
PEDESTRIAN X-ING	ALL	ALL	DW	DW	DW	DW	DW	DW	W	FDW	DW	DW	DW	DW	DW	DW	DW				OUT
DETECTOR			N	ON-LC	CK	N	ON-LO	OCK	NON-LOCK			NON-LOCK			NON-LOCK					•	
RECALL				OFF			SOFT	-		OFF			OFF			SOFT	-				
<u> </u>				ø1			ø2			ø3*			ø4			ø6		ø5	,ø7 &	ø8	]
TOMATIC FLASHING OPE I.T.C.D. SECTION 4D.12. JPON PEDESTRIAN PUSI							<b>4</b>				<b>•</b>										
XIMUM 1 = ALL OTHER XIMUM 2 = 11:00AM - DP AND GO OPERATION ASHING OPERATION FOR	TIMES 7:00PM, S FOR 24 H	SUN-SAT OURS PER	$  \preceq  $	•	-		<b>١</b> ]	Ţ	<	- √ - √]	•	= 7	(۲	-		11	•	N	IOT US	SED	

SEQUENCE & TIMING NOTES:

- 1. IF THE ASSIGNED RIGHT OF WAY FOR ANY TRAFFIC MOVEMENT IS TO REMAIN IN EFFECT DURING THE NEXT CALLED PHASE, THE SIGNAL INDICATIONS FOR THAT TRAFFIC MOVEMENT WILL NOT CHANGE DURING THE CLEARANCE INTERVAL.
- 2. THE RIGHT OF WAY MAY BE ASSIGNED TO ANY PHASE OR ANY COMBINATION OF NON-CONFLICTING PHASES.
- 3. IF CALLS EXIST ON ALL PHASES, THE ASSIGNMENT OF RIGHT OF WAY SHALL BE IN ACCORDANCE WITH THE PREFERENTIAL PHASE SEQUENCE.
- 4. IF THE ASSIGNED RIGHT-OF-WAY FOR ANY TRAFFIC MOVEMENT IS TO CHANGE DURING THE NEXT CALLED PHASE, THE SIGNAL INDICATION FOR THAT MOVEMENT WILL DISPLAY THE APPROPRIATE CLEARANCE INTERVALS.

ROUTE 1A AT LORING ROAD  STATE SIGNAL ID NO. REVISION SHEET TOTAL NO. NO. SHEETS  MASS 1201 01 3	SALEM												
STATE SIGNAL ID NO. NO. NO. SHEETS	RO	UTE	1A	ΑT	LORIN	IG RC	)AD						
MACC 1201 01 3	STATE	SIGN	NAL ID	NO.	REVISION NO.		TOTAL SHEETS						
MASS 1201   01   3	MASS	1	201	1	01	3							

TRAFFIC SIGNAL DATA

Р	PRE-EMPTION PHASING & PRIORITY														
DETECTOR & PRIORITY	PRE-EMPT PHASE ASSIGNMENT	MOVEMENT	VEHICLE PHASE ASSIGNMENT												
D1	1	71	ø1&ø6												
D2	2	4	ø2												
D3	3	~	ø4												

### DAILY & WEEKLY COORDINATION PROGRAM

	MONDAY THRU FRIDAY	SATURDAY	SUNDAY
PLAN 1 100" CYCLE	0700–1100	_	_
PLAN 2 100" CYCLE	1100–1900	-	_
PLAN 3 90" CYCLE	I	1000-1800	_
FREE OPERATION	0000-0700 1900-2400	0000-1000 1800-2400	0000-2400
FLASH OPERATION	_	_	_

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- 5. MINIMUM GREEN AND NORMAL VEHICLE CLEARANCE SHALL BE PROVIDED ON PHASES THAT ARE TO BE TERMINATED BY PRE-EMPTION DEMAND.
- 6. PRE-EMPTION STROBE SHALL BE ILLUMINATED WHENEVER ANY EMERGENCY VEHICLE PRE-EMPTION GREEN IS ON.
- 7. EMERGENCY VEHICLE PRE-EMPTION SHALL OVERRIDE COORDINATION.

## <u>COORDINATION DATA</u> (ALL ENTRIES IN SECONDS)

PLAN 1 PLAN 2 PLAN 3

CYCLE LENGTH 100 SEC 100 SEC 90 SEC

OFFSET 93 92 83

SPLIT Ø1&Ø6 16 (16) 14 (14) 13 (13)

SPLIT Ø2&Ø6 55 (31) 57 (33) 54 (30)

SPLIT Ø3 PED - (24) - (24) - (24)

SPLIT Ø4 29 (29) 29 (29) 23 (23)

COORDINATED PHASE Ø2&Ø6 Ø2&Ø6 Ø2&Ø6

- NOTES: 1. \( \phi 2 \& \phi 6 \) "CALL NOT ACTUATED" DURING COORDINATION. 2. OFFSET: BEGINNING OF \( \phi 2 \& \phi 6 \) GREEN.
  - 3. FLOATING FORCE OFF SHALL BE IN EFFECT.
  - 4. SPLIT TIMES EQUAL GREEN PLUS CLEARANCES.
  - 5. ( ) = SPLIT TIMES WITH PEDESTRIAN PHASE ACTUATED.
    6. INHIBIT MAX TERMINATION SHALL BE IN EFFECT
  - DURING COORDINATION.
  - 7. PERMISSIVE MODE SHALL BE IN EFFECT.

# DESCRIPTION MODIFY EXIST TS PEEK 3000E CONTROLLER & CABINET TO PROPOSED TIMINGS SHOWN

ITEM 816.03
TRAFFIC SIGNAL RECONSTRUCTION

PARADISE ROAD @ LORING AVENUE

LIST OF MAJOR ITEMS REQUIRED

PREFERENTIAL PHASE SEQUENCE

6. DURING PEDESTRIAN INTERVAL, FDW THROUGH

7. INHIBIT MAX TERMINATION SHALL BE IN EFFECT

* UPON PEDESTRIAN PUSH BUTTON ACTUATION

YELLOW OPERATION SHALL BE IN EFFECT.

IQUANTITY

DURING COORDINATION.

1 MODIFT EXIST IS FEEK SOUDE CONTROLLER & CABINET TO FROFOSED TIMINGS SHOWN

2 PEDESTRIAN PUSH BUTTON W/R10-3f AND SIGN SADDLE

7 12" CIRCULAR YELLOW L.E.D. MODULES (B,C,D,E,F,G,H)

12" YELLOW LEFT ARROW L.E.D. MODULES (A)

PLUS NECESSARY DUCT, CABLE, LABOR, MISCELLANEOUS MATERIAL AND EQUIPMENT TO COMPLETE THE INSTALLATION AND PROVIDE AN OPERATING TRAFFIC CONTROL SIGNAL.

### DETECTOR SCHEDULE

	DE ⁻	TECTOR		AMP	LIFIER	PHASE	PHASE	DELAY/		LC	OPS		DETECTION
NO.	STREET	DIRECTION	LANE	CHANNEL	SETTING	CALLED	EXTENDED	EXTENSION	SIZE (FT)	SEGMENTS	TURNS	CONNECTIONS	MODE
1	PARADISE RD	NB	LEFT	1	PRESENCE	1	1		6 X 6	3	3	SERIES	PRESENCE
2	PARADISE RD	NB	LEFT	2	PRESENCE	_	_		6 X 6	1	3	_	SYSTEM
3	PARADISE RD	NB	THROUGH	3	PRESENCE	_	_		6 X 6	1	3	_	SYSTEM
4	PARADISE RD	NB	THROUGH	4	PRESENCE	6	6		6 X 6	3	3	SERIES	PRESENCE
5	LORING AVENUE	SB	RIGHT/THROUGH	5	PRESENCE	2	2		6 X 6	3	3	SERIES	PRESENCE
7	LORING AVENUE	SB	THROUGH	7	PRESENCE	2	2		6 X 6	4	3	S/P	PRESENCE
8	LORING AVENUE	NEB	RIGHT	8	PRESENCE	4	4		*	3	3	SERIES	PRESENCE
9	LORING AVENUE	NEB	RIGHT	9	PRESENCE	_	_		6 X 6	1	3	_	SYSTEM
10	LORING AVENUE	NEB	LEFT	10	PRESENCE	_	_		6 X 6	1	3	_	SYSTEM
11	LORING AVENUE	NEB	LEFT	11	PRESENCE	4	4		6 X 6	3	3	SERIES	PRESENCE
B1	PARADISE RD	NB	LEFT	12	PRESENCE	1	1		6 X 6	1	4	_	BICYCLE
B4	PARADISE RD	NB	THROUGH	13	PRESENCE	6	6		6 X 6	1	4	_	BICYCLE
B5	LORING AVENUE	SB	RIGHT/THROUGH	14	PRESENCE	2	2		6 X 6	1	4	_	BICYCLE
B8	LORING AVENUE	NEB	RIGHT	15	PRESENCE	4	4		6 X 8	1	4	_	BICYCLE
B11	LORING AVENUE	NEB	LEFT	16	PRESENCE	4	4		6 X 6	1	4	_	BICYCLE

SIGNAL HEAD DATA

B,C,D,G

E,F,H

A

P1-P6

(L.E.D.)

CONTROLLER MAKE & MODEL:	PEEK 3000E
UTILITY POLE No.	NET&T 87, MECO 2641
METER No.	94717408
EMERGENCY PRE-EMPTION (TYPI	E): OPTICOM

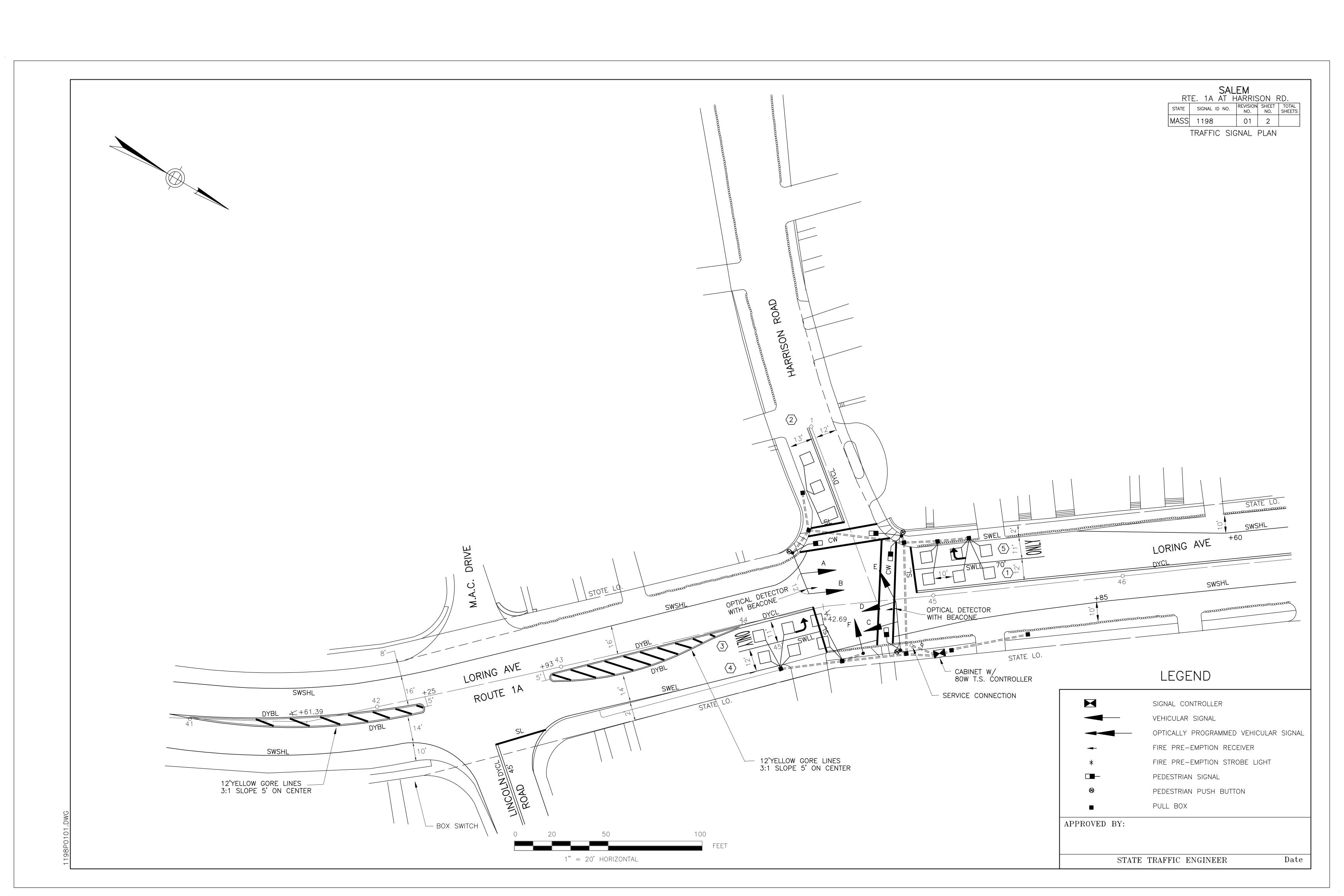
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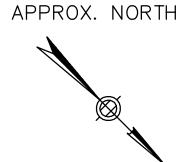
STATE TRAFFIC ENGINEER

Date

201D01_DWC

* 2 - 6 x 6, 1 - 6 x 10





#### Ø 1 ø 2 ø 3 ø 4 ø 5 ø 6 ø 7 ø 8

EMERGENCY PRE-EMPTION DATA

APPROACH

PHASE

TIME

(SEC)

STREET	DIRECTION	HOUSINGS	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	FLASH OPER.
LORING AVE (RTE. 1A)	NB	C,D				R	R	R	R	R	R	R	R	R				G	Υ	R							FY
LORING AVE (RTE. 1A)	SB	A,B				G	Υ	R	R	R	R	R	R	R				R	R	R							FY
HARRISON RD.	EB	E,F				R	R	R	R	R	R	G	Υ	R				R	R	R							FR
PEDESTRIAN	ALL	ALL				DW	DW	DW	W	FDW [	DW _	DW	DW	DW				DW	DW	DW							OFF
					7	'IMIN	VG II	V SE	CONI	DS'																	
MINIMUM GREEN (INITIAL)						10						8						10									
PASSAGE TIME (VECHICLE)						4						4						4									
MAXIMUM 1						52						13						52									
MAXIMUM 2						50						15						50									1.2
YELLOW CLEARANCE							3						3						3								EN
RED CLEARANCE								2						2						2							RC NZ
WALK (W)									4																		VE.
PEDESTRIAN CLEARANCE										14																	EMERGENCY ONLY
RECALL							SOF	<u> </u> - T		OFF			OFF						SOF	<u> </u>							
MEMORY						L	OCKIN	۱G	NON	-LOCK	ING I	NON-	-I OC	KING				L	OCKI	٧G							

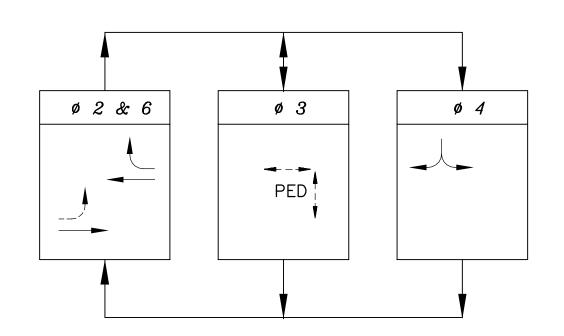
QUANTITY	ITEM
1	CONTROLLER TYPE 8DW, CAB.& FDN.
1	SERVICE CONNECTION, TYPE OVERHEAD
2	8' SIGNAL POLE, BASE, & FDN.
1	10' SIGNAL POLE, BASE, & FDN.
2	25 FT MAST ARM ASSEMBLY, BASE & FDN.
4	1 WAY, 3 SECTION, SIGNAL HOUSING (12" LENS
1	2 WAY, 3 SECTION, SIGNAL HOUSING (12" LENS
4	PEDESTRIAN HOUSING (TYPE FIBER OPTIC)
3	PEDESTRIAN PUSH BUTTON, SIGN & SADDLES
3	DUAL CHANNEL LOOP DETECTOR AMPLIFIER
15	ROADWAY LOOP DETECTOR
10	12" X 12" PULL BOX
	Necessary duct, cable, labor, miscellaneous
	material and equipment to complete the installation

### LOOP DETECTOR DATA

DETECTOR NUMBER	NUMBER OF SEGMENTS	LOOP SIZE	NUM. OF TURNS	ø CALLED	ø EXT.	MODE PULSE PRESENCE	DELAY TIME	EXT. TIME
1	3	6'x6'	3	Ø ₂	Ø ₂	PRESENCE	_	_
2	1 2	10'x6" 6'x6'	2 3	Ø ₄	Ø ₄	PRESENCE	3	_
3	3	6'x6'	3	Ø ₆	ø ₆	PRESENCE	_	_
4	3	6'x6'	3	Ø ₆	ø ₆	PRESENCE	_	_
5	3	6'x6'	3	Ø ₂	ø ₂	PRESENCE	3	ı
	· · · · · · · · · · · · · · · · · · ·		· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·	·			·

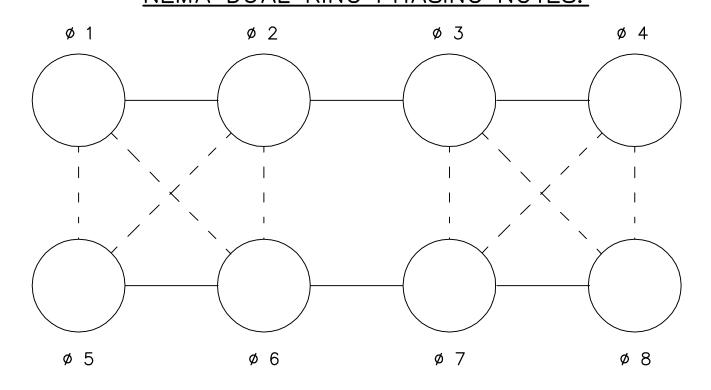
DETECTOR NUMBER	NUMBER OF SEGMENTS	LOOP SIZE	NUM. OF TURNS	ø CALLED	ø EXT.	MODE PULSE PRESENCE	DELAY TIME	EXT. TIME
1	3	6'x6'	3	Ø ₂	Ø ₂	PRESENCE	_	_
2	1 2	10'x6" 6'x6'	2 3	Ø ₄	Ø ₄	PRESENCE	3	_
3	3	6'x6'	3	Ø ₆	Ø ₆	PRESENCE	_	_
4	3	6'x6'	3	Ø ₆	Ø ₆	PRESENCE	_	_
5	3	6'x6'	3	Ø ₂	Ø ₂	PRESENCE	3	_

Ø ₆	Ø ₆	PRESENCE	_	_
ø ₆	Ø ₆	PRESENCE	_	_
ø ₂	Ø ₂	PRESENCE	3	_

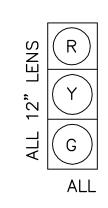


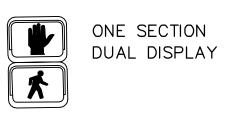
PREFERENTIAL PHASING SEQUENCE

### NEMA DUAL RING PHASING NOTES:



### SIGNAL IDENTIFICATION





12" FIBER OPTIC

CONTROLLER MAKE & MODEL: TCT LMD 9200 UTILITY POLE No. MECO 2606 98 966 009 METER No. EMERGENCY PRE-EMPTION (TYPE): OPTICOM

Date

SALEM RTE. 1A AT HARRISON RD.

STATE SIGNAL ID NO. REVISION SHEET TOTAL NO. NO. SHEETS

TRAFFIC SIGNAL DATA

MASS 1198 01 3

APPROVED BY:

NOTES:

SEQUENCE AND TIMING NOTES:

NEMA DUAL RING PHASING NOTES:

OPERATE CONCURRENTLY.

OPERATE CONCURRENTLY.

LOOP DETECTOR NOTES:

NOTED.

1. PHASES ASSOCIATED BY A SOLID LINE SHALL NOT

3. THROUGH MOVEMENTS MAY INCLUDE RIGHT TURNS.

4. IF THE ASSIGNED RIGHT OF WAY FOR ANY TRAFFIC MOVEMENT IS TO REMAIN IN EFFECT DURING THE NEXT CALLED PHASE, THE SIGNAL INDICATIONS FOR

THAT TRAFFIC MOVEMENT SHALL NOT CHANGE

SPLICE PATTERN AND OTHER INFORMATION.

2. DELAY AND EXTENSION TIMES ARE IN SECONDS.

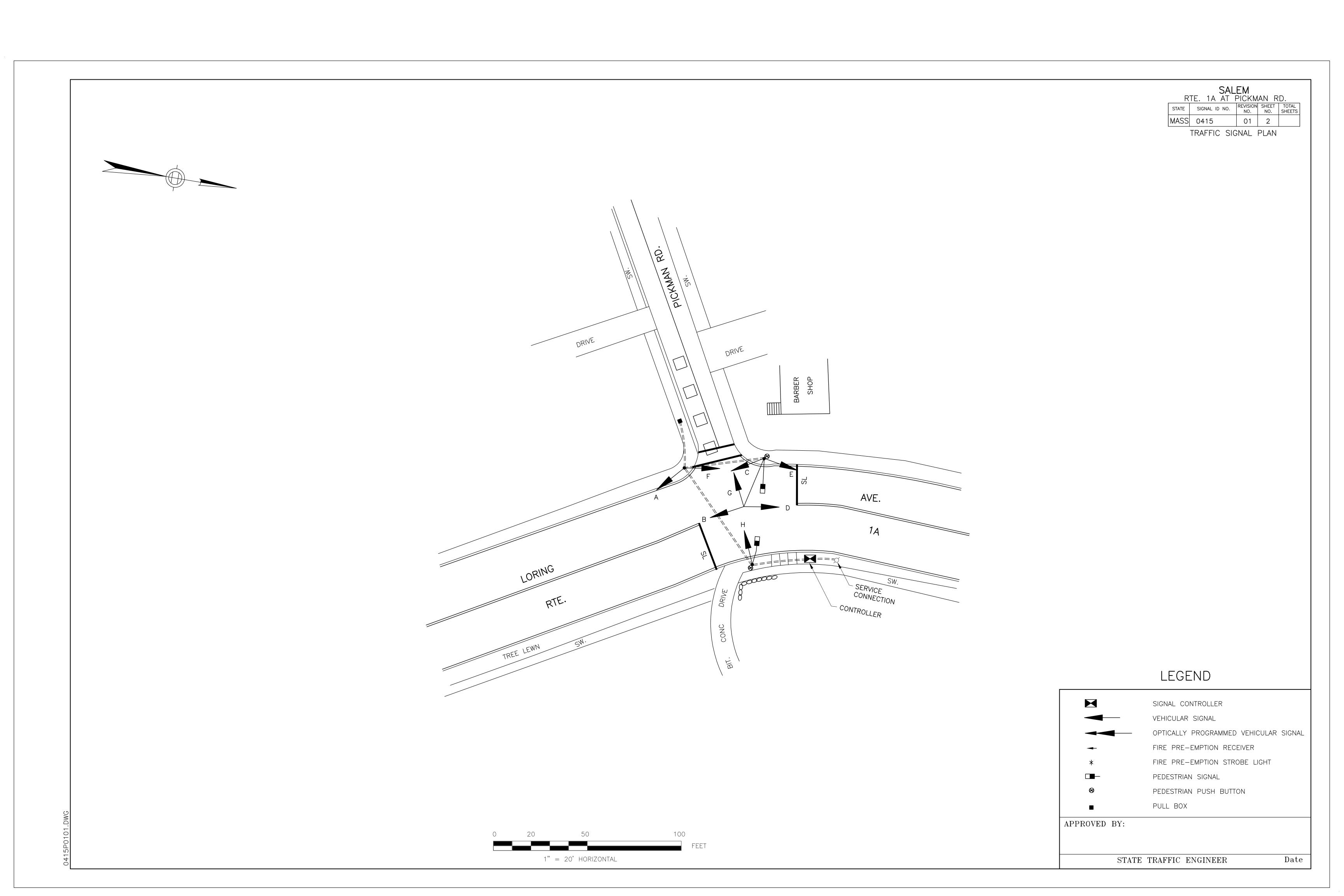
DURING THE CHANGE INTERVAL(S) UNLESS OTHERWISE

1. SEE LOOP DETECTOR DETAIL SHEET FROM DESIGN DOCUMENT FOR

DELAY TIME SHALL BE EFFECTIVE ONLY DURING THE RED PORTION OF THE PHASE THAT IS CALLED BY THE DETECTOR.

2. PHASES ASSOCIATED BY A DASHED LINE MAY

STATE TRAFFIC ENGINEER



STREET	DIRECTION	HOUSINGS	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	FLASH OPER.
LORING AVE (RTE. 1A)	NB	A,B,C	G	Υ	R	R	R	R	R	R	R																FY
LORING AVE (RTE. 1A)	SB	D,E,F	G	Υ	R	R	R	R	R	R	R																FY
PICKMAN RD.	EB	G,H	R	R	R	R	R	R	G	Y	R																FR
PEDESTRIAN	ALL	ALL	DW	DW	DW	W	FDW	/ DW	DW	DW	DW																OFF
					I	'IMIN	G IN	SE	CONL	25							1		l		1						
MINIMUM GREEN (INITIAL)			0						7																		
PASSAGE TIME (VECHICLE)			5						5																		
MAXIMUM 1			35						20																		
MAXIMUM 2																											1.2
YELLOW CLEARANCE				4						4																	EN
RED CLEARANCE					2			1			1																RC
WALK (W)						7																					ZE O
PEDESTRIAN CLEARANCE							9																				EMERCENCY ONLY
RECALL																											
MEMORY																											

QUANTITY	ITEM
1	CONTROLLER, CAB. & FDN.
1	SERVICE CONNECTION, TYPE OVERHEAD
1	MAST ARM ASSEMBLY 30', BASE & FDN.
2	10' SIGNAL POLE, BASE, & FDN.
1	1 WAY 3 SECTION SIGNAL HEAD, 12" LENS
2	2 WAY 3 SECTION SIGNAL HEAD, 12" LENS
1	3 WAY 3 SECTION SIGNAL HEAD, 12" LENS
2	PEDESTRIAN HOUSING INCANDESCENT
2	PEDESTRIAN PUSH BUTTON, SIGN & SADDLES
1	DUAL CHANNEL LOOP DETECTOR AMPLIFIER
4	ROADWAY LOOP DETECTOR
1	12" x 12" PULL BOX
	Necessary duct, cable, labor, miscellaneous

### LOOP DETECTOR DATA

DETECTOR NUMBER	NUMBER OF SEGMENTS	LOOP SIZE	NUM. OF TURNS	ø CALLED	ø EXT.	MODE PULSE PRESENCE	DELAY TIME	EXT. TIME
1	4	6'x6'		Ø ₃	Ø ₃	PRESENCE	_	_
2								
3								
4								
5								
6								
7								
8								

SALEM
RTE. 1A AT PICKMAN RD.

STATE SIGNAL ID NO. REVISION SHEET TOTAL NO. NO. SHEETS

MASS 0415 01 3

TRAFFIC SIGNAL DATA

NOTES:

SEQUENCE AND TIMING NOTES:

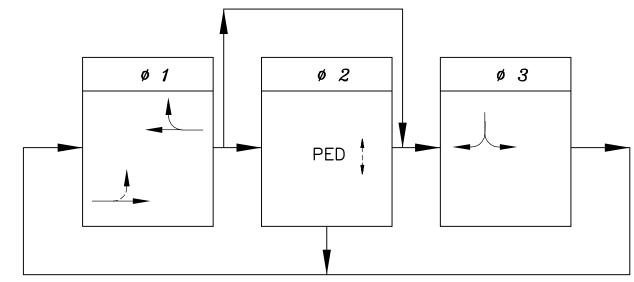
### NEMA DUAL RING PHASING NOTES:

- PHASES ASSOCIATED BY A SOLID LINE SHALL NOT OPERATE CONCURRENTLY.
- PHASES ASSOCIATED BY A DASHED LINE MAY OPERATE CONCURRENTLY.
- 3. THROUGH MOVEMENTS MAY INCLUDE RIGHT TURNS.
- 4. IF THE ASSIGNED RIGHT OF WAY FOR ANY TRAFFIC MOVEMENT IS TO REMAIN IN EFFECT DURING THE NEXT CALLED PHASE, THE SIGNAL INDICATIONS FOR THAT TRAFFIC MOVEMENT SHALL NOT CHANGE DURING THE CHANGE INTERVAL(S) UNLESS OTHERWISE NOTED.

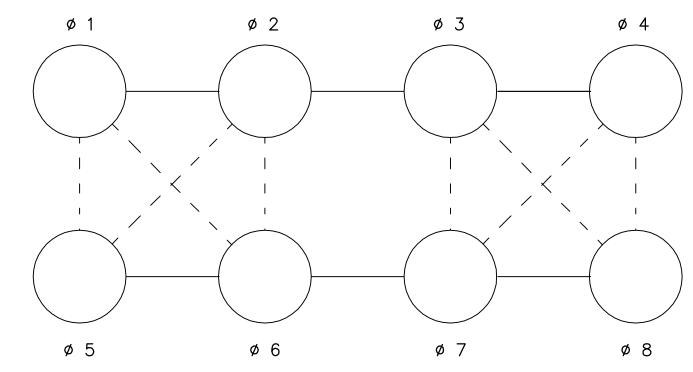
### LOOP DETECTOR NOTES:

- 1. SEE LOOP DETECTOR DETAIL SHEET FROM DESIGN DOCUMENT FOR SPLICE PATTERN AND OTHER INFORMATION.
- 2. DELAY AND EXTENSION TIMES ARE IN SECONDS.
- 3. DELAY TIME SHALL BE EFFECTIVE ONLY DURING THE RED PORTION OF THE PHASE THAT IS CALLED BY THE DETECTOR.

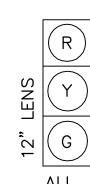
PREFERENTIAL PHASING SEQUENCE



### NEMA DUAL RING PHASING NOTES:



### SIGNAL IDENTIFICATION





12" INCANDESCENT

CONTROLLER MAKE & MODEL: EPAC 300

UTILITY POLE No. NET & T 44 MECO 2595

METER No. 93 049 597

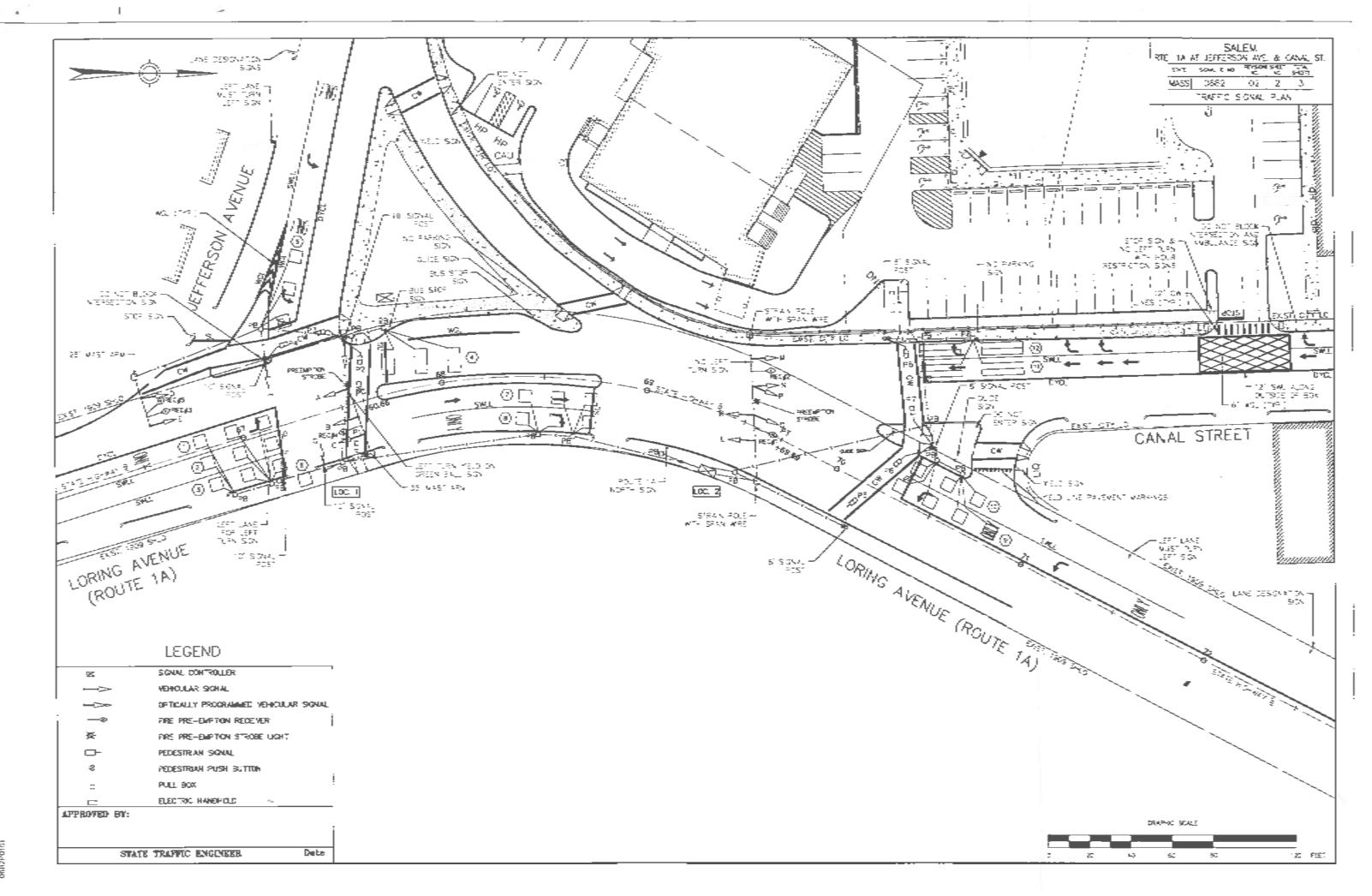
EMERGENCY PRE-EMPTION (TYPE): NONE

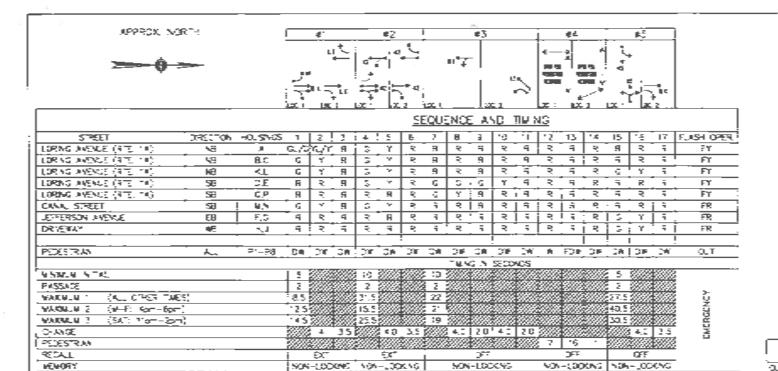
APPROVED BY:

STATE TRAFFIC ENGINEER

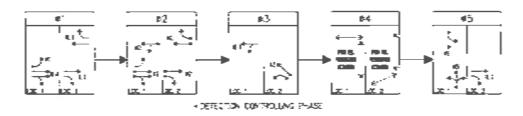
Date

0415D01 DWC





#### PREFERENTIAL PHASE SEQUENCE



EMERSENCY PRE-EMPT	TON DA	<u>TA</u>
Y2000CTCI-	P-ASE	11€ (\$20)
LORING AVE. (9TE. 10), 548	i 42	_
LORANG LAYE (9TE 10) & CANAL STISS	12	_
CORNO (45, (45, 11) NB		-
JEMPSON JAK. EB	165	-

				OP DETECTOR D	CATA				
DETECTOR NO.	NO OF SEGMENTS:	.002 375	SP_OE	NO. OF IUNS	# (U.E)	● Exi.	#306 #=2JLSE #=2865	DELAY	ENT THE
0	3	÷,322.	Р	_		:	8	- '	-
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(B)	3	6X12	2	_	2	2	2	-	-
(3)	5	505	=	<del>-</del>	5	ŝ	3	-	-
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(3)	. ;	€'45	=	_	3		3	-	-
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(3)	į	50000	>	Q.1090,PQL 7995	2	2	3	_	_

SIGNAL IDENTIFICATION.

___NANGP

 $\approx$ 

MAJOR TEMS REQUIRED

DESCRIPTION

ALL 12" LEVS,

CONTROLLER TIPE BOOK CARNET & FON

25' WAST ARM ASSEMBLY, BASE & FIDM. 25" WAS " ARM ASSEMBLY BASE IN FIDE.

SPANIARE ASSOCIATION TO LET, POLES, & FON

" WAY 3 SECTION SCALL HERD, 12" LED LEVS 2 MAY 3 SECTION SOME HEAD, NOT LED LENS

" TAY 5 SECTON SOME HEAD, 12" TED LEVS

PEDESTRUM PLISH PLITTON, SON IN SUDDLE

PRESENCION CONFRHATON STROKE (CLEAR)

A COMPLETE OPERATIVE TRAFFIC CONTROL SOME.

FOADHAY WERDLE LOOP DETECTOR

STT 804 (824.5) - 205/15. PRESENTION RECEIVES

PEDESTRUM HOUSING CRAPHIC LET WITH COLMIDORN TRUST

DUAL DIAMEL LOOP DETECTOR AMPLIFER PHOX WOUNT

PLUS ALL MISCELLANEOUS SOUPMENT AND MATERIAL MEDESSARY TO PROVIDE

ROCHAY VEHICE LOOP DETECTOR (QUADRUPOLE TIPE)

SERVICE CONNECTION (CHERNEAD)

"O" SONAL POST, BASE & FON

E 500A, POST, BASK & FDN.

Q.WTTY₁

BODEFOR R- N

#### SALEM

RTE, TALAT JEFFERSON AVE. & CANALIST.

2712	904K ID	AC SENSOR	S-EET WG.	S-65-2
WSS	9662	63	1	3
-	RAFFIC	SIGNAL	CATA	

#### EMERGENCY PREMETRON CATAL MOTES!

CLEARANCE OF ANY CONFLICTNO NOVEMBNIS IN PROCESSS. FOLIONED BY FRE PRE-BURTON PHASE, FOLIONED BY RETURN TO THE BEGINNING OF THE CHOLE.

#### NOTES:

#### SEQUENCE AND TWAN NOTES:

TO INFORM DALY LIPON PED LICONATION.

#### LOOP DETECTOR NOTES:

- 1. DELAY AND EXPENSION THES ARE IN SECONDS.
- 2. DELAY THE SHALL BE EFFECTIVE ONLY DURING THE RED POSITION OF THE PHASE THAT IS CALLED BY THE DETECTION.

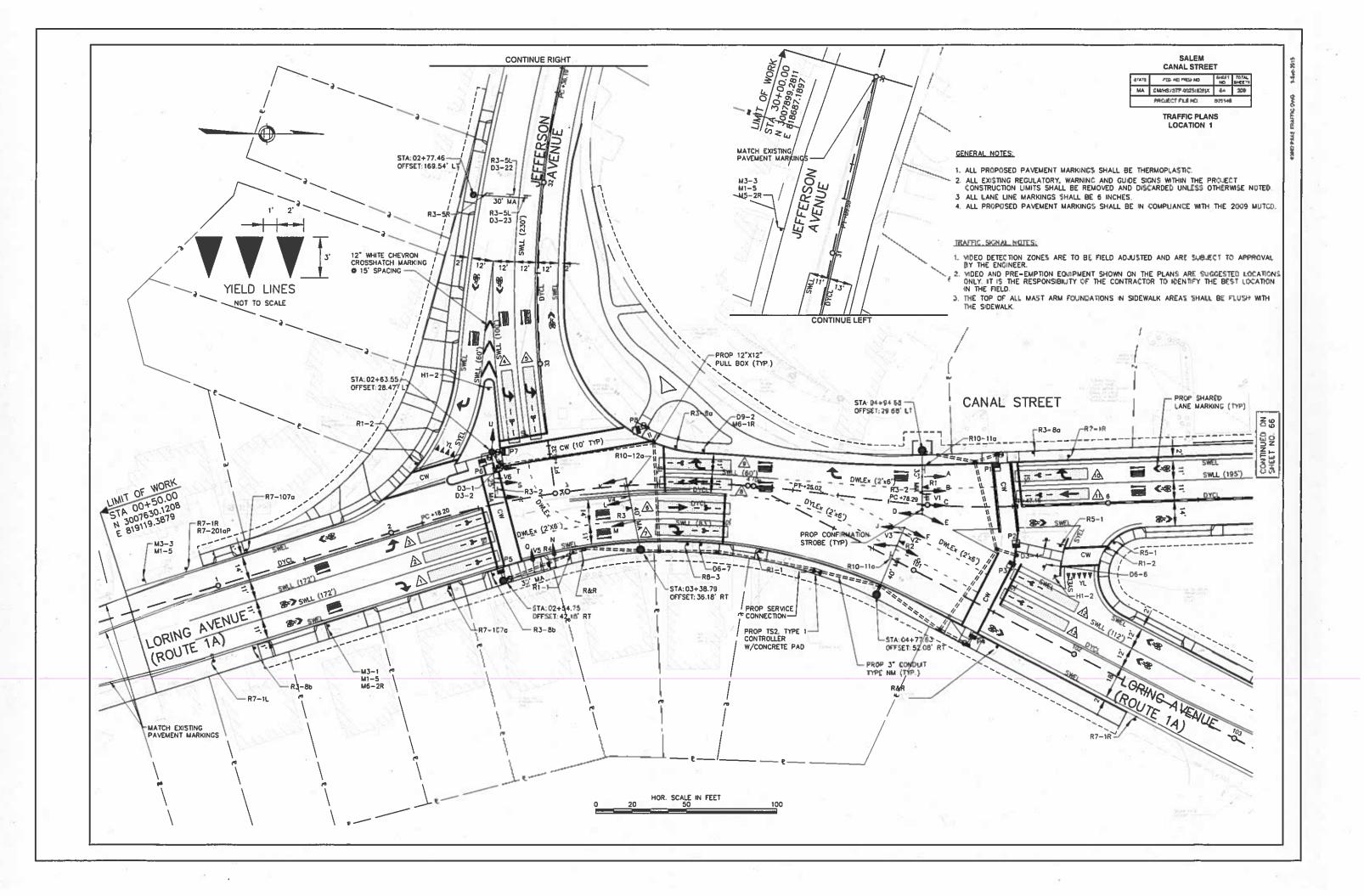
#### SONAL IOENTECATION NOTES:

- TO ALL INDICATIONS ARE LESS TYPE.
- 2 ALL DIESMEAD SONICS ARE ROOLY WOUNTED

1 DODARDA OF RISE PRE-ENFTON SHALL RESULT IN NACOUTE

TOT EVO 8000 CONTROLLER MAKE & MODEL VETHT 1/23 VECO 2577 41 673 550 UDUTY POLE Se. VETER No EVEROENCY PRE-EVPTION (TYPE): OPTICOM APPROVED BY:

STATE TRAFFIC ENGINEER Date



MAJOR ITEMS LIST

6 0 STRAIGHT ARROW
1 WAY J SECTION VEHICLE SIGNAL HOUSING — 12" LED
0 STRAIGHT ARROW
1 WAY 4 SECTION VEHICLE SIGNAL HOUSING — 12" LED
R/Y/Y/G LEFT TURN ARROWS. FLASHING YELLOW ARROW
PEDESTRIAN SIGNAL HOUSING — 18" CRAPHIC LED
ACCESSIBLE PEDESTRIAN SIGNAL ASSEMBLY W/R10—3E SIGN
1 SERVICE CONNECTION (OVERNEAD)
OPTICAL PHASE SELECTOR — TWO CHANNELS
PREEMPTION CONVENIATION STROBE (WHITE)
9 811.31 12" # 12" PULLBOX (SD2.031)
6 WAST ARM CAMERA MOUNT EXTENSION BRACKETS — 8"
VOCO DETECTION PROCESSOR

VIDEO DETECTION PROCESSOR BUS INTERFACE UNIT (SPARE) 2 MALFUNCTION MANAGEMENT UNIT (MMU)
PLUS ALL NECESSARY DUCT, CABLE, LABOR, MISCELLANEOUS MATERIAL AND EQUIPMENT TO COMPLETE THE INSTALLATION

TRAFFIC SIGNAL MAST ARMS, POSTS AND BASES, SHALL BE ORNAMENTAL TYPE CONSISTENT WITH CITY OF SALEM STANDARDS AND PAINTED BLACK.

4

__2__

3

3

5

6

6 2

DESCRIPTION
TSZ TYPE 1 CONTROLLER W/CABINET & FOUNDATION
SICHAL POST & BASE - W/ FDN - B'
25' MAST ARM - STEEL, TYPE 8 W/ FDN
30' MAST ARM - STEEL, TYPE 8 W/ FDN
35' MAST ARM - STEEL, TYPE 8 W/ FDN
40' MAST ARM - STEEL, TYPE 8 W/ FDN
40' MAST ARM - STEEL, TYPE 8 W/ FDN

1 WAY J SECTION VEHICLE SIGNAL HOUSING - 12" LED, R/Y/G LEFT TURN ARROWS 1 WAY J SECTION VEHICLE SIGNAL HOUSING - 12" LED,

1 WAY 3 SECTION VEHICLE SIGNAL HOUSING - 12 (ED., 1 WAY 3 SECTION VEHICLE SIGNAL HOUSING - 12 (LED.) 1 WAY 3 SECTION VEHICLE SIGNAL HOUSING - 12 (LED.)

	CANAL STREE	T	
STATE	FED. AID PROJ. NO	SHEET	TOTAL SHEETS
MA	CM/HSI/STP-002S(826)X	85	209
	PROJECT FILE NO	605146	
S	EQUENCE & TIMING LOCATION 1	3 PLÄ	N

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LORING AVENUE	NB	D,J	R	R	R	R	R	R	R	R	R	F .	Y (							8		_			$\square$	- 6	Y ①			R	R	R	R	R 1
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PEDESTRIAN	ALL	P1-P8	DW	DW	DW	DW	DW	DW	DW	DW.	DW	DW	OW	DW		+	DY	ı D	w c	) WC		_		1		DW	DW	DW	OW	DW	DW	w	FDW	DW O
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			* YIEL	.D CO1	NTROL									II			TIMIN	IG IN	SECO	DNDS														
MINIMUM CREEN			4	- 0.8		10			1 4		1	4	1			1	10			1						- 3			3					
MAX GREEN 1 (M - F, SAM TO			16			39		I	19		I	23					16									3			3					
MAX GREEN 2 (ALL OTHER TIM	ES)		11			38			20			23					22									2			_ 3					,
																																		}
																																		;
VEHICLE EXTENSION			3			2			3			3					3						<u> </u>			3			3					}
PED INTERVALS																IT																5	28	
YELLOW CLEARANCE				3			4			4			3					4		100							4			4				
RED CLEARANCE					2			3		1	3		1	2						3		1						3			3			4 6
DETECTION (MEMORY)				HON-LOC	ж		NON-LOC			NON-LOC	К		NON-LOC	K					-LOCK								HON-LOCK		!	HON-LOC	<u> </u>		LOCK	
RECALL				OFF			SOFT		]	OFF			OFF					SC	YT .								OFF			OFF			OFF	

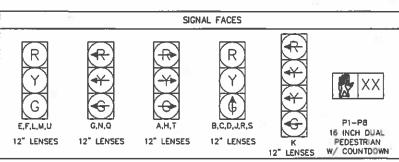
- 1. STANDARD NEMA CLEARANCES SHALL APPLY.
- 2. ANY TWO NON-CONFLICTING PHASES MAY RUN CONCURRENTLY. IN PRESENCE OF CALLS ON MULTIPLE PHASES SEQUENCE SHALL CONFORM TO PREFERENTIAL DIAGRAM.
- 3. SHE FOLLOWED BY #4. 4. SIF FOLLOWED BY #2+#6
- 5. & IF FOLLOWED BY #2+#6.
- 6. \$ IF FOLLOWED BY #4 OR #2+#6.
- 7. → IF FOLLOWED BY #2+#6.
- 8. IF FOLLOWED BY #4 OR #2+#5. 9. F+YA IF FOLLOWED BY #2+#6.
- 10. \$ IF FOLLOWED BY #2+#6 OR #3.
- 11 -- IF FOLLOWED BY #2+#6 OR #3.
- 12. EXCLUSIVE PED PHASE SHALL BE CALLED UPON PUSH BUTTON ACTUATION ONLY.

		VIDEO C	ETECTION	V ZONI	E\$	
DETECTION ZONE	VIDED CAMERA	SIZE	OPERATION	CALL	(SECONOS)	EXTEND
$\triangle$	V4	6'X40'	PRESENCE	6	-	6
1	V4	6"X40"	PRESENCE	6	- 1	6
_&	V4	6'X40'	PRESENCE	_1_		1
	V5	6'X40'	PRESENCE	4	-	4
<u>\$</u>	V5	6'X40'	PRESENCE	4	-	4
<u>(8)</u>	V3	6'X40'	PRESENCE	6	-	6 & 4
$\triangle$	V3	6'X40'	PRESENCE	6	-	5 & 4
<u>8</u>	V6	6'X40'	PRESENCE	2	-	2 & 3
	V6	6'X40'	PRESENCE	2	-	2 & 3
4	۷ı	6'X40'	PRESENCE	2	-	2
	VI	6'X40'	PRESENCE	2	-	2
12	V2	6'X40'	PRESENCE	3	-	3
133	V2	6'X40'	PRESENCE	3	-	3

	PRE-EMPTION PHASING											
RECEIVER	PRE-EMPT		VEHICLE									
AND	PHASE	MOVEMENT	PHASE									
PRIORITY	ASSIGNMENT		ASSIGNMENT									
R1	1	<b>→</b> 58	#2									
R2	2	MB MB	#3									
R3	3	→ HB	#1 & #6									
R4	4	<b>₩</b> 8	64									

#### NOTES:

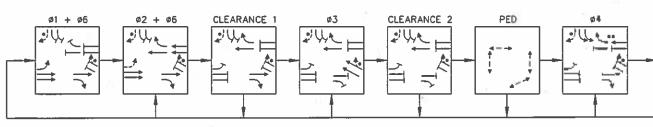
- 1. EMERGENCY VEHICLE PRE-EMPTION SIGNALS SMALL BE OPTICALLY TRANSMITTED BY OPTICAL EMITTERS MOUNTED IN EMERGENCY VEHICLES AND RECEIVED BY OPTICAL RECEIVERS LOCATED AT THE INTERSECTION.
- 2. PRE-EMPTION SIGNALS SHALL BE SERVICED IN THE ORDER IN WHICH THEY ARE RECEIVED, WITH NO PRIORITY.
- 3. IN RESPONSE TO A PRE-EMPTION SIGNAL RECEIVED AT AN INTERSECTION BY OPTICAL RECEIVER RY (OR OTHERS AS PROVIDED) THE CONTROLLER SHALL HOLD OR ADVANCE TO AND HOLD IN EMERCENCY PRE-EMPTION THE ASSOCIATED CREEN PHASE FOR A MINIMUM OF TEN (10) SECONDS. OR UNTIL PRE-EMPTION SIGNAL CEASES. THE CONTROLLER SHALL THEN PROVIDE PRE-EMPTION PHASE CLEARANCE SERVICE THEN RESUME NORMAL OPERATION, AND RETURN TO THE START OF #2 + #6.
- 4. MINIMUM GREEN, NORMAL VEHICLE AND PEDESTRIAN CLEARANCE SHALL BE PROVIDED ON PHASES THAT ARE TO BE TERMINATED BY PRE-EMPTION DEMAND.
- 5. EMERGENCY VEHICLE PRE-EMPTION SHALL OVERRIDE COORDINATION.



- 1 ALL VEHICLE AND PEDESTRIAN LENSES SHALL BE LED TYPE.
- 2. ALL HOUSINGS TO BE PROVIDED WITH TUNNEL VISORS AND 5" BACKPLATES.
- 3. ALL HOUSINGS TO BE FIXED MOUNTED.
- 4. ALL HOUSINGS TO HAVE LOUVERED BACKPLATES WITH 2" YELLOW RETRO-REFLECTIVE BORDERS.

#### PREFERENTIAL SEQUENCE DIAGRAM (CALLS ON ALL PHASES)

CANAL STREET & LORING AVENUE/JEFFERSON AVENUE



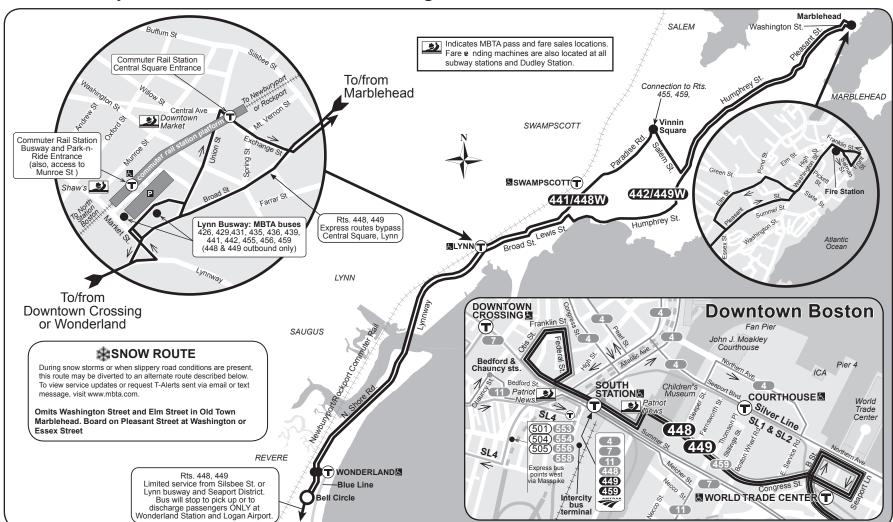
* YIELD CONTROL

** OVERLAP

### **APPENDIX D**

**Bus Schedules** 

### Route 441/442 Marblehead - Wonderland Route 448/449 Marblehead - Downtown Crossing



schedule change

441/442•448/449

Fall September 3, 2016 - December 30, 2016

441/442 Marblehead-Wonderland 448/449 Marblehead-Downtown Crossing

#### Serving

- Vinnin Squa e
- Swampscott
- Wonderland Station
- Central Square, Lynn
- Logan Airport
- Blue Line
- Green Line
- Red Line
- Orange Line
- Newburyport/Rockport Commuter Rail



Information 617-222-3200 • 1-800-392-6100 (TTY) 617-222-5146 • www.mbta.com

441/4	442 &	448/44				Wee	kday						Ī
	L∜ Arrie	L de Aussina	Inbound			Aurim	l			Outbound	Austra	Amrim	
Lea <b>e</b> Marble- head	Phillips Beach iv a Paradise	Ll⁄ Arrie Phillips Beach i⁄ a Humphrey	L# Arrie Central Square	Arrie Wonderland Station	Arri <b>e</b> Logan Term. C	Arrie Otis & Summer Sts.	Leave Otis & Summer Sts.	Arri <b>e</b> Logan Term. C	L\( \text{Arrise}\) Wonderland Station	L# Arrie Central Square	Arrie Phillips Beach i a Humphrey	Arrie Phillips Beach iv a Paradise	Arri <b>e</b> Marble- head
			5:45A	5:59A						w 5:13A	5:22A		5:32A
5:40A		5:50A 6:00	6:05 6:18	6:21 6:30					6:00A	6:13 w 6:16	6:31	6:29	6:40
a 6:00		6:11	0.10	6:36	6:49A	7:03A			6:27	6:39		0.29	0.40
6:10	6:20A		6:40	6:56						w 6:42		6:55	7:06
a 6:28	6:40	6:30	6:48	7:01 7:21	7:32	7:53			6:35 6:45	6:48 6:56	7:06 7:09		7:20
a 0.20	6:45		7:01	7:21		7.55			7:30	7:42	7.03	7:57	8:10
6:43		6:54	7:13	7:34					7:35	7:47	0.44	8:04	
a 6:58		7:00 7:12	7:16	7:36 7:45	8:06	8:22	7:15A	7:29A	7:40 7:47	7:53 7:59	8:11 8:14		8:26
	7:15	7.12	7:35	7:58					7:50	8:03	8:21		
		7:30	7:46	8:05			7:45	7:59	8:06	8:17	8:30	8:45	8:41
7:13 a 7:28	7:26 7:41		7:46	8:09 8:24	8:40	9:01	7.43	7.59	8:18 8:35	8:30 8:47		9:01	8:55 9:13
a 7.20	7:45		8:05	8:28	0.40	9.01			8:55	9:07			
7:43		7:57	8:24	8:44					9:15	9:27	9:42	10.11	9:54 10:23
a 7:58	0.15	8:12	8:35	8:45 8:51	9:06	9:22			9:40 10:15	9:53 10:29	10:44	10:11	10:23
	8:15	8:30	8:47	9:05					10:45	10:58		11:16	11:28
8:13	8:26		8:46	9:09					11:20	11:32	11:47	40-40D	12:00N
0.50	8:45		9:04	9:20					11:45	11:58		12:18P	12:30P
8:50 9:10	9:23	9:04	9:31 9:44	9:51 10:01					12:15P	12:27P	12:42P		12:55P
9:40		9:54	10:21	10:41					12:45	12:56	<u>.</u>	1:13P	1:24
10:10	10:22	. ::	10:44	11:01					1:15 1:45	1:28 2:00	1:45	2:19	2:01 2:32
10:40 11:10	11:22	10:54	11:19 11:44	11:36 <b>12:01P</b>					2:05	2:18	2:35		2:51
11:40		11:52	12:14P	12:31					2:30	2:44		3:08	3:23
									3:00 3:15	3:13 3:30	3:30		3:43
12:10P	12:22P	10.500	12:46P	1:04P					3:30	3:45			
12:40 1:10	1:23	12:52P	1:14 1:47	1:31 2:06					3:35	3:49		4:13	4:28
1:40		1:52	2:14	2:31					4:00 4:05	4:15 4:21	4:40		4:53
2:10	2:25		2:49	3:08					4:10	4:25	4.40		4.55
2:40	••••	2:52	3:14 3:41	3:31 3:56					4:20	4:34		4:58	5:09
3:10	3:23		3:46	4:04					4:35 4:44	4:51 4:59	5:10	••••	5:23
3:40		3:52	4:10	4:29					4:50	5:05		5:29	5:40
4:10	4:22		4:40 4:45	4:55 5:01	••••	••••			5:00	5:15			
4.10			5:10	5:28					5:05 5:20	5:19 5:35	5:37	5:59	5:50 6:08
4:50		5:05	5:25 5:54	5:42			a 4:45	5:05	5:26	5:39	5:56	3.33	6:07
5:20 5:35	5:35	5:47	5:54 6:09	6:13 6:25	••••	••••			5:35	5:49	6:04		6:16
5:50	6:03	3.47	6:22	6:41			a 5:15	5:35	5:50 5:52	6:04 6:05		6:26 6:25	6:36 6:34
6:05		6:17	6:35	6:49			a 5.15	5.35	6:05	6:18	6:32	0.23	6:44
w 6:34 6:35	6:44	6:47	7:00 7:02	7:16			a 5:45	6:04	6:25	6:38	6:55		7:06
w 7:06	7:16	0.47	7:02	7:10			a 6:15	6:33	6:35 6:51	6:49 7:01		7:05 7:21	7:16 7:30
7:15		7:27	7:41	7:56			a 0.15	0.33	7:05	7:18			7.30
w 7:30	7:41		7:56		••••	••••			7:15	7:28	7:42		7:54
8:15 9:15		8:26 9:26	8:40 9:40	8:55 9:55					7:47 9:15	8:00	9:40		9:52
10:15		10:26	10:40	10:55					8:15 9:15	8:27 9:27	8:40 9:40		8:52 9:52
11:15		11:26	11:40	11:55					10:15	10:27	10:40		10:52
w 12:10A	 : Dt-	12:20A	12:28A						11:15	<b>11:26</b> 12:26A	<b>11:38</b> 12:38A		11:47
				an Airport, the s					12:15A x 1:10	1:220A	12:36A		12:47A
Inbound tr	ips bypass C	entral Square	, Lynn buswa	<ol><li>Limited serio</li></ol>	ce between o	downtown	l						000
Lynn and	Seaport Distr	ict, stopping C	JINLY at Wond	derland and Log	an Airport.		E A	เเมนอชอ ส	re accessi	nie to het	SUIIS WILII	นเอสมเไไป	<b>C</b> 3

441/4	42	Satu	ırday		
	Inbound			Outbound	
Lea <b>e</b> Marble- head	Arri <b>e</b> Central Square	Arrie Wonderland Station	Lea <b>e</b> Wonderland Station	L/v Arrie Central Square	Arrie Marble head
p 6:30A h 7:00	6:56A 7:25	7:09A 7:39		wp 6:05A	6:28/ 6:53
p 7:25	7:25 7:52	8:06		wh 6:30 6:57	7:21
h 7:53			p 6:45A h 7:10	7:20	7:42
p 8:25	8:18 8:52	8:32 9:08	p 7:45	7.20 7:57	8:21
h 8:56	9:21	9:36	h 8:13	8:23	8:48
p 9:25	9:54	10:11	p 8:40	8:52	9:17
h 10:00	10:28	10:11	h 9:15	9:28	9:55
p 10:25	10:57	11:15	p 9:40	9:52	10:18
h 10:55	11:23	11:39	h 10:00	10:13	10:10
p 11:20	11:52	12:10P	p 10:25	10:13	11:08
h 11:50	12:17P	12:34	h 10:55	11:08	11:35
11 11.50	12.17	12.54	p 11:22	11:36	12:05F
p 12:15P	12:48P	1:06P	h 11:50	12:03P	12:30
h 12:45	1:13	1:30	11 11.00	12.001	12.00
p 1:10	1:43	2:01	p 12:17P	12:31P	1:011
h 1:40	2:08	2:25	h 12:45	12:58	1:25
p 2:05	2:38	2:56	p 1:12	1:26	1:56
h 2:35	3:03	3:20	h 1:40	1:56	2:22
p 3:00	3:33	3:51	p 2:07	2:20	2:48
h 3:30	3:58	4:15	h 2:35	2:52	3:20
p 3:55	4:26	4:43	p 3:02	3:15	3:43
h 4:25	4:53	5:10	h 3:30	3:44	4:12
p 4:50	5:18	5:34	p 3:57	4:10	4:38
h 5:20	5:46	6:02	h 4:25	4:38	5:02
p 5:45	6:12	6:28	p 4:52	5:05	5:33
h 6:15	6:41	6:57	h 5:20	5:33	5:57
p 6:40	7:07	7:23	p 5:47	6:00	6:28
h 7:10	7:34	7:48	h 6:15	6:28	6:52
h 7:35	7:59	8:13	h 6:40	6:53	7:16
h 8:00	8:24	8:38	h 7:10	7:23	7:47
h 8:35	8:59	9:13	h 7:40	7:53	8:17
h 9:35	9:59	10:10	h 8:40	8:53	9:16
h 10:35	10:57	11:08	h 9:40	9:50	10:13

a - Omits Point of Pines.

h 12:25A 12:47A 12:58 h 11:40

h - Via Humphrey St.

11:57

h 11:35

- p Via Paradise Rd.
- w To or from W. Lynn & operates iv a Western Ae . and does not service Wonderland Station

12:08A **h 10:40** 

10:50

11:50

11:13

12:13A

x - Waits for last train to arrie at station.

Route 441/442 & 448/449 Marblehead-Wonderland Station or Downtown Crossing

11/442	Sunday
Inbound	1

	Inbound			Outbound	l
Lea <b>e</b> Marble- head	Arri <b>e</b> Central Square	Arrie Wonderland Station	Leave Wonderland Station	L\( \text{Arrise} \) Central Square	Arrie Marb hea
h 7:55	8:14	8:37		wh 7:30	7:5
h 8:55	9:13	9:37		wh 8:30	8:54
p 9:25	9:45	10:07	p 8:45	9:00	9:23
h 10:00	10:18	10:42	h 9:15	9:25	9:50
p 10:40	11:00	11:22	p 9:45	10:00	10:23
h 11:05	11:23	11:47	h 10:10	10:20	10:4
p 11:35	11:55	12:22P	p 10:40	10:55	11:19
			h 11:05	11:16	11:42
h 12:00N	12:20P	12:44P	p 11:35	11:50	12:1
p 12:30	12:49	1:16			
h 12:55	1:15	1:39	h 12:00N	12:11P	
p 1:25	1:44	2:11	p 12:30	12:45	1:1
h 1:50	2:08	2:32	h 12:55	1:06	1:3
p 2:20	2:39	3:06	p 1:25	1:40	2:0
h 2:45	3:03	3:27	h 1:50	2:01	2:2
p 3:15	3:34	4:01	p 2:20	2:34	2:5
h 3:40	3:57	4:21	h 2:45	2:56	3:2
p 4:10	4:28	4:51	p 3:15	3:29	3:5
h 4:35	4:52	5:16	h 3:40	3:52	4:18
p 5:05	w 5:23		p 4:10	4:24	4:4
h 5:30	5:47	6:11	h 4:35	4:47	5:1
p 6:00	6:18	6:41	p 5:05	5:19	5:4
h 6:25	6:40	7:02	h 5:30	5:41	6:0
p 6:55	7:13	7:36	p 6:00	6:14	6:3
h 7:25	7:40	8:02	h 6:45	6:56	7:2
h 8:25	8:40	9:02	h 7:45	7:56	8:2
h 9:25	9:40	10:02	h 8:45	8:56	9:2
h 10:25	10:39	11:00	h 9:45	9:55	10:1
h 11:25	w 11:42		h 10:45	10:55	11:1
h12:25A	12:39A	1:00A	h 11:45	11:55	12:18
		Inner	Inner Expre	SS Inner	Fynress

Fare	Inner Express	Inner Express + Local Bus	Inner Express + Subway
CharlieCard	\$4.00	\$4.00	\$4.00
CharlieTicket	\$5.00	\$7.00	\$7.75
Cash-on-Board	\$5.00	\$7.00	\$7.75
Student*	\$2.50	\$2.50	\$2.50
Senior/TAP**	\$2.50	\$2.50	\$2.50

VALID PASSES: Inner Express Bus (\$128/mo.), Outer Express Bus (\$168/mo.), commuter rail, and boat passes.

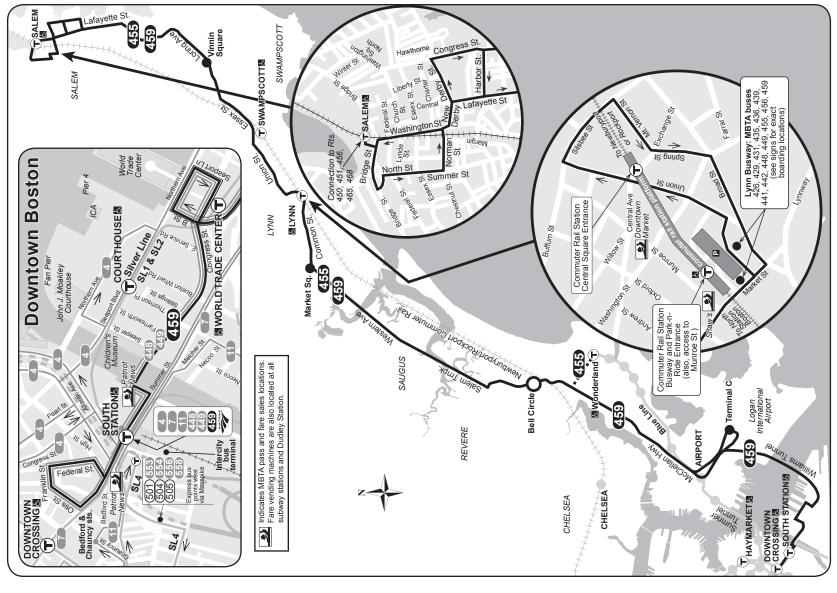
FREE FARES: Children under 12 ride free when accompanied by an adult; Blind Access CharlieCard holders ride free and if using a guide, the guide rides free.

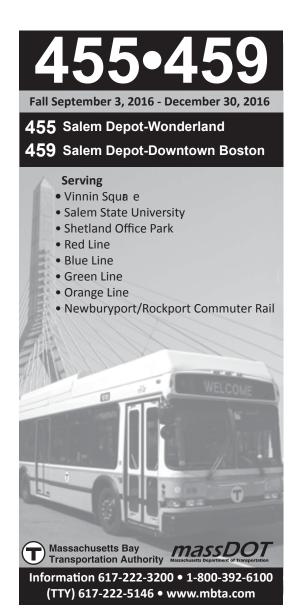
- * Requires Student CharlieCard, available to students through participating middle schools and high schools.
- ** Requires Senior/TAP CharlieCard, available to Medicare cardholders, seniors 65+, and persons with disabilities.

#### Fall 2016 Holidays

October 10 & November 11: see Weekday September 5, November 24 & December 26: see Sunday

Route 455 Salem Depot - Wonderland Route 459 Salem Depot - Downton Boston





455/4	459		Inbound			Wee	kday			Outbound				45		ound	Satu	r(
Lea <b>e</b> Salem Depot	Arrie Vinnin Square	L# Arrie Central Square	Arri <b>e</b> West Lynn	Arrie Wonder- land Station	Arri <b>e</b> Logan Term. C	Arrie Downtown Crossing	Lea <b>e</b> Downtown Crossing	Arri <b>e</b> Logan Term. C	Lea <b>e</b> Wonder- land Station	L∜ Arrie West Lynn	Arri <b>e</b> Central Square	Arrie Vinnin Square	Arrie Salem Depot	Leae Salem Depot	Arri <b>e</b> Vinnin Square	L# Arrie Central Square	Arrie Wonder- land Station	,
		5:00A	5:07A	5:20A						5:21A	5:27A	5:37A	5:50A			5:15A	5:37A	
		5:15	5:22	5:35						5:57	6:04	6:15	6:30			5:55	6:17	
5:10A	5:23A	5:34	5:44	5:56					6:15A	6:26	6:35	6:48	7:06	6:05A	6:16A	6:32	6:53	
5:30	5:44	5:53	6:03	6:15			6:20A	6:35A		6:52	7:06	7:21	7:39	6:35	6:46	7:02	7:23	
5:50	6:04	6:17	6:27		6:47A	7:03A			7:05	7:17	7:27	7:40	7:58	7:05 7:35	7:16 7:46	7:32	7:53	
6:10	6:24	6:39	6:47	6:59					7:55	8:07	8:17	8:30	8:48	8:05	8:17	8:02 8:33	8:25 8:56	
6:30	6:44	6:59	7:11	7:26			7:45	8:02		8:18	8:31	8:46	9:06	8:35	8:47	9:03	9:26	
6:50	7:06	7:22	7:34	7:49					8:50	9:02	9:12	9:25	9:43	9:05	9:17	9:33	9:56	
7:05	7:21	7:38	7:49		8:16	8:34	9:00	9:17		9:34	9:48	10:04	10:22	9:35	9:48	10:04	10:27	
7:20	7:39	7:55	8:07	8:22					10:00	10:12	10:22	10:37	10:56	10:05	10:18	10:34	10:58	1
7:35	7:54	8:10	8:22	8:37			10:10	10:24		10:42	10:56	11:12	11:30	10:35	10:49		11:30	1
7:55	8:14	8:30	8:42	8:57					11:10	11:23	11:35	11:49	12:08P	11:10	11:24		12:06P	1
8:15	8:37	8:52	9:03		9:19	9:39	11:20	11:34		11:51	12:03P	12:19P	12:45	11:45	11:59	12:16P	12:42	
8:50	9:09	9:25	9:37	9:52										12.20P	12-36D	12:53P	1:18P	۔ ا
9:30	9:49	10:07	10:22		10:43	10:57			12:20P	12:33P	12:48P	1:04P	1:25P	12:55	1:10	1:25	1:50	1
10:05	10:24	10:40	10:52	11:07			12:30P	12:46P		1:04	1:16	1:28	1:52	1:30	1:44	1:59	2:24	'
10:45	11:09	11:26	11:41		12:02P	12:16P			1:30	1:43	1:58	2:14	2:35	2:05	2:19	2:34	2:59	
11:20	11:39	11:55	12:07P	12:22P			1:40	1:56		2:14	2:26	2:42	3:06	2:40	2:54	3:09	3:34	
11:55	12:19P	12:36P	12:51		1:15	1:34			2:40	2:53	3:08	3:26	3:48	3:15	3:29	3:44	4:08	
							2:50	3:08		3:33	3:46	4:04	4:25	3:50	4:03	4:18	4:42	
									3:50	4:09	4:22	4:40	5:02	4:25 5:00	4:38 5:13	4:53 5:28	5:17 5:52	
12:30P	12:55P	1:13P	1:26P	1:39P			4:00	4:20		4:45	4:58	5:11	5:31	5:35	5:48	6:03	6:24	
1:05	1:28	1:44	1:58		2:22P	2:42P			5:00	5:19	5:32	5:50	6:08	6:05	6:17	6:32	6:53	
1:40	2:05	2:25	2:41	2:53	••••		5:10	5:32		6:03	6:14	6:26	6:43	6:35	6:47	7:02	7:23	
e 2:35		2:47	3:01						6:10	6:25	6:39	6:54	7:10	a 7:05	7:16	a 7:30		
2:15	2:38	2:55	3:06		3:26	3:42	6:20	6:38		7:01	7:09	7:21	7:32	7:35	7:47	8:02	8:23	
2:50	3:15	3:35	3:47	3:57	4.07	4.40	7.05	7.40	7:15	7:30	7:39	7:51	8:06	8:35	8:47	9:02	9:23	١.
3:25	3:50	4:07	4:18		4:37	4:49	7:25	7:40		7:56	8:04	8:16	8:27	9:35	9:47	10:02	10:21	
4:00	4:27	4:45	4:57	5:07	F-00	F. FO		••••	8:25	8:37	8:46	8:58	9:13	10:35 11:35	10:45 11:45	11:00	<b>11:19</b> 112:19A	C
4:35	4:59	5:14	5:23		5:39	5:53		•••••	9:30	9:42	9:51	10:03	10:18	11.33	11.45	12.001	12.19A	ı
5:10	5:37	5:55	6:05	6:17	C- 44	C-E0		•••••	10:30	10:40	10:50	11:02	11:17		Wee	kend No	ote: This	ro
5:45	6:04	6:19	6:28	7.11	6:44	6:58		•••••	11:30	11:40	11:50	12:01A	12:12A		Shet	land Offi	ice Park.	
6:15 7:15	6:33 7:33	6:49 7:49	6:59	7:11	•••••	••••		••••	12:30A	12:38A	12:47A	12:57	1:10					
	8:33	7:49 8:49	7:59	8:11	••••	••••	W	eekday l	Note: All t	rips to/fro	m Salem	Depot sei	ne.	ŁΑ	II buse	s are ac	cessible	e t
8:15 9:20	9:36	9:49	8:59 9:57	9:10 10:08	••••	••••	Sh	netland C	office Park	ζ.				0.				
10:35	10:51	11:03	11:08	11:18	••••	••••										Ro	oute 4	5
11:30	11:42	11:53	11:58	12:08A	••••		Shada	d area tri	ne com	l ogan Air	nort the	Soonart D	lictrict	S	lem	Deno	t-Wor	1
	12:42A	12:53A	12:58A	1:08A			Snade			Logan Air n Crossin			istrict			•		
12.00A	12.727	12.00/	12.007	1.00		••••		and	DOWNTOWN	1 01000111	g (Noute :	100).				DOMI	ntown	1 (

45	55		Satu	rday				45	5
	Inl	bound		ı	Outb	ound			
Lea <b>e</b> Salem Depot	Arrie Vinnin Square	Ll/ Arrie Central Square	Arrie Wonder- land Station	Leave Wonder- land Station	Arrie Central Square	Arrie Vinnin Square	Arrie Salem Depot	Leae Salem Depot	Ar Vii Sq
6:05A 6:35 7:05 7:35 8:05 8:35 9:05 10:05 10:35 11:10 11:45	6:16A 6:46 7:16 7:46 8:17 9:17 9:48 10:18 10:49 11:24 11:59	5:15A 5:55 6:32 7:02 7:32 8:02 8:33 9:03 9:33 10:04 11:06 11:41 12:16P	5:37A 6:17 6:53 7:23 7:53 8:25 8:56 9:26 9:56 10:27 10:58 11:30 12:06P	6:35A 7:05 7:35 8:05 8:35 9:05 9:35 10:05 10:35 11:10	a 5:28A a 5:58 a 6:28 6:58 7:28 7:58 8:28 8:58 9:28 9:58 10:28 11:01 11:35 <b>12:12P</b>	5:39A 6:09 6:39 7:12 7:42 8:12 8:42 9:12 9:42 10:12 10:13 11:16 11:50 12:27P	5:51A 6:21 6:51 7:23 7:53 8:23 8:53 9:25 9:25 10:25 10:57 11:30 12:04P	7:05A 8:05 9:05 10:05 11:05 12:05P 1:05 2:05 3:05 4:05 5:05 6:05 7:05 8:05	7: 8: 9: 10: 11: 12: 1: 2: 3: 4: 5: 6: 7: 8:
12:20P 12:55 1:30 2:05 2:40	12:36F 1:10 1:44 2:19 2:54	12:53P 1:25 1:59 2:34 3:09	1:18P 1:50 2:24 2:59 3:34	12:20P 12:55 1:30 2:05 2:40	12:47P 1:22 1:57 2:32 3:07	1:02P 1:37 2:12 2:47 3:22	1:16P 1:51 2:26 3:01 3:36	9:05 10:05 a 11:05 11:30	9: 10: 11: 11: a
3:15 3:50 4:25 5:00 5:35 6:05 6:35 a 7:05 7:35 8:35	3:29 4:03 4:38 5:13 5:48 6:17 6:47 7:16 7:47	3:44 4:18 4:53 5:28 6:03 6:32 7:02 a 7:30 8:02 9:02	4:08 4:42 5:17 5:52 6:24 6:53 7:23  8:23 9:23	3:15 3:50 4:25 5:00 5:35 6:05 6:35 7:35 8:35 9:35	3:41 4:15 4:50 5:25 5:57 6:27 6:57 7:57 8:56 9:56	3:56 4:30 5:05 5:40 6:12 6:42 7:12 8:12 9:10	4:10 4:44 5:19 5:53 6:25 6:55 7:25 8:25 9:22 10:22	Cha Cas	e arlie arlie h-ordent
9:35 10:35	9:47 10:45	10:02 11:00	10:21 11:19	10:35 c11:20	<b>10:56</b> 12:02A	<b>11:10</b> 12:18A	<b>11:20</b> 12:32A	Ser	ior/

Weekend Note: This route does not sere Shetland Office Park.

All buses are accessible to persons with disabilities

Route 455 & 459 **Salem Depot-Wonderland Station or Downtown Crosssing** 

55		S	u	าต	ay
	Lorde account				

		Inbo	ound		Outbound						
	Lea <b>e</b> Salem Depot	Arrite Vinnin Square	Arrie Central Square	Arrite Wonder- land Station	Lea <b>e</b> Wonder- land Station	L/v Arrie Central Square	Arrite Vinnin Square	Arrite Salem Depot			
۲	7:05A	7:19A	7:35A	7:58A		a 6:30A	6:44A	6:54A			
	8:05	8:19	8:35	8:58		a 7:00	7:14	7:24			
	9:05	9:19	9:35	9:58	8:05A	8:30	8:44	8:54			
	10:05	10:19	10:35	10:58	9:05	9:30	9:44	9:54			
	11:05	11:19	11:35	11:58	10:05	10:30	10:44	10:54			
					11:05	11:30	11:44	11:54			
	12:05P	12:19P	12:35P	12:58P							
	1:05	1:19	1:35	1:58	12:05P	12:30P	12:44P	12:54F			
	2:05	2:19	2:35	2:58	1:05	1:30	1:44	1:54			
	3:05	3:19	3:35	3:58	2:05	2:30	2:44	2:54			
	4:05	4:19	4:35	4:58	3:05	3:30	3:44	3:54			
	5:05	5:19	5:35	5:58	4:05	4:30	4:44	4:54			
	6:05	6:19	6:35	6:58	5:05	5:30	5:44	5:54			
Ρ	7:05	7:19	7:35	7:58	6:05	6:30	6:44	6:54			
	8:05	8:19	8:35	8:58	7:05	7:30	7:44	7:54			
	9:05	9:19	9:35	9:58	8:05	8:30	8:44	8:54			
Р	10:05	10:19	10:35	10:58	9:05	9:30	9:44	9:54			
	a 11:05	11:22	11:35		10:05	10:30	10:44	10:54			
	11:30	11:44	12:00M	12:23A	10:58	11:23	11:37	11:47			

- a To/from West Lynn Garage
- c Route 426 iv a Cliftondale Square
- e From Brookline St. at Empire St. and does NOT run during school a cation

Fare	Local Bus	Inner Express	Inner Express + Local Bus	Inner Express + Subway
CharlieCard	\$1.70	\$4.00	\$4.00	\$4.00
CharlieTicket	\$2.00	\$5.00	\$7.00	\$7.75
Cash-on-Board	\$2.00	\$5.00	\$7.00	\$7.75
Student*	\$0.85	\$2.50	\$2.50	\$2.50
Senior/TAP**	\$0.85	\$2.50	\$2.50	\$2.50

VALID PASSES: Inner Express Bus (\$128/mo.), Outer Express Bus (\$168/mo.),

commuter rail, and boat passes.

FREE FARES: Children under 12 ride free when accompanied by an adult; Blind Access CharlieCard holders ride free and if using a guide, the guide rides free.

- * Requires Student CharlieCard, available to students through participating middle schools and high schools.
- ** Requires Senior/TAP CharlieCard, available to Medicare cardholders, seniors 65+, and persons with disabilities.

Local bus fare applies if your trip does not cross the Tobin Bridge or Boston Harbor

Fall 2016 Holidays October 10 & November 11: see Weekday September 5, November 24 & December 26: see Sunday

#### **NEWBURYPORT/ROCKPORT LINE Effective November 21, 2016**

Trains shaded in blue WILL NOT OPERATE when the Commuter Rail is operating at a BLUE LEVEL

**Massachusetts Bay Transportation Authority** 



Inbound to Boston 100 150 102 104 106 160 108 162 112 114 170 172 120 174 122 180 128 182 Bikes Allowed 60 Rockport 4:55 5:48 6:30 11:00 12:30 1:50 3:20 Gloucester 12:37 5:02 7:17 9:27 11:07 1:57 West Gloucester 6:43 8:28 f 7:28 f 8:58 ሌ 5:08 6:01 7:23 f 11:13 f 12:43 f 2:03 f 3:33 f 10:43 3:40 Manchester 6:50 7:30 12:50 10:50 5:15 6:08 2:10 **Beverly Farms** b 5:22 6:15 6:57 7:37 8:42 f 11:27 f 12:57 f 2:17 f 3:47 f 6:12 f 7:42 f 9:12 f 10:57 **Prides Crossing** Montserrat b 5:28 6:22 7:04 7:44 8:48 f 9:53 f 11:33 f 1:03 f 2:23 f 3:53 f 5:43 f 6:18 f 7:48 f 9:18 f 11:03 Newburyport 5:20 5:50 6:25 7:00 7:30 7:55 9:15 - 10:00 - 11:40 1:10 2:55 4:44 5:46 8:10 9:25 - 11:03 Rowley 5:25 5:55 6:30 7:05 8:00 f 9:20 f 10:05 f 11:45 f 3:00 f 4:49 f 5:51 f 8:15 f 9:30 Ipswich 5:31 6:02 6:37 7:12 7:42 8:07 10:11 11:51 1:21 4.55 5:57 Hamilton/Wenham & 5:37 6:08 6:43 7:18 8:13 f 9:32 f 10:17 f 11:57 f 1:27 f 3:12 f 5:01 f 6:06 f 8:32 f 9:42 f 11:20 North Beverly 5 5:41 6:13 6:48 7:23 7:53 8:18 Beverly **b** 5:33 5:47 6:18 6:27 6:53 7:09 7:28 7:49 7:58 8:24 8:33 8:54 9:42 9:58 10:27 11:38 12:07 1:08 1:37 2:28 3:22 3:58 5:11 6:14 8:42 9:52 11:08 11:30 Salem **5** 5:37 5:51 6:22 6:31 6:57 7:13 7:33 7:53 8:02 8:28 8:37 8:58 9:46 10:02 10:31 11:42 12:11 1:12 1:41 2:32 3:26 4:02 5:15 5:39 5:52 6:18 6:27 7:37 7:57 8:46 9:27 9:56 11:12 11:34 Swampscott b 5:45 5:59 6:39 7:05 7:41 8:01 9:06 9:54 10:10 10:39 11:50 12:19 1:20 1:49 2:40 3:34 4:10 5:23 6:00 6:35 8:54 2 Lvnn \$ 5:49 6:03 6:43 7:09 7:45 8:49 9:10 9:58 10:14 10:43 11:54 12:23 1:24 1:53 2:44 3:38 4:14 5:27 6:04 6:39 7:49 8:09 8:58 9:39 10:08 11:24 f 5:52 f 6:06 River Works f 6:46 f 7:12 f 7:48 f 2:47 f 3:41 f 4:17 f 5:30 f 6:07 f 6:42 f 9:42 f 11:27 Chalsas 5:59 6:13 6:53 7:19 7:55 8-59 9-19 9:11 9:31 10:18 10:34 11:03 12:14 12:43 1:44 2:13 3:05 3:59 4:35 5:50 6:05 6:25 6:44 7:00 8:09 8:29 9:18 10:00 10:28 11:45 12:00 1A North Station 8 6:11 6:25 6:49 7:05 7:31 7:40 8:08

Trains in purple box indicate peak period trains

#### Monday to Eriday

Monday to Friday

Mone	lay to Friday																																		
Outh	ound from Boston						Al	М															РМ											A	M
ZONE	STATION TRAIN#	1!	53	101	191	155	103	157	105	159	107	161	109	163	111	165	113	115	167	193	117	169	119	171	173	121	175	123	177	125	179	127	181	129	183
	Bikes Allowed	d	<b>4</b> 6	₫\$	640	₫	640	<i>6</i> €6	640	₫	640	<i>6</i> %	<i>6</i> €6	₫6	640	640												<i>6</i> ₹6	₫\$	<i>6</i> %	₫6	640	<i>6</i> €6	646	₫6
1A		b 6:	26	6:39	7:08	7:37	7:50	8:10	8:35	9:40	10:35	11:20	12:00	1:20	1:50	3:15	3:35	4:15	4:30	4:40	5:00	5:15	5:30	5:40	6:05	6:25	6:45	7:15	7:35	8:45	9:10	10:20	10:50	12:10	12:15
1A	Chelsea		- 1	6:50		177/49			f 8:47			f 11:32	f 12:12			3:27			-	4:52	-	5:27	-	5:52	6:17	6:37	6:57	f 7:27				f 10:32	if 11:02	f 12:22	
2	River Works		- 1	6:57	f 7:26	-	f 8:09	f 8:29	-	-	-	-	-	-	f 2:09		f 3:55		-	-	-	-	-	f 6:00		f 6:45	f 7:05		-	-	-	f 10:39	-	-	-
2	Lynn	8	-	-	7:28	7:57	8:11	8:31	8:55	10:00		11:40			2:11	3:37		4:37	-	5:00	-	5:35	-	6:02	6:25	6:47	7:07	7:36	7:55	9:05	9:30		11:10		
3	Swampscott	8	-	-	7:33	8:02	8:16	8:36	9:00	10:05	11:00			1:45		3:42	4:02	4:42	-	5:05	-	5:40	-	6:07	6:30	6:52	7:12	7:41	8:00	9:10	9:35		11:15	12:35	12:40
3	Salem	b 6:	52	7:07	7:40	8:09	8:23	8:43	9:07	10:12	11:07				2:23	3:49	4:09	4:49	4:57	5:12	5:26	5:47	5:57	6:14	6:37	6:59	7:19	7:48	8:07	9:17					
4		8 6:	56	7:11	7:44	8:13	8:27	8:47	9:11	10:16	11:11	11:56	12:36	1:56	2:27		4:13	4:53	5:02	5:16	5:30	5:51	6:02	6:18	6:41	7:03		7:52	8:11	9:21	9:46	10:57	11:26	12:46	12:51
5	North Beverly	8 87	:00	-	-	f 8:17	-	f 8:51	-		-	f 12:00			-	3:59	-	-	5:07	-	-	5:56	-	6:23	6:46	-	7:28	-	8:16	-	f 9:50	-	f 11:30	-	
5	Hamilton/Wenham	8 f 7		-	-	f 8:21	-	f 8:55	-	f 10:24		f 12:04	-	f 2:04	-	4:03	-	-	5:11	-	-	6:00	-	6:27	6:50	-	7:32	-	8:21	-	f 9:54		f 11:34	- 1	f 12:59
6		8 7:	15	-	-	8:27	-	9:03	-	10:30		12:10	-	2:10	-	4:09	-	-	5:17	-	-	6:06	-	6:33	6:56	-	7:38	-	8:27	-	10:00		11:40	- 1	1:05
7	Rowley	8	-	-	-	f 8:32	-	f 9:08	-	f 10:35		f 12:15	-	f 2:15	-	4:15	-	-	5:23	-	-	6:12	-	6:39	7:02	-	7:44	-	f 8:32	-	f 10:05		f 11:45	-	f 1:10
8		8 7:		-	-	8:40	-	9:16	-	10:43		12:23	-	2:23	-	4:24	-	-	5:31	-	-	6:20	-	6:48	7:10	-	7:52	-	8:40	-	10:13		11:53	-	1:18
4	Montserrat	8	- 1	f 7:15	-	-	f 8:31	-	f 9:15	-	f 11:15	-	f 12:40	-	f 2:31	-	4:17	4:57	-	-	5:34	-	6:06	-	-	7:07	-	f 7:56	-	f 9:25	-	f 11:01		f 12:50	-
5	Prides Crossing		-	-	-	-	-	-	-	-	-	-	-	-	-	-	f 4:21	-	-	-	f 5:38	-		-	-	f 7:11	-	f 8:00	-	-	-		-	-	-
5	Beverly Farms	8		f 7:21	-	-	f 8:37	-	f 9:21	-	f 11:21	-	f 12:46	-	f 2:37	-	4:25	5:03	-	-	5:42	-	6:14	-	-	7:15	-	f 8:04	-	f 9:31	-	f 11:07	-	f 12:56	-
6	Manchester	8		7:26	-	-	8:42	-	9:26	-	11:26	-	12:51	-	2:42	-	4:30	5:08	-	-	5:47	-	6:19	-	-	7:20	-	8:09	-	9:36	-	11:12		1:01	-
7	West Gloucester	8		7:32	-	-	f 8:48	-	f 9:32	-	f 11:32	-	f 12:57	-	f 2:48	-	4:36	5:15	-	-	5:53	-	6:25	-	-	7:26	-	f 8:15	-	f 9:42	-	f 11:18	-	f 1:07	-
7	Gloucester	8		7:39	-	-	8:55	-	9:39	-	11:39	-	1:04	-	2:55	-	4:44	5:22	-	-	6:01	-	6:33	-	-	7:34	-	8:22	-	9:49	-	11:25		1:14	-
8	Rockport	8	-	7:47	-	-	9:03	-	9:47	-	11:47	-	1:12	-	3:03	-	4:53	5:32	-	-	6:10	-	6:42	-	-	7:43	-	8:30	-	9:57	-	11:33		1:22	-

#### Saturday & Sunday

lab	ound to Boston			ΔΙ							РМ				
IIID	ound to boston														
	SATURDAY TRAIN	#	1100	1150	1102	1152	1104	1154	1106	1156	1108	1158	1110	1160	1112
ZONE	STATION SUNDAY TRAIN	#	2100	2150	2102	2152	2104	2154	2106	2156	2108	2158	2110	2160	2112
	Bikes Allowed		<i>6</i> %	<i>6</i> %	949	<i>₫</i> ₺	<i>₫</i> ₽	₫ <b>%</b>	<i>6</i> %	<i>6</i> %	66	949	₫\$	₫6	₫ <b>%</b>
8	Rockport	b	7:00	-	10:00	-	12:00	-	2:00	-	5:10	-	7:30	-	10:00
7	Gloucester	8	7:07	-	10:07	-	12:07	-	2:07	-	5:17	-	7:37	-	10:07
7	West Gloucester	8	f 7:13	-	f 10:13	-	f 12:13	-	f 2:13	-	f 5:23	-	f 7:43	-	f 10:13
6	Manchester	8	7:20	-	10:20	-	12:20	-	2:20	-	5:30	-	7:50	-	10:20
5	Beverly Farms	8	f 7:25	-	f 10:25	-	f 12:25	-	f 2:25	-	f 5:35	-	f 7:55	-	f 10:25
4	Montserrat	8	f 7:31	-	f 10:31	-	f 12:31	-	f 2:31	-	f 5:41	-	f 8:01	-	f 10:31
8	Newburyport	8	-	8:52	-	10:52	-	12:52	-	2:52	-	5:52	-	9:00	-
7	Rowley	8	-	8:58	-	10:58	-	12:58	-	2:58	-	5:58	-	9:06	-
6	lpswich	8	-	9:05	-	11:05	-	1:05	-	3:05	-	6:05	-	9:13	-
5	Hamilton/Wenham	8	-	9:12	-	11:12	-	1:12	-	3:12	-	6:12	-	9:20	-
5	North Beverly	8	-	f 9:15	-	f 11:15	-	f 1:15	-	f 3:15	-	f 6:15	-	f 9:23	-
4	Beverly	8	7:36	9:20	10:36	11:20	12:36	1:20	2:36	3:20	5:46	6:20	8:06	9:28	10:36
3	Salem	b	7:40	9:24	10:40	11:24	12:40	1:24	2:40	3:24	5:50	6:24	8:10	9:32	10:40
3	Swampscott	8	7:46	9:30	10:46	11:30	12:46	1:30	2:46	3:30	5:56	6:30	8:16	9:38	10:46
2	Lynn	b	7:50	9:34	10:50	11:34	12:50	1:34	2:50	3:34	6:00	6:34	8:20	9:42	10:50
1A	Chelsea		f 7:59	f 9:44	f 10:59	f 11:44	f 12:59	f 1:44	f 2:59	f 3:44	f 6:09	f 6:44	f 8:29	f 9:52	f 10:59
1A	North Station	b	8:11	9:56	11:11	11:56	1:11	1:56	3:11	3:56	6:21	6:56	8:41	10:04	11:11

#### Saturday & Sunday

Trains in purple box indicate peak period trains

Οι	tbound from Boston			A	VI						PM				
	SATURDAY TRA	N#	1101	1151	1103	1153	1105	1155	1107	1157	1109	1159	1111	1161	1113
ZONE	STATION SUNDAY TRA	N#	2101	2151	2103	2153	2105	2155	2107	2157	2109	2159	2111	2161	2113
	Bikes Allowed		₫\$	<i>త</i> %	56	640	<i>త</i> ేశు	646	8	56	66	66	₫\$	6%	949
1A	North Station	b	8:30	9:30	10:20	11:30	12:20	1:30	2:20	4:30	5:30	7:15	8:30	10:20	11:30
<b>1A</b>	Chelsea		f 8:41	f 9:41	f 10:31	f 11:41	f 12:31	f 1:41	f 2:31	f 4:41	f 5:41	f 7:26	f 8:41	f 10:31	f 11:41
2	Lynn	b	8:51	9:51	10:41	11:51	12:41	1:51	2:41	4:51	5:51	7:36	8:51	10:41	11:51
3	Swampscott	8	8:54	9:54	10:44	11:54	12:44	1:54	2:44	4:54	5:54	7:39	8:54	10:44	11:54
3	Salem	b	9:01	10:01	10:51	12:01	12:51	2:01	2:51	5:01	6:01	7:46	9:01	10:51	12:01
4	Beverly	8	9:05	10:05	10:55	12:05	12:55	2:05	2:55	5:05	6:05	7:50	9:05	10:55	12:05
5	North Beverly	b	-	f 10:09	-	f 12:09	-	f 2:09	-	f 5:09	-	f 7:54	-	f 10:59	-
5	Hamilton/Wenham	8	-	10:13	-	12:13	-	2:13	-	5:13	-	7:58	-	11:03	-
6	lpswich	b	-	10:20	-	12:20	-	2:20	-	5:20	-	8:05	-	11:10	-
7	Rowley	8	-	10:27	-	12:27	-	2:27	-	5:27	-	8:12	-	11:17	-
8	Newburyport	b	-	10:34	-	12:34	-	2:34	-	5:34	-	8:19	-	11:24	-
4	Montserrat	8	f 9:09	-	f 10:59	-	f 12:59	-	f 2:59	-	f 6:09	-	f 9:09	-	f 12:09
5	Beverly Farms	b	f 9:14	-	f 11:04	-	f 1:04	-	f 3:04	-	f 6:14	-	f 9:14	-	f 12:14
6	Manchester	b	9:20	-	11:10	-	1:10	-	3:10	-	6:20	-	9:20	-	12:20
7	West Gloucester	b	f 9:27	-	f 11:17	-	f 1:17	-	f 3:17	-	f 6:27	-	f 9:27	-	f 12:27
7	Gloucester	8	9:34	-	11:24	-	1:24	-	3:24	-	6:34	-	9:34	-	12:34
8	Rockport	b	9:42	-	11:32	-	1:32	-	3:32	-	6:42	-	9:42	-	12:42

#### **Keep in Mind:**

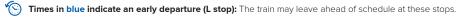
This schedule will be effective from November 21, 2016, and will replace the schedule of May 23, 2016.

Presidents' Day and 4th of July operate on a Saturday service schedule.

New Year's Day, Memorial Day, Labor Day, Thanksgiving Day, and Christmas Day operate on a Sunday service schedule.

For all other holiday schedules, please check MBTÁ.com or call 617-222-3200.

Times in purple with "f" indicate a flag stop: Passengers must tell the conductor that they wish to leave. Passengers waiting to board must be visible on the platform for the train to stop.



Bikes: Bicycles are allowed on trains with the bicycle symbol shown below the train number.









#### **PLEASE NOTE: Schedules** may change in the event of severe weather

Throughout the winter, the MBTA and Keolis will closely monitor weather forecasts to determine if conditions necessitate any change in schedule for the Commuter Rail.

During this time, colors will be used to communicate the system's service level and impact on passengers. The color for the next day will be announced by mid-afternoon the day prior.



Trains will operate on a normal schedule

Moderate changes to train schedule. Shaded trains WILL NOT operate. ORANGE

@MBTA_CR.

Major changes to No passenger train schedule. service on the Schedules will be Commuter Rail available in Boston stations, at MBTA.com and via Twitter

**GRAY** 

### **APPENDIX E**

**Traffic Safety Data** 

### Crash Cluster 1 2011 to 2014

					·			011 to 2014		Г	Г		
						Total	Total	_	Road			Non	
Collision	Crash				Number of	Nonfatal		Manner of		Ambient Light		Motorist	Bike_
Number	Number1	Crash Date1		Crash Severity	Vehicles	Injury	Injury	Collision		Condition	Condition	Туре	Ped
1	2949513	17-Jan-2011		Non-fatal injury	2	3	0	Angle	Dry	Daylight	Clear		
2	2949518	23-Jan-2011	12:20 PM	Non-fatal injury	2	1	0	Angle	Dry	Daylight	Clear		
3	2949520	24-Jan-2011	7:58 AM	Non-fatal injury	2	1	0	Rear-end	Dry	Not reported	Clear		
4	2949567	01-Jul-2011	5:44 PM	Property damage only (n	2	0	0	Rear-end	Dry	Daylight	Clear		
5	2949576	04-Aug-2011	8:21 PM	Property damage only (n	2	0	0	Rear-end	Dry	Dark - lighted road	Clear		
6	2949591	09-Dec-2011	2:10 PM	Non-fatal injury	3	1	0	Rear-end	Dry	Daylight	Clear		
7	2949593	15-Dec-2011	8:58 AM	Non-fatal injury	2	2	0	Angle	Wet	Daylight	Cloudy/Rain		
8	2949609	14-Feb-2012	4:02 PM	Property damage only (n	2	0	0	Angle	Dry	Daylight	Clear		
9	2949630	17-Feb-2011	2:16 PM	Non-fatal injury	2	1	0	Rear-end	Dry	Daylight	Clear		
10	2949647	22-May-2011	8:37 PM	Property damage only (n	2	0	0	Rear-end	Dry	Dark - lighted road	Cloudy/Clear		
11	2949680	09-Sep-2011	2:49 PM	Non-fatal injury	3	6	0	Rear-end	Dry	Daylight	Clear		
12	2949716	11-Dec-2011		Non-fatal injury	2	4	0	Rear-end		Daylight	Clear		1
13	2949737	27-Jan-2012		Non-fatal injury	2	2	0	Angle		Daylight	Rain/Cloudy		
14	2949740	02-Feb-2012		Not Reported	2	0	0	Sideswipe, same di		Daylight	Cloudy		
15	2949741	10-Feb-2012		Non-fatal injury	2	1	0	Rear-end		Dark - lighted road	-		
16	3001162	03-May-2011		Not Reported	3	0	0	Angle		Daylight	Cloudy/Cloudy		1
17	3340479	10-Oct-2012		Property damage only (n		0		Rear-end	-	Daylight	Rain		
18	3340480	10-Oct-2012		Property damage only (n		0	0	Angle		Daylight	Rain		_
19	3340493	06-Nov-2012		Not Reported	2	0	_	Rear-end		Dark - lighted road			
20	3340611	27-Sep-2012		Non-fatal injury	3	1	0	Rear-end		Daylight	Clear		_
21	3340614	05-Oct-2012		Not Reported	1	0		Single vehicle crash		Dark - lighted road		l nke/Rain	
22	3340616	04-Dec-2012		Property damage only (no		0	0	Rear-end		Daylight	Cloudy	JKC/ Kalli	_
23	3340625	19-Dec-2012		Property damage only (no		0		Angle	-	Daylight	Cloudy/Cloudy		
24	3340627	26-Dec-2012		Property damage only (n		0	0	Rear-end		Daylight	Clear		
25	3340873	21-Dec-2012		Not Reported	2	0		Rear-end	-	Daylight	Rain		
26	3340873	01-Oct-2012		Property damage only (n		0		Head-on		Daylight	Not Reported		
27	3368117	12-Feb-2013		Property damage only (n		0	0	Rear-end			·		
		23-Feb-2013				-		Rear-end		Dark - lighted road			-
28	3368119 3391555	12-Mar-2013		Non-fatal injury Not Reported	2	0			-	Daylight	Cloudy		
29				•				Angle		Daylight	Cloudy		-
30	3391556	20-Mar-2013		Property damage only (no		0		Rear-end	Not reporte		Clear/Claar		
31	3481858	12-May-2013		Non-fatal injury	2	2		Head-on		Daylight	Clear/Clear		_
32	3482063	18-May-2013		Non-fatal injury	2	1	0	Angle		Daylight	Clear		
33	3590764	10-Jun-2013		Property damage only (n		0		Sideswipe, opposit		Daylight	Cloudy/Cloudy		_
34	3590769	21-Jun-2013		Property damage only (n		0	0	Rear-end		Daylight	Clear	DC OIL	
35	3590779	06-Jul-2013		Property damage only (no		0		Rear-end	-	Daylight		P6:Other n	qped
36	3590791	30-Aug-2013		Property damage only (n		0	0	Sideswipe, opposit	-	Dark - lighted road			
37	3590840	08-Aug-2013		Property damage only (no		0		Angle		Daylight	Clear		
38	3591025	07-Jun-2013		Property damage only (n		0	0	Angle		Daylight	Rain/Cloudy		
39	3663054	12-Oct-2013		Not Reported	2	0	0	Rear-end		Daylight	Clear		
40	3663439	03-Oct-2013		Property damage only (n		0		Rear-end		Daylight	Clear		<del></del>
41	3711696	03-Sep-2013		Non-fatal injury	2	2		Angle		Daylight	Not Reported		
42	2949608	14-Feb-2012		Property damage only (n		0		Angle		Daylight	Cloudy/Cloudy		
43	2949686	22-Sep-2011		Non-fatal injury	2	2		Rear-end		Daylight	Cloudy		
44	3340482	17-Oct-2012	10:09 AM	Non-fatal injury	2	1	0	Angle	Dry	Daylight	Clear		

### Crash Cluster 1 2011 to 2014

							2	011 to 2014					
Collision	Crash				Number of	Total Nonfatal	Total Fatal	Manner of	Road Surface	Ambient Light	Weather	Non Motorist	Bike
Number	Number1	Crash Date1	Crash Time1	Crash Severity	Vehicles	Injury	Injury	Collision	Condition	Condition	Condition	Туре	Ped
45	3340621	14-Dec-2012		Non-fatal injury	2	1	0	Angle	Dry	Daylight	Clear	7,7	1 2 2
46	3340860	28-Sep-2012		Not Reported	2	0	0	Rear-end	Wet	Daylight	Rain/Cloudy		
47	3391553	05-Mar-2013		Property damage only (n	3	0	0	Rear-end	Dry	Daylight	Cloudy		
48	3481855	07-May-2013		Property damage only (n		0	0	Rear-end	Dry	Daylight	Clear/Clear		$\top$
49	3663042	03-Sep-2013		Property damage only (n		0	0	Angle	Dry	Daylight	Clear		
50	3663047	16-Sep-2013		Property damage only (n	2	0	0	Sideswipe, same di	-	Daylight	Cloudy		
51	3745223	04-Nov-2013	6:25 AM	Property damage only (n	1	0	0	Sideswipe, same di	Dry	Daylight	Clear/Clear		
52	3348277	26-Apr-2011	3:08 PM	Property damage only (n	2	0	0	Angle	Dry	Daylight	Cloudy/Clear		1
53	3549591	25-Jun-2013	10:01 AM	Non-fatal injury	2	1	0	Angle	Dry	Daylight	Clear/Clear		
54	3928144	28-Mar-2014	11:17 AM	Not Reported	1	0	0	Single vehicle crash	Dry	Daylight	Cloudy		
55	3928075	15-Jun-2014	1:08 PM	Property damage only (n	1	0	0	Single vehicle crash	Dry	Daylight	Clear		
56	3928078	24-Jun-2014	6:00 PM	Property damage only (n	2	0	0	Rear-end	Dry	Daylight	Clear		1
57	4003343	24-Dec-2014	11:03 AM	Non-fatal injury	2	1	0	Rear-end	Wet	Daylight	Rain/Cloudy		
58	3928369	21-May-2014	2:20 PM	Not Reported	2	0	0	Angle	Dry	Daylight	Clear/Clear		1
59	4003323	06-Nov-2014	11:37 AM	Not Reported	2	0	0	Rear-end	Wet	Daylight	Rain		
60	4003327	07-Nov-2014	6:13 PM	Property damage only (n	2	0	0	Sideswipe, same di	Dry	Dark - lighted road	Clear		
61	4003337	25-Nov-2014	8:44 PM	Non-fatal injury	1	1	0	Single vehicle crash	Dry	Dark - lighted road	Clear		
62	4003341	07-Dec-2014	3:10 PM	Non-fatal injury	2	1	0	Head-on	Dry	Daylight	Clear		Ī
63	4003334	20-Nov-2014	12:42 PM	Not Reported	2	0	0	Angle	Dry	Daylight	Clear		
64	4003340	06-Dec-2014	12:03 PM	Property damage only (n	3	0	0	Angle	Wet	Daylight	Other		
65	3352280	31-Jan-2013	1:20 PM	Non-fatal injury	2	1	0	Angle	Dry	Daylight	Clear/Clear		
66	3590785	19-Jul-2013	6:14 PM	Property damage only (n	2	0	0	Angle	Dry	Daylight	Clear		
67	3662011	05-May-2012	9:20 PM	Non-fatal injury	1	1	0	Rear-end	Wet	Dark - lighted road	Cloudy		
68	3662013	05-May-2012	2:12 PM	Property damage only (n	2	0	0	Rear-end	Dry	Daylight	Cloudy/Cloudy	,	
69	3662014	21-May-2012	11:17 AM	Property damage only (n	2	0	0	Rear-end	Dry	Daylight	Clear/Clear		
70	2949649	25-May-2011		Non-fatal injury	1	1	0	Single vehicle crash	Dry	Daylight	Clear		$\perp$
71	2949684	18-Sep-2011		Property damage only (n		0	0	Sideswipe, same di	Dry	Daylight	Clear/Clear		
72	3928085	15-Jul-2014	1:43 PM	Property damage only (n	1	0	0	Single vehicle crash		Daylight	Clear		$\perp$
73	3928138	14-Mar-2014	12:26 PM	Non-fatal injury	1	1	0	Single vehicle crash	Dry	Daylight	Cloudy		
74													

### Crash Cluster 2 2011 to 2014

								2011 10 2014				
Collision Number	Crash Number 1	Crash Date1	Crash Time1	Crash Severity	Number of Vehicles	Total Nonfatal Injury	Total Fatal Injury	Manner of Collision	Road Surface Condition		Weather Condition	Bike_Ped
	2711429	27-Feb-2011		Property damage only (no		0	0	Not reported	Not reported		Not Reported	
	2720884	22-Apr-2011		Non-fatal injury	2	1	0	Rear-end	Dry		Clear/Cloudy	
	2737603	03-Jun-2011		Property damage only (no		0	0	Sideswipe, same directi			Clear/Other	
	2743378	21-Jun-2011		Non-fatal injury	2	1	0	Rear-end	Dry		Clear	
	2956630	10-Nov-2011	3:34 PM	Property damage only (no	3	0	0	Rear-end	Wet		Cloudy/Rain	
	2957630	21-Nov-2011	5:05 PM	Non-fatal injury	2	3	0	Rear-end	Dry	Dark - lighted road	Clear	
	3066445	23-Feb-2012	12:55 PM	Property damage only (no	2	0	0	Sideswipe, same directi	Wet	Daylight	Rain/Cloudy	
	3116137	06-Mar-2012	3:55 PM	Property damage only (no	3	0	0	Rear-end	Dry	Daylight	Clear	
	3245624	28-Apr-2012	11:00 PM	Property damage only (no	1	0	0	Single vehicle crash	Dry	Dark - lighted road	Clear	
8	3826322	20-May-2014	6:41 AM	Non-fatal injury	1	1	0	Single vehicle crash	Wet	Daylight	Cloudy/Rain	
	3388582	16-Feb-2013	00:00 AM	Property damage only (no	2	0	0	Rear-end	Wet	Daylight	Cloudy	
g	3999130	04-Dec-2014	5:06 PM	Property damage only (no	1	0	0	Head-on	Dry	Dark - roadway no	Clear/Clear	ped
	3422220	07-Apr-2013	7:14 PM	Property damage only (no	2	0	0	Rear-end	Dry	Daylight	Clear	
	3429093	15-Apr-2013	6:32 PM	Property damage only (no	2	0	0	Rear-end	Dry	Daylight	Clear	
	3487734	25-May-2013	4:38 PM	Non-fatal injury	1	1	0	Single vehicle crash	Wet	Dark - lighted road	Cloudy/Rain	
1	3562261	15-Aug-2013	11:17 AM	Property damage only (no	2	0	0	Rear-end	Dry	Daylight	Clear	
	3563577	28-Jun-2013	9:04 AM	Non-fatal injury	2	1	0	Head-on	Not reported	Daylight	Rain	
2	3590784	19-Jul-2013	9:37 AM	Non-fatal injury	3	1	0	Rear-end	Dry	Daylight	Clear	
6	3623145	15-Oct-2013	11:20 AM	Property damage only (no	1	0	0	Single vehicle crash	Dry	Daylight	Cloudy/Cloudy	
	3665400	31-Oct-2013	9:10 AM	Property damage only (no	2	0	0	Not reported	Not reported	Not reported	Not Reported	
	3711092	20-Dec-2013	12:32 PM	Property damage only (no	3	0	0	Rear-end	Dry	Daylight	Clear/Clear	

### Crash Cluster 3 2011 to 2014

			1	,				10 2014		T	1	1	1
Collision Number	Crash Number 1	Crash Date_1	Crash Time_1	Crash Severity	Number of Vehicles	Total Nonfatal Injuries	Total Fatal Injuries		Road Surface Condition	Ambient Light Condition	Weather Condition	Non Motorist Type	Bike_Ped
1	3603666	04-Sep-2013	10:02 AM	Property damage only (no	2	0	0	Sideswipe, same direc	Dry	Daylight	Clear/Clear		
2	3663044	10-Sep-2013	7:44 AM	Not Reported	2	0	0	Angle	Dry	Daylight	Clear/Clear		
3	3663051	03-Oct-2013	6:17 PM	Property damage only (no	2	0	0	Angle	Dry	Dusk	Clear/Clear		
4	3710967	23-Dec-2013	1:45 PM	Property damage only (no	2	0	0	Angle	Wet	Daylight	Cloudy/Rain		
5	3711086	13-Dec-2013	5:07 PM	Property damage only (no	2	0	0	Angle	Dry	Dark - lighted ro	Clear/Cloudy		
6	3710971	26-Dec-2013	12:31 PM	Non-fatal injury	3	1	0	Angle	Wet	Daylight	Cloudy/Cloudy		
7	3711087	14-Dec-2013	9:18 PM	Property damage only (no	2	0	0	Angle	Dry	Dark - lighted ro	Clear		
8	3711093	21-Dec-2013	1:20 PM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear/Clear		
9	3554204	10-Aug-2013	11:33 AM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear/Clear		
10	3573534	25-Aug-2013	2:03 AM	Property damage only (no	1	0	0	Single vehicle crash	Dry	Dark - lighted ro	Clear/Clear		
11	3608232	01-Oct-2013	7:40 AM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear/Clear		
12	3743308	13-Feb-2014	1:18 PM	Property damage only (no	2	0	0	Angle	Snow	Daylight	Snow/Sleet, hail (fi	reezing rain or drizzle)	
13	3803390	28-Apr-2014	12:45 PM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear/Clear		
14	4003331	14-Nov-2014	4:26 PM	Not Reported	2	0	0	Rear-end	Dry	Dusk	Clear		
15	3803608	13-Apr-2014	3:45 AM	Property damage only (no	1	0	0	Single vehicle crash	Dry	Dark - lighted ro	Clear		
16	3803925	09-May-2014	11:30 AM	Property damage only (no	2	0	0	Rear-end	Wet	Daylight	Cloudy/Rain		
17	3865095	13-Jun-2014	1:16 PM	Non-fatal injury	2	1	0	Sideswipe, opposite d	Wet	Daylight	Cloudy/Rain		
18	3981741	01-Nov-2014	1:00 PM	Non-fatal injury	2	2	0	Angle	Wet	Daylight	Rain/Rain		
19	4000028	09-Dec-2014	3:05 PM	Property damage only (no	2	0	0	Sideswipe, same direc	Wet	Dusk	Cloudy/Rain		
20	3870842	24-Jun-2014	6:30 PM	Property damage only (no	2	0	0	Head-on	Dry	Daylight	Clear/Clear		
21	3909961	17-Jul-2014	1:52 PM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear/Clear		
22		13-Aug-2014	8:45 AM	Non-fatal injury	2	1	0	Angle	Wet		Rain/Cloudy		
23		19-Aug-2014	5:49 PM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear/Clear		
24		19-Oct-2014	10:45 AM	Non-fatal injury	2	1	0	Angle	Dry	Daylight	Clear		
25		23-Oct-2014	6:06 PM	Property damage only (no	2	0	0	Angle	Wet	Dark - lighted ro	Rain/Rain		
26		04-Nov-2014	1:24 PM	Property damage only (no	2	0	0	Sideswipe, opposite d	Dry	Daylight	Clear		
27	3741261	07-Nov-2013	5:45 PM	Property damage only (no	2	0	0	Angle	Wet	Dark - lighted ro	Cloudy/Rain		
28	3563599	15-Jun-2013	4:00 PM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear		
29	3424882	27-Apr-2013	1:15 PM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear		
30	3420007	20-Mar-2013	4:56 PM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear		
31	3417906	14-Mar-2013	8:45 PM	Not Reported	2	0	0	Sideswipe, same direc	Dry	Dark - lighted ro	Clear		
32		25-Jan-2013	3:38 PM	Property damage only (no		0		Angle	Dry	Daylight	Clear		
33	33/4070	09-Jan-2013	2:15 AM	Property damage only (no	2	0	U	Angle	Dry	Daylight	Cloudy		

### Crash Cluster 3 2011 to 2014

			1	1			ı	I	I	I			
					Number	Total	Total		Road				
Collision	Crash				of	Nonfatal	Fatal		Surface	Ambient Light	Weather		
Number	Number 1	Crash Date_1	Crash Time_1	Crash Severity	Vehicles	Injuries	Injuries	Manner of Collision	Condition	Condition	Condition	Non Motorist Type	Bike_Ped
34	3741257	27-Nov-2013	2:55 PM	Non-fatal injury	2	1	0	Angle	Wet	Daylight	Cloudy/Rain		
35	3352280	31-Jan-2013	1:20 PM	Non-fatal injury	2	1	0	Angle	Dry	Daylight	Clear/Clear		
36	3590785	19-Jul-2013	6:14 PM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear		
37	3562261	15-Aug-2013	11:17 AM	Property damage only (no	2	0	0	Rear-end	Dry	Daylight	Clear		
38	3826322	20-May-2014	6:41 AM	Non-fatal injury	1	1	0	Single vehicle crash	Wet	Daylight	Cloudy/Rain		
39	3999130	04-Dec-2014	5:06 PM	Property damage only (no	1	0	0	Head-on	Dry	Dark - roadway	Clear/Clear		ped
	2702347	22-Jan-2011	1	Property damage only (no	3	0	0	Rear-end	Dry	Daylight	Clear		
	2711456	18-Feb-2011	10:15 AM	Non-fatal injury	2	1	0	Rear-end	Dry	Daylight	Clear		
	2712878	13-Feb-2011	4:05 PM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear		
	2721437	16-Apr-2011	8:49 PM	Non-fatal injury	2	1	0	Head-on	Wet	Dark - lighted re	Rain		
	2728110	18-May-2011	2:45 AM	Non-fatal injury	2	3	0	Rear-end	Wet	Daylight	Cloudy/Rain		
	2729229	06-May-2011	7:34 PM	Non-fatal injury	2	1	0	Head-on	Dry	Dawn	Clear		
	2743355	27-Jun-2011	9:45 PM	Non-fatal injury	2	1	0	Angle	Dry	Dark - lighted re	Clear		
	2743381	21-Jun-2011	3:43 PM	Non-fatal injury	2	1	0	Rear-end	Dry	Daylight	Clear		
	2751552	26-Jul-2011	6:33 PM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear		
	2764237	07-Sep-2011	5:07 PM	Property damage only (no	2	0	0	Head-on	Wet	Daylight	Rain		
	2791278	04-Oct-2011	2:26 PM	Non-fatal injury	3	1	0	Rear-end	Wet	Daylight	Cloudy		
	2850972	23-Oct-2011	5:58 PM	Non-fatal injury	2	1	0	Angle	Dry	Daylight	Clear		
	2902580	28-Feb-2011	8:00 AM	Non-fatal injury	1	1	0	Single vehicle crash	Ice	Daylight	Rain/Sleet, hail (fre	ezing rain or drizzle)	
	2949608	14-Feb-2012	3:54 PM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Cloudy/Cloudy		
	2954364	07-Nov-2011	2:30 PM	Non-fatal injury	2	1	0	Rear-end	Dry	Daylight	Clear		
	3038448	17-Jan-2012	9:42 PM	Property damage only (no	2	0	0	Head-on	Wet	Dark - lighted re	Rain		
	3063175	13-Jan-2012	12:30 PM	Non-fatal injury	2	2	0	Rear-end	Dry	Daylight	Cloudy		
	3066730	21-Feb-2012	12:19 PM	Non-fatal injury	2	1	0	Rear-end	Dry	Daylight	Clear/Clear		
	3090230	24-Feb-2012	7:13 PM	Property damage only (no	2	0	0	Head-on	Wet	Dark - lighted re	Rain		
	3091131	01-Mar-2012	9:35 PM	Property damage only (no	2	0	0	Rear-end	Snow	Dark - lighted re	Snow		
	3105753	27-Apr-2012	2:52 AM	Non-fatal injury	1	2	0	Single vehicle crash	Wet	Dark - lighted re	Rain		
	3114094	30-Mar-2012	4:37 PM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear		
	3222219	09-Jun-2012	2:33 PM	Property damage only (no	3	0	0	Rear-end	Dry	Daylight	Clear		
	3270683	24-Jul-2012	4:19 PM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear		
	3279002	14-Aug-2012	9:16 PM	Non-fatal injury	2	1	0	Angle	Dry	Dark - lighted re	Clear/Clear		
	3289518	01-Sep-2012	11:40 AM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear/Clear		
	3289651	19-Sep-2012	12:19 PM	Property damage only (no	2	0	0	Head-on	Dry	Daylight	Clear		
	3291152	27-Sep-2012	5:45 PM	Property damage only (no	2	0	0	Rear-end	Dry	Daylight	Clear		
	3293319	10-Oct-2012	12:45 PM	Non-fatal injury	2	1	0	Rear-end	Wet	Daylight	Cloudy/Rain		
	3333437	06-Nov-2012	4:00 PM	Property damage only (no	2	0	0	Sideswipe, same direc	Dry	Daylight	Clear		
	3339951	19-Oct-2012	6:43 AM	Property damage only (no	2	0	0	Angle	Wet	Dawn	Rain		
	3340621	14-Dec-2012	10:03 AM	Non-fatal injury	2	1	0	Angle	Dry	Daylight	Clear		
	3360617	25-Dec-2012	8:32 PM	Property damage only (no	2	0	0	Sideswipe, same direc	Dry	Dark - lighted re	Clear		
		03-Jun-2013	9:19 AM	Property damage only (no	2	0	0	Angle		Daylight	Rain/Cloudy		
	3603329	19-Sep-2013	4:12 PM	Property damage only (no	2	0	0	Sideswipe, same direc	Dry	Daylight	Clear		

### Crash Cluster 4 2011 to 2014

0.111.11.	01				<b>a.</b>	Total	Total		Road	A		Non	D.1
Collision	Crash				Number of	Nonfatal	Fatai		Surface	Ambient Light		Motorist	Bike_
Number	Number1	Crash Date1	Crash Time1	Crash Severity	Vehicles	Injury	Injury	Manner of Collision	Condition	Condition	Weather Condition	Туре	Ped
1	2750404	28-Jul-2011	00:00 AM	Not Reported	1	. 0	0	Single vehicle crash	Dry	Dark - lighted roadwa	Clear		
2	2833039	09-Nov-2011	8:17 PM	Property damage only (no	1	. 0	0	Single vehicle crash	Dry	Dark - lighted roadwa	Clear		
3	3360264	13-Dec-2012	5:08 PM	Property damage only (no	2	0	0	Rear-end	Dry	Dark - lighted roadwa	Clear		
4	3361576	01-Feb-2012	3:31 PM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear		
5	3361651	02-Nov-2011	7:50 AM	Non-fatal injury	1	. 1	0	Single vehicle crash	Dry	Daylight	Clear/Cloudy	P2:Pedalcycli	сус
6	3361712	25-Mar-2012	10:52 AM	Property damage only (no	2	0	0	Rear-end	Dry	Daylight	Clear/Cloudy		
7	3361918	07-Aug-2012	2:42 PM	Not Reported	2	0	0	Rear-end	Dry	Daylight	Clear		
8	3361926	31-Jan-2012	6:48 PM	Property damage only (no	3	0	0	Rear-end	Dry	Dark - lighted roadwa	Clear		
9	3362137	05-Mar-2012	6:53 PM	Property damage only (no	2	0	0	Single vehicle crash	Dry	Dark - lighted roadwa	Clear		
10	3733725	19-Dec-2013	5:19 PM	Property damage only (no	2	0	0	Angle	Dry	Dark - lighted roadwa	Clear		
11	3733735	18-Feb-2013	7:47 PM	Property damage only (no	2	0	0	Angle	Dry	Dark - lighted roadwa	Clear		
12	3822228	41782	2:47 PM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear/Cloudy		
13	3822331	41787	8:24 AM	Non-fatal injury	2	2	0	Angle	Wet	Daylight	Rain/Cloudy		
14	3893435	41847	4:36 AM	Property damage only (no	1	. 0	0	Single vehicle crash	Dry	Daylight	Clear		
15	3954558	41913	4:26 PM	Property damage only (no	2	0	0	Head-on	Wet	Daylight	Cloudy/Rain		
_	2832863	02-Nov-2011	7:50 AM	Non-fatal injury	1	1	0	Single vehicle crash	Dry	Daylight	Clear/Cloudy	P2:Pedalcycli	сус
	2919331	20-Dec-2011	5:24 PM	Property damage only (no	2	0	0	Rear-end	Dry	Dark - lighted roadwa	Clear		

# Crash Data Summary Table Loring Avenue, Salem MA 2010, 2011, 2012, 2013

Crash	Crash					Weather		, 2012, 2013				
		Crash Day	Time of Day	Manner of Collision	Light Condition	Condition	Road Surface	Driver Contributing Code		Ages		Comments
Ref#	m/d/y	o.uo Duy	· ······ c·· zuj	Type	Type	Type	Type	Type	D1	D2	D3	
	,			.,,,,,	.,,,,,,	.,,,,,	.,,,,,	Operating Vehicle in erratic, reckless,				
								careless, negligent, or aggressive				
1	1/10/10	Sunday	4:03 AM	Single Vehicle Crash	Dark - lighted roadway	Snow	Dry	manner	20			MV lost out of control and struck a hydrant and telephone pole
												· · ·
2	1/11/10	Monday	4:45 PM	Angle	Dark - lighted roadway	Cloudy	Dry	Other improper action	23	33		
												MV3(uninvolved) was traveling very slowly in front of MV1 and MV2. MV1
												and MV2 were both trying to pass MV3 and each claimed that the other MV
3		Friday	1:17 PM	Sideswipe, same direction	Daylight	Cloudy	Dry	Unknown	25			was at fault for crash. 219 Loring Avenue
4		Wednesday	12:18 PM	Rear-end	Daylight	Clear	Dry	Followed too closely	36	23		MV's stopped in traffic. 221 Loring Avenue
5		Friday	1:30 PM	Rear-end	Daylight	Rain	Wet	Followed too closely	33	38		MV's stopped in traffic. 223 Loring Avenue
6	4/30/10	Friday	11:32 AM	Angle	Daylight	Clear	Dry	Failed to yield to right of way	21	27		
												MV2 pulled out in front of MV1 in an attempt to reverse direction and collided
7	5/27/10	Thursday	12:33 PM	Angle	Daylight	Clear	Dry	Failed to yield to right of way	33	18		with MV1
								Failure to keep in proper lane or				MV1 in lane designated for a right hand turn, MV2 collided with MV1. 220
8	6/1/10	Tuesday	3:34 PM	Angle	Daylight	Rain	Wet	running off road	23	19		Loring Avenue
												MV's stopped in heavy traffic, MV was distracted by someone at the side of
9	6/18/10	Friday	4:11 PM	Rear-end	Daylight	Clear	Dry	Inattention	40	51		the road. 220 Loring Avenue
10		Monday	2:00 PM	Angle	Daylight	Clear	Dry	Failed to yield to right of way	22			256 Loring Avenue
11	11/1/10	Monday	9:20 AM	Rear-end	Daylight	Clear	Dry	Followed too closely	24	53		MV's stopped in traffic
						l		Failure to keep in proper lane or				L
12	12/6/10	Monday	1:20 AM	Head on	Dark - lighted roadway		Dry	running off road	27			MV struck flashing yellow traffic light and a telephone pole
13	12/20/10	Monday	2:59 PM	Single Vehicle Crash	Daylight	Snow	Snow	No Improper Driving	20			MV slid into the guardrail due to the inclement weather
	4/4/44	0.1.1.	40.00 PM	0:11/-101	D. P. L.	01		Failure to keep in proper lane or	07			NO
14	1/1/11	Saturday	12:20 PM	Single Vehicle Crash	Daylight	Clear	Dry	running off road	27			MV swerved in road striking a snowbank
15	2/3/11	Thursday	1:20 PM	Rear-end	Daylight	Clear	Dry	Failure to keep in proper lane or running off road	23			MV struck parked car
15	2/3/11	Thursday	1.20 FIVI	Real-ellu	Daylight	Clear	Diy	Turning on road	23			INIV Struck parked car
16	2/13/11	Sunday	6:00 PM	Angle	Dark - lighted roadway	Clear	Dry	Unknown	78	32		MV turning into driveway at 462 Loring Avenue
	2, 10, 11		0.00				,	Operating Vehicle in erratic, reckless,				int turning into arroway at 102 25thing / troilide
								careless, negligent, or aggressive				Operator claims he saw a pedestrian in the middle of the road which caused
17	3/10/11	Thursday	12:57 AM	Single Vehicle Crash	Dark - lighted roadway	Clear	Dry	manner	93			him to swerve to the right and strike a tree. Operator charged with OUI
	0, 10, 11	maroday	12.07 7	engle veniele eraen	Dan ignouroadia)	Oloui	5.,		- 00			MV crossed into the opposite lane of traffic, drove over the curb and struck
18	4/2/11	Saturday	2:25 AM	Single Vehicle Crash	Dark - lighted roadway	Clear	Wet	No Improper Driving	18			house at 221 Loring Ave, charged for OUI
19		Friday	8:30 AM	Rear-end	Daylight	Clear	Dry	Unknown	65	49		MV stopped at traffic light was rear-ended
20		Tuesday	10:30 AM	Rear-end	Daylight	Cloudy	Dry	Followed too closely	65			MV stopped for a turkey in the road was rear-ended. 452 Loring Avenue
						,	,	- construction of the control of the		<u> </u>		
21	6/16/11	Thursday	3:30 PM	Single Vehicle Crash	Daylight	Clear	Dry	Illness	46			Operator may have blacked out and and struck a fire hydrant and utility pole
				g.c · c · · · · · · · · · · · · · · · · ·			,	Operating Vehicle in erratic, reckless,				
								careless, negligent, or aggressive				MV struck a pedestrian who was walking along the solid double yellow line,
22	6/25/11	Saturday	1:05 AM	Single Vehicle Crash	Dark - lighted roadway	Rain	Wet	manner	21			MV charged with OUI
												Operator of MV2 was distracted and rear-ended MV1 as traffic slowed. 430
23	7/2/11	Saturday	10:59 AM	Rear-end	Daylight	Clear	Dry	Distracted	33	28		Loring Avenue
24	7/13/11	Wednesday	3:00 PM	Rear-end	Daylight	Clear	Dry	Followed too closely	53	41		MV's stopped in traffic. 229 Loring Avenue
25		Wednesday	3:02 PM	Sideswipe, opposite direction	Daylight	Clear	Dry	Made an improper turn	45	35		MV attempted to make an illegal U-Turn. 270 Loring Avenue
26	9/19/11	Monday	12:04 PM	Rear-end	Daylight	Clear	Dry	Followed too closely	24	26		MV's stopped in traffic at Loring Ave & Harrison Rd
			L							l		
27		Sunday	2:27 AM	Single Vehicle Crash	Dark - lighted roadway		Dry	Exceeded authorized speed limit	28			MV swerved to the right and struck standing traffic light tower
28	10/14/11	Friday	11:45 AM	Angle	Daylight	Rain	Wet	Failed to yield to right of way	23	23	<u> </u>	Entering Salem State University South Campus
						ĺ		Operating Vehicle in erratic, reckless,			1	MO transfer at a kink ant of annual as 1811 as a Laurent Laure
	44/40/44	0.1.1.	40.00 514	S	D. F.L.			careless, negligent, or aggressive			1	MC traveling at a high rate of speed and "doing wheelies", lost control and
29	11/19/11	Saturday	12:30 PM	Single Vehicle Crash	Daylight	Clear	Dry	manner	24	<u> </u>	<u> </u>	crashed onto the pavement
	4/40/46		44.04.504			l	l					MV traveling at a high rate of speed crossed over the solid white lines
30	1/12/12	Thursday	11:24 PM	Rear-end	Dark - lighted roadway		Wet	Exceeded authorized speed limit	22	<u> </u>	<u> </u>	striking parked MV. 208 Loring Avenue
31	2/19/12	Sunday	11:30 AM	Rear-end	Daylight	Clear	Dry	No Improper Driving	62	21	36	MV's stopped in traffic
			1	I		L.				l .		MV2 was following a friend when turning out of Lincoln Rd. and did not see
32	4/6/12	Friday	11:16 PM	Angle	Dark - lighted roadway	Clear	Dry	Inattention	18	18		MV1. 206 Loring Avenue

# Crash Data Summary Table Loring Avenue, Salem MA 2010, 2011, 2012, 2013

Crash	Crash					Weather									
Diagram		Crash Day	Time of Day	Manner of Collision	Light Condition	Condition	Road Surface	Driver Contributing Code				Comments			
Ref#	m/d/y			Туре	Туре	Type	Type	Туре	D1 D2 D3		D3				
												MV2 attempted to make an illegal U-Turn on Loring Avenue from the SB			
												lane into the NB lane and was hit by MV1 who was traveling NB on Loring			
33	5/16/12	Wednesday	5:41 PM	Head on	Daylight	Clear	Dry	Made an improper turn	23	23 68		Avenue; No injuries reported.			
								Operating Vehicle in erratic, reckless,							
							L	careless, negligent, or aggressive							
34	6/22/12		3:13 AM	Single Vehicle Crash	Dark - lighted roadway		Dry	manner	24			MV traveling at a high rate of speed crashed into a utility pole			
35	7/25/12	Wednesday	10:00 AM	Rear-end	Daylight	Clear	Dry	No Improper Driving	48	47	31	1 MV's stopped in traffic			
												Bicylcist operating outbound on Loring Ave (Inbound Breakdown lane) & hit			
36		Friday	9:55 AM	Angle	,	Clear	Dry	No Improper Driving	43	, 00		MV.			
37		Sunday	2:01 PM	Rear-end	., 5	Clear	Dry	Followed too closely	71			MV stopped for pedestrians crossing the street. 450 Loring Avenue			
38		Friday	9:00 AM	Angle	, 0	Clear	Dry	Failed to yield to right of way	28	47					
39	9/28/12	Friday	10:35 AM	Rear-end	Daylight	Rain	Wet	Followed too closely	36	39		Stopped for traffic light			
								Failure to keep in proper lane or				MV traveling at high rate of speed, hitting the curb and striking utlity pole an			
40		Sunday	9:23 PM	Single Vehicle Crash	Dark - lighted roadway	Rain	Wet	running off road	19			then guardrail			
41	2/2/13	Saturday	9:30 AM	Rear-end	Daylight	Clear	Dry	Followed too closely	38	42		MV's stopped in traffic			
42	2/5/13	Tuesday	5:26 PM	Rear-end	Dusk	Clear	Dry	Followed too closely	52	30		MV's stopped in traffic. 214 Loring Avenue			
1															
43	3/1/13	Friday	3:20 PM	Single Vehicle Crash	Daylight	Cloudy	Dry	Exceeded authorized speed limit	59			MV took his eyes off the road and drove up on the curb, struck a hydrant			
												MV failed to stay in marked lanes and struck legally parked MV's. 220 Loring			
44		Monday	11:33 AM	Angle		Clear	Dry	No Improper Driving	21			Avenue			
45	3/26/13	Tuesday	6:49 AM	Angle	Daylight	Clear	Dry	No Improper Driving	28	51		MV backing out of a driveway at 223 Loring Avenue			
46	4/23/13	Tuesday	1:38 PM	Angle	Daylight	Cloudy	Wet	Failed to yield to right of way	59	24		Turning out of the Salem State University Driveway			
47	5/6/13	Monday	11:12 AM	Rear-end	Daylight	Cloudy	Dry	Followed too closely	45	87		MV's stopped for traffic. 215 Loring Avenue			
												Hit & run crash; Pedestrian struck & killed while crossing the street in front of			
48	6/29/13	Saturday	9:11 AM	Single Vehicle Crash	Daylight	Cloudy	Dry	Unknown	unk			her residence			

Summary based on Crash Reports obtained from the Salem Police Department

### Crash Cluster 6 2011 to 2014

,													
1						Total	Total		Road			Non	
Cluster	Crash		Crash		Number of	Nonfatal	Fatal		Surface	Ambient Light	Weather	Motorist	Bike_
Number	Number	Crash Date1	Time1	Crash Severity	Vehicles	Injuries	injuries	Manner of Collision	Condition	Condition	Condition	Туре	Ped
1	3549768	26-Jul-2013	8:15 AM	Property damage only (n	2	0	0	Rear-end	Wet	Daylight	Cloudy/Rain		
2	3380037	08-Feb-2013		Property damage only (n		0	0	Rear-end	Dry	Dusk	Clear		
3	3563584	25-Jun-2013		Property damage only (n		0	0		Dry	Daylight	Clear/Clear		
4	3740408	27-Nov-2013		Property damage only (n		0	0		Wet	Daylight	Cloudy/Rain		
5	3743294	31-Jan-2014		Property damage only (n		0	0		Dry	Daylight	Clear/Clear		
6	3803562	21-Mar-2014		Property damage only (n		0	0		Dry	Daylight	Clear/Clear		
7	3803580	31-Mar-2014		Property damage only (n		0	0		Wet	Daylight	Cloudy/Cloudy		
8	3943753	12-Jul-2014		Property damage only (n		0	0		Dry	Daylight	Clear/Clear		
9	3980617	6-Nov-2014		Non-fatal injury	3	1	0		Wet	Dusk	Cloudy/Rain		
10	3579631	08-Aug-2013		Property damage only (n		0	0		Dry	Daylight	Clear		+
11	3549774	01-Aug-2013		Property damage only (n		0	0	_	Dry	Daylight	Clear/Clear		
12	3430021	06-May-2013		Property damage only (n		0	0		Dry	Daylight	Cloudy		
		·				0	0		-		,		
13	3422444	26-Mar-2013		Property damage only (n		~	_	-	Dry	Daylight Tool too durant	Clear		+
14	3740427	23-Nov-2013		Property damage only (n		0	0		Wet	Dark - lighted roadway	Rain/Rain		
15	3422214	27-Mar-2013		Non-fatal injury	2	1	0		Dry	Daylight	Clear		
16	3647352	25-Oct-2013		Property damage only (n		0	0		Dry	Daylight	Clear/Clear		
17	3608241	10-Oct-2013		Property damage only (n		0	0		Dry	Daylight	Clear/Clear		
18	3689058	29-Aug-2013		Not Reported	2	0	0	Sideswipe, opposite dire		Dark - lighted roadway	Clear/Clear		
19	3603303	05-Sep-2013		Property damage only (no		0	0	Rear-end	Wet	Daylight	Cloudy/Rain		
20	3965585	04-Sep-2013		Property damage only (no		0	0		Dry	Daylight	Clear		
21	3743293	31-Jan-2014		Property damage only (n		0	0		Dry	Daylight	Cloudy/Cloudy		
22	3803520	25-Feb-2014		Property damage only (n	2	0	0		Dry	Daylight	Clear/Clear		
23	3803604	11-Apr-2014		Non-fatal injury	5	1	0		Dry	Dark - lighted roadway	Clear/Clear		
24	3865087	03-Jun-2014		Property damage only (n		0	0		Dry	Daylight	Clear/Clear		
25	3965675	30-Sep-2014		Property damage only (n	2	0	0		Wet	Daylight	Cloudy/Rain		
26	3995707	26-Nov-2014	7:01 PM	Property damage only (n	1	0	0	Single vehicle crash	Wet	Dark - lighted roadway	Rain/Sleet, hail (fre	ezing rain c	ped
27	4000270	29-Dec-2014		Non-fatal injury	3	1	0	Rear-end	Dry	Daylight	Clear/Clear		
	2721398	08-Apr-2011	8:30 AM	Non-fatal injury	2	1	0	Rear-end	Dry	Daylight	Clear		
	3118258	11-Apr-2011	3:07 AM	Property damage only (n	1	0	0	Single vehicle crash	Wet	Dark - lighted roadway	Clear		
	2743363	25-Jun-2011	1:05 AM	Property damage only (n	1	0	0	Single vehicle crash	Wet	Dark - lighted roadway	Rain	P5:Pedesti	riped
	2752171	13-Jul-2011	3:00 PM	Non-fatal injury	2	1	0	Rear-end	Dry	Daylight	Clear		
	2789096	19-Sep-2011	12:04 PM	Non-fatal injury	2	1	0	Rear-end	Dry	Daylight	Clear		
	2791854	25-Sep-2011	2:27 AM	Property damage only (n	1	0	0	Single vehicle crash	Dry	Dark - lighted roadway	Clear		
	2791290	30-Sep-2011	2:14 PM	Non-fatal injury	2	2	0	Rear-end	Dry	Daylight	Clear/Clear		
	2853309	14-Oct-2011	11:45 AM	Non-fatal injury	3	2	0	Angle	Wet	Daylight	Cloudy/Rain		
	2990334	12-Jan-2012	11:24 PM	Property damage only (n	2	0	0	Rear-end	Wet	Dark - lighted roadway	Cloudy		
	3089895	01-Feb-2012	3:56 PM	Non-fatal injury	2	1	0	Rear-end	Dry	Daylight	Clear		
	3066830	19-Feb-2012		Property damage only (n	3	0	0		Dry	Daylight	Clear		1
	3066441	24-Feb-2012		Property damage only (n		0	0		Wet	Daylight	Cloudy/Rain		
	3102759	06-Apr-2012		Property damage only (n		0	0		Dry	Dark - lighted roadway	Clear		
	3220334	22-Jun-2012		Property damage only (n		0	0		Dry	Dark - lighted roadway	Clear		
	3291149	28-Sep-2012		Non-fatal injury	2	1	0		Wet	Daylight	Rain		
	3357052	15-Dec-2012		Property damage only (n		0	0		Dry	Daylight	Clear		
	3389742	04-Mar-2013		Property damage only (n		0	0		Dry	Daylight	Clear		

### Crash Cluster 6 2011 to 2014

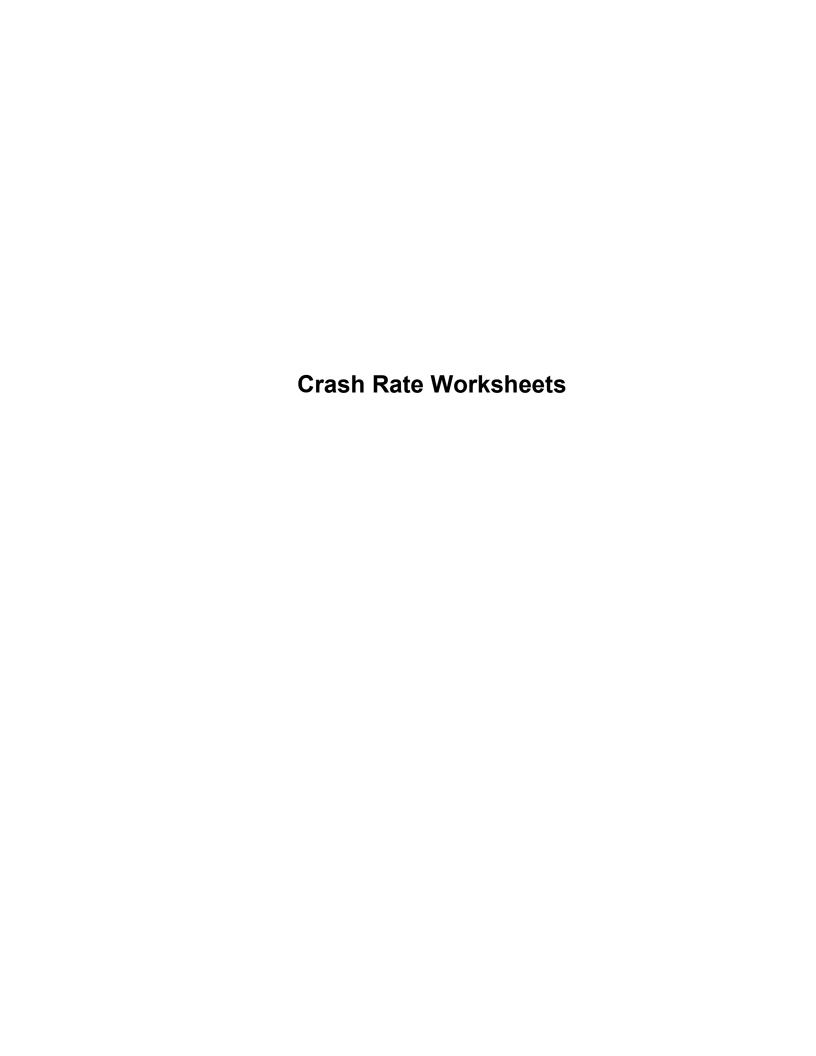
		2011 to 2014											
						Total	Total		Road			Non	
Cluster	Crash		Crash		Number of	Nonfatal	Fatal		Surface	Ambient Light	Weather	Motorist	Bike_
Number	Number	Crash Date1	Time1	Crash Severity	Vehicles	Injuries	injuries	Manner of Collision	Condition	Condition	Condition	Туре	Ped
	3865880	24-Apr-2014	11:00 PM	Non-fatal injury	2	1	0	Rear-end	Dry	Dark - lighted roadway	Clear		
	2709094	28-Jan-2011	4:05 PM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear		
	2707350	08-Feb-2011	4:25 PM	Property damage only (no	3	0	0	Rear-end	Wet	Daylight	Clear		
	2716541	31-Mar-2011	8:10 PM	Property damage only (no	2	0	0	Rear-end	Wet	Dark - lighted roadway	Rain/Sleet, hail (fre	ezing rain o	r drizzle)
	3154064	01-Jun-2011	12:47 PM	Fatal injury	2	0	1	Rear-end	Dry	Daylight	Clear		
	2765632	14-Aug-2011	7:25 PM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear		
	2764265	01-Sep-2011	3:35 PM	Property damage only (no	3	0	0	Rear-end	Dry	Daylight	Clear		
	2763559	09-Sep-2011	5:20 PM	Non-fatal injury	3	1	0	Rear-end	Dry	Daylight	Clear		
	2789091	20-Sep-2011	3:00 PM	Property damage only (no	2	0	0	Single vehicle crash	Wet	Daylight	Cloudy/Rain		
	2791266	23-Sep-2011	2:32 PM	Property damage only (no	3	0	0	Rear-end	Wet	Daylight	Rain		
	3297927	19-Oct-2012	2:23 PM	Property damage only (no	2	0	0	Sideswipe, same directi	Dry	Daylight	Cloudy/Cloudy		
	3045263	03-Feb-2012	1:50 PM	Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear/Clear		
	3116477	21-Mar-2012	3:07 AM	Property damage only (no	2	0	0	Rear-end	Dry	Daylight	Clear		
	3068433	18-Apr-2012	1:01 PM	Non-fatal injury	2	1	0	Angle	Dry	Daylight	Clear		
	3245618	03-May-2012	10:40 AM	Non-fatal injury	2	1	0	Rear-end	Wet	Daylight	Cloudy		
	3270668	30-Jul-2012	3:00 AM	Non-fatal injury	2	1	0	Angle	Dry	Dark - lighted roadway	Clear		
	3254625	23-Aug-2012	2:30 PM	Non-fatal injury	2	1	0	Angle	Dry	Daylight	Clear		
	3289842	07-Sep-2012	2:34 AM	Property damage only (no	2	0	0	Sideswipe, same directi	Dry	Dark - lighted roadway	Clear		
	3293315	10-Oct-2012	4:20 PM	Property damage only (no	2	0	0	Rear-end	Wet	Daylight	Cloudy/Rain		
	3333937	13-Nov-2012	8:15 AM	Property damage only (no	2	0	0	Rear-end	Wet	Daylight	Cloudy/Rain		
	3360625	31-Dec-2012	6:00 PM	Property damage only (no	2	0	0	Sideswipe, same directi	Wet	Dark - lighted roadway	Clear		
	3420013	20-Mar-2013	4:20 PM	Property damage only (no	2	0	0	Rear-end	Dry	Daylight	Clear		
	3430829	29-Apr-2013	5:00 PM	Property damage only (no	3	0	0	Rear-end	Dry	Dusk	Clear/Clear		
	3689058	29-Aug-2013	1:24 AM	Not Reported	2	0	0	Sideswipe, opposite dire	Dry	Dark - lighted roadway	Clear/Clear		

# Crash Cluster 7 2011 to 2014

								2011 to 2014	1		T		
	_					Total	Total				_		
Collision	Crash		Crash		Number of		Fatal				Weather	Non Motorist	
Number	Number	Crash Date1		Crash Severity	Vehicles	Injury	Injury	Manner of Collision		Ambient Light Conditions		Crash	Ped
1	2949506	03-Jan-2011		Non-fatal injury	3	2	0	Head-on	Dry	Dark - lighted roadway	Clear		
2	2949540	01-Feb-2011		Non-fatal injury	2	1	0	Rear-end	Snow	, , ,	Snow		
3	2949638	05-Apr-2011		Non-fatal injury	3	2	0	Rear-end	Wet	, ,	Clear		
4	2949681	09-Sep-2011		Not Reported	3	0	0	Rear-end	Dry	<u> </u>	Clear		
5	2949720	21-Dec-2011		Property damage only (no		0	0	Angle	Wet	, ,	Rain/Cloudy		
6	3340481	14-Oct-2012		Non-fatal injury	3	2	0	Angle	Dry	<u> </u>	Clear		
7	3340597	06-Aug-2012		Not Reported	3	0	0	Rear-end	Dry	Daylight	Clear		
8	3340485	21-Oct-2012		Property damage only (no		0	0	Not reported	Dry	, · ·	Clear		
9	3340497	14-Nov-2012		Non-fatal injury	3	1	0	Rear-end	Dry	, ,	Clear		
10	3663437	24-Sep-2013		Not Reported	3	0	0	Not reported	Not reported	·	Not Reported		
11	3745230	23-Dec-2013		Non-fatal injury	2	1	0	Rear-end	Wet		Rain/Cloudy		
12	3352276	01-Jan-2013		Property damage only (no	1	0	0	Single vehicle crash	Dry	, , ,	Clear		
13	3590836	16-Jul-2013		Non-fatal injury	1	1	0	Angle	Dry	, ,	Clear	P3:Skater	ped
14	3590844	24-Aug-2013		Non-fatal injury	3	1	0	Rear-end	Dry	, , ,	Clear		
15	3591009	02-Jul-2013		Non-fatal injury	2	2	0	Angle	Dry		Cloudy		
16	3745243	15-Jan-2014	11:00 AM	Non-fatal injury	2	1	0	Angle	Wet	Daylight	Clear/Clear		
17	3745245	22-Jan-2014		Property damage only (no	2	0	0	Sideswipe, same direction	Dry		Clear		
18	3928070	20-May-2014	8:43 AM	Non-fatal injury	2	1	0	Angle	Dry	, ,	Clear		
19	3928377	23-Jun-2014	6:41 AM	Property damage only (no	2	0	0	Rear-end	Dry	Daylight	Clear		
20	3928084	14-Jul-2014	4:44 PM	Non-fatal injury	2	1	0	Angle	Dry	Daylight	Clear		
21	3928095	13-Aug-2014	7:13 AM	Not Reported	2	0	0	Angle	Wet	Daylight	Rain/Cloudy		
22	3928131	17-Feb-2014	1:39 AM	Property damage only (no	2	0	0	Not reported	Not reported	Dark - lighted roadway	Clear		
23	3928143	22-Mar-2014	1:20 PM	Non-fatal injury	2	1	0	Rear-end	Dry	Daylight	Cloudy		
24	3928151	10-May-2014	3:41 AM	Non-fatal injury	1	1	0	Single vehicle crash	Dry	Dark - roadway not lighted	Cloudy		
25	4003338	26-Nov-2014	11:56 AM	Property damage only (no	2	0	0	Angle	Wet	Daylight	Rain/Cloudy		
26	4003336	24-Nov-2014	9:33 AM	Non-fatal injury	2	1	0	Angle	Wet	Daylight	Rain/Cloudy		
	2949659	27-Jun-2011	7:02 PM	Non-fatal injury	2	2	0	Rear-end	Dry	Daylight	Clear		
	2949639	15-Apr-2011	10:23 AM	Non-fatal injury	2	1	0	Sideswipe, same direction	Dry	Daylight	Clear/Clear		
	2949594	15-Dec-2011	3:30 PM	Property damage only (no	2	0	0	Angle	Wet	Daylight	Cloudy		
	2949635	27-Mar-2011	6:55 AM	Non-fatal injury	1	1	0	Single vehicle crash	Dry	Dawn	Clear		
	2949703	28-Oct-2011	4:56 PM	Property damage only (no	3	0	0	Rear-end	Dry	Daylight	Clear		
	2949555	30-Apr-2011	12:58 PM	Non-fatal injury	1	1	0	Single vehicle crash	Dry	Daylight	Cloudy	P2:Pedestrian	ped
	2949597	23-Dec-2011	3:54 PM	Non-fatal injury	2	1	0	Rear-end	Dry	Dusk	Cloudy		
	3348276	08-Oct-2011	4:19 PM	Property damage only (no	2	0	0	Rear-end	Dry	Daylight	Clear/Clear		
	2949588	15-Nov-2011	7:18 PM	Non-fatal injury	2	2	0	Angle	Dry	Dark - lighted roadway	Cloudy		
	2949721	23-Dec-2011	3:08 PM	Non-fatal injury	2	2	0	Rear-end	Dry	Daylight	Clear/Clear		
	2949646	21-May-2011	1:26 PM	Non-fatal injury	2	1	0	Rear-end	Dry		Clear		
	3340612	28-Sep-2012	5:38 PM	Non-fatal injury	2	2	0	Rear-end	Wet		Rain/Cloudy		
	3340617	06-Dec-2012		Unknown	2	0	0	Sideswipe, same direction	Dry	Dark - lighted roadway	Clear		
	3340596	01-Aug-2012		Not Reported	2	0	0	Not reported	Not reported	•	Not Reported		
	3340601	15-Aug-2012		Not Reported	2	0	0	Rear-end	Wet	·	Cloudy		
	3340857	09-Aug-2012		Non-fatal injury	3	2	0	Sideswipe, opposite direction	Dry		Clear		
	3340600	11-Aug-2012		Non-fatal injury	2	6	0	Rear-end	Dry	, ,	Clear		
	3590787	26-Jul-2013		Non-fatal injury	2	1	0	Rear-end	Wet	, ,	Cloudy/Rain		

# Crash Cluster 7 2011 to 2014

						Total	Total						
Collision	Crash		Crash		Number of	Nonfatal	Fatal				Weather	Non Motorist	Bike_
Number	Number	Crash Date1	Time1	Crash Severity	Vehicles	Injury	Injury	Manner of Collision	Road Surface	Ambient Light Conditions	Conditions	Crash	Ped
	3591133	20-Aug-2013	2:36 PM	Non-fatal injury	2	3	0	Angle	Dry	Daylight	Clear/Clear		
	3481865	26-May-2013	3:15 PM	Non-fatal injury	3	2	0	Rear-end	Dry	Daylight	Clear		
	3391557	22-Mar-2013	1:36 PM	Non-fatal injury	2	1	0	Rear-end	Dry	Daylight	Clear		
	3590763	09-Jun-2013	11:11 AM	Non-fatal injury	2	1	0	Rear-end	Dry	Daylight	Clear		
	4003335	22-Nov-2014	6:31 PM	Non-fatal injury	2	1	0	Rear-end	Dry	Dark - lighted roadway	Clear		
	4003309	23-Sep-2014	9:31 AM	Property damage only (no	2	0	0	Rear-end	Dry	Daylight	Clear/Clear		
	3928098	25-Aug-2014	9:58 AM	Non-fatal injury	2	1	0	Rear-end	Dry	Daylight	Clear		





CITY/TOWN : _		Swampscott			COUNT DA	TE:	May-16
DISTRICT :	4						
			~ SEGMEN	ΓDATA ~			
ROADWAY NAM	ЛЕ:	Route 1A (Clu	ster 1)				
START POINT:\	√innin Liqı	uor/Whole Foo	ds Driveway				
END POINT: 5	Santander	Driveway					
FUNCTIONAL C	LASSIFIC	CATION OF RO	DADWAY: <u>I</u>	Principal Arte	erial		
_							
F	ROADWA	Y DIAGRAM (I	LABEL ROAI	DWAY AND	CROSS STE	REETS)	
		`				,	
North							
l	Vinnin Loa	luor Driveway				Santander Driv	/ewav
1		,,				2	<b>,</b>
		Δ\	/ERAGE DAI	I V TDAEEI	`		
			LENGTH IN				
	AVE	ERAGE DAILY					
			-	- ( )			
TOTAL # OF CF	RASHES:	73	# OF YEARS :	4		GE # OF PER YEAR (	18.25
			TEARS:		A	\ ):	
CRASH RA		7.33	RATE =		( A * 1,	000,000 ) ' * 365 )	
CALCULAT	ION :	7.00			(L* V	/ * 365 )	
Comments :							
Project Title & Da	ate:	Route 1A Vinr	nin Square Pri	ority Corrido	or Study		



CITY/TOWN :	Salem/Swam	oscott	COUNT DA	ATE :	May-16
DISTRICT:	4				
		~ SEGMENT I	DATA ~		
ROADWAY NAME:	Essex Street	(Cluster 2)			
START POINT: Swa	ampscott Mall Drivew	ay			
END POINT: Car	ol Way				
FUNCTIONAL CLA	SSIFICATION OF RO	DADWAY: <u>Mi</u>	nor Arterial		
RO	ADWAY DIAGRAM (	LABEL ROADV	VAY AND CROSS ST	REETS)	
North					
	ampscott Mall Drivew	ay		Carol Way	
1				2	
	A	/ERAGE DAIL\	TRAFFIC	7	
	SEGMENT	LENGTH IN M	ILES ( <b>L</b> ): <b>0.3</b>		
	AVERAGE DAILY	TRAFFIC VOL	JME ( <b>V</b> ): 16,900		
TOTAL # OF CRAS	SHES: 22	# OF YEARS :	4 CRASHES	AGE # OF S PER YEAR ( A):	5.50
CRASH RATE	7 9 9	RATE =	( A * ^	1,000,000 ) V * 365 )	
Comments :					
Project Title & Date	: Route 1A Vinr	nin Square Prior	ity Corridor Study		



CITY/TOWN :		Salem/Swamp	oscott		COUNT DA	TE:	May-16
DISTRICT :	4						
			~ SEGMENT	DATA ~			***************************************
ROADWAY NAM	E:	Vinnin Square	(Cluster 3)				
START POINT: S	antander	Bank					
END POINT: M	laple Ave	enue					
FUNCTIONAL CL	ASSIFIC	CATION OF RO	DADWAY: <u>Pr</u>	rincipal Arto	erial		
 R	OADWA	Y DIAGRAM (	LABEL ROAD	WAY AND	CROSS STE	REETS)	
North							
<u>S</u> i	antander	· Bank				Maple Avenue 2	
			/ERAGE DAIL			I	
	AVE		LENGTH IN M				
TOTAL # OF CRA	ASHES:	74	# OF YEARS :	4	CRASHES	GE#OF PER YEAR (	18.50
CRASH RA CALCULATIO		4.33	RATE =		( A * 1, ( L * V	000,000 ) ′ * 365 )	
Comments :							
Project Title & Da	te:	Route 1A Vinr	nin Square Prio	rity Corrido	or Study		



CITY/TOWN :		Marblehead			COUNT DA	TE:	May-16
DISTRICT:	4	<u>-</u>					
			~ SEGMEN	T DATA ~			
ROADWAY NA	ME:	Tedesco Stre	et(Cluster 4)				
START POINT:	Brookhous	se Drive					
END POINT:	leggs Hill	Road					
FUNCTIONAL (	CLASSIFIC	CATION OF RO	DADWAY:	Principal Arte	erial		
	ROADWA	Y DIAGRAM (	LABEL ROA	DWAY AND	CROSS STE	REETS)	
North							
	•						
	Brookhous	se Drive				Leggs Hill Road	d
1						2	
		Δ\	VERAGE DA	II Y TRAFFIC	<b>:</b>		
			LENGTH IN			]	
	AVE	ERAGE DAILY	TRAFFIC VO	DLUME ( <b>V</b> ):	15,000		
			ſ		AVERA	GE#OF	
TOTAL # OF C	RASHES:	17	# OF YEARS :	4	CRASHES	PER YEAR (	4.25
			L				
CRASH R		3.38	RATE =		(A*1,	000,000 ) / * 365 )	
Comments : Project Title & D	Date:	Route 1A Vini	nin Square Pr	iority Corrido	r Studv		
,				<i>j</i>	, , , , , , , , , , , , , , , , , , ,		



CITY/TOWN:		Salem			COUNT DA	TE:	May-16
DISTRICT:	4	_					
			~ SEGMEN	T DATA ~			
ROADWAY NA	ME:	Route 1A-Lori	ng Avenue (0	Cluster 5)			
START POINT	: Intervale F	Road					
END POINT:	Leggs Hill	Road					
FUNCTIONAL	CLASSIFIC	CATION OF RO	ADWAY:	Principal Arte	erial		
	ROADWA	Y DIAGRAM (I	ABEL ROA	DWAY AND	CROSS STE	REETS)	
						,	
North							
	1						
	Intervale f	Road				Leggs Hill Road	
1						2	
		Δ\	/ERAGE DA	II V TDAEEI			
			LENGTH IN				
	AVI	ERAGE DAILY					
					<u> </u>		
TOTAL # OF C	CRASHES:	25	# OF YEARS :	4		GE # OF PER YEAR (	6.25
			1 LANS .			<b></b>	
CRASH R		2.57	RATE =		(A * 1,	000,000 ) / * 365 )	
CALCULA	TION :	2.01			(L* V	/ ^ 365 )	
Comments :							
Project Title & [	Date:	Route 1A Vinn	nin Square Pr	iority Corrido	or Study		



CITY/TOWN:		Salem			COUNT DA	TE:	May-16
DISTRICT:	4	_					
		***************************************	~ SEGMEN	ΓDATA ~			***************************************
ROADWAY NA	ME:	Route 1A-Lori	ng Avenue (C	Cluster 6)			
START POINT	:Lincoln R	oad					
END POINT:	Sumner F	Road					
FUNCTIONAL	CLASSIFIC	CATION OF RO	DADWAY: <u>I</u>	Principal Arte	erial		
	ROADWA	Y DIAGRAM (I	LABEL ROAI	OWAY AND	CROSS STR	REETS)	
		(					
North							
	<u>1</u> 1						
	Lincoln Ro	oad				Sumner Road	
1						2	
		Δ1	/ERAGE DAI	I V TDAEEI			
			LENGTH IN				
	AVI	ERAGE DAILY					
			Г	. ,	•		
TOTAL # OF C	RASHES:	68	# OF YEARS :	4	CRASHES		17.00
	***************************************		12/10 · L		A	·):	
CRASH R		6.13	RATE =		( A * 1,	000,000 ) ' * 365 )	
CALCULA	HON:	0.10			(L^V	305)	
Comments :		_ ,					
Project Title & [	Date:	Route 1A Vinr	nin Square Pri	ority Corrido	or Study		



CITY/TOWN :	Swampscott			COUNT DATE	E:	May-16							
DISTRICT: 4	_												
		~ SEGMENT	DATA ~										
ROADWAY NAME:	Route 1A (Clu	ster 7)											
START POINT: Ellis Road	ΓART POINT: Ellis Road												
END POINT: Longwoo	d Drive				_								
FUNCTIONAL CLASSIFI	CATION OF RO	DADWAY: <u>P</u>	rincipal Art	erial									
ROADW	AY DIAGRAM (	LABEL ROAD	WAY AND	CROSS STRE	ETS)								
North													
Ellis Road	d			L	ongwood dribe								
1					2								
	A\	/ERAGE DAIL	Y TRAFFI	C									
	SEGMENT	LENGTH IN N	MILES ( L )	0.85									
AV	ERAGE DAILY	TRAFFIC VOL	LUME ( <b>V</b> ):	19,500									
TOTAL # OF CRASHES	51	# OF YEARS :	4	AVERAG CRASHES PI <b>A</b> )	ER YEAR (	12.75							
CRASH RATE CALCULATION :	2.11	RATE =	_	( A * 1,00 ( L * V *	<u>0,000 )</u> 365 )								
Comments :													
Project Title & Date:	Route 1A Vinr	nin Square Prio	ority Corrido	or Study									

# **APPENDIX F**

Level of Service (LOS) Analysis Existing Conditions

#610

626

1216

0

0

0

0.63

	<b>4</b>	$\mathbf{x}$	Ì	~	×	₹	ን	×	~	Ĺ	×	*
Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		₩			4			44			4	
Traffic Volume (vph)	22	45	4	15	44	7	5	417	8	5	659	31
Future Volume (vph)	22	45	4	15	44	7	5	417	8	5	659	31
Satd. Flow (prot)	0	1761	0	0	1754	0	0	1793	0	0	1790	0
Flt Permitted		0.869			0.901			0.993			0.997	
Satd. Flow (perm)	0	1554	0	0	1598	0	0	1783	0	0	1784	0
Satd. Flow (RTOR)		3			7			2			4	
Lane Group Flow (vph)	0	77	0	0	72	0	0	472	0	0	763	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		6			2			4			8	
Permitted Phases	6			2			4			8		
Total Split (s)	16.0	16.0		16.0	16.0		36.0	36.0		36.0	36.0	
Total Lost Time (s)		6.0			6.0			6.0			6.0	
Act Effct Green (s)		8.4			8.3			37.0			37.0	
Actuated g/C Ratio		0.15			0.15			0.68			0.68	
v/c Ratio		0.32			0.29			0.39			0.63	
Control Delay		29.3			27.5			13.1			19.7	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		29.3			27.5			13.1			19.7	
LOS		С			С			В			В	
Approach Delay		29.3			27.5			13.1			19.7	
Approach LOS		С			С			В			В	
Queue Length 50th (ft)		20			17			61			126	

#### Intersection Summary

Queue Length 95th (ft)

Internal Link Dist (ft)

Turn Bay Length (ft)
Base Capacity (vph)

Starvation Cap Reductn

Spillback Cap Reductn

Storage Cap Reductn

Cycle Length: 75

Reduced v/c Ratio

Actuated Cycle Length: 54.3

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.63 Intersection Signal Delay: 18.4

Intersection Capacity Utilization 55.3%

Intersection LOS: B ICU Level of Service B

65

218

332

0

0

0

0.22

275

904

1214

0

0

0

0.39

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

71

155

320

0

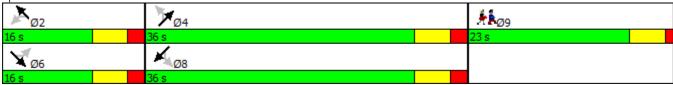
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0

0.24

Queue shown is maximum after two cycles.

Splits and Phases: 1: Paradise Road & Ellis Rd



	<b>F</b>	₹	×	~	Ĺ	×
Lane Group	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	7	7	f)		¥	<b>*</b>
Traffic Volume (vph)	11	28	698	3	37	876
Future Volume (vph)	11	28	698	3	37	876
Satd. Flow (prot)	1496	1338	1510	0	1496	1511
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1450	1338	1510	0	1484	1511
Satd. Flow (RTOR)						
Lane Group Flow (vph)	11	29	723	0	38	903
Turn Type	Prot	pt+ov	NA		Prot	NA
Protected Phases	2	2 3	4		3	8
Permitted Phases						
Total Split (s)	24.0		49.0		12.0	61.0
Total Lost Time (s)	5.0		5.0		6.0	5.0
Act Effct Green (s)	19.0	31.0	48.8		6.0	56.0
Actuated g/C Ratio	0.22	0.36	0.57		0.07	0.66
v/c Ratio	0.03	0.06	0.83		0.36	0.91
Control Delay	26.3	18.1	27.8		48.4	19.4
Queue Delay	0.0	0.0	0.0		0.0	0.0
Total Delay	26.3	18.1	27.8		48.4	19.4
LOS	С	В	С		D	В
Approach Delay	20.4		27.8			20.6
Approach LOS	С		С			С
Queue Length 50th (ft)	5	10	337		23	139
Queue Length 95th (ft)	18	28	#584		m28	#677
Internal Link Dist (ft)	133		711			783
Turn Bay Length (ft)					150	
Base Capacity (vph)	334	487	866		105	995
Starvation Cap Reductn	0	0	0		0	0
Spillback Cap Reductn	0	0	0		0	0
Storage Cap Reductn	0	0	0		0	0
Reduced v/c Ratio	0.03	0.06	0.83		0.36	0.91

Cycle Length: 85

Actuated Cycle Length: 85

Offset: 78 (92%), Referenced to phase 4:NET and 8:SWT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.91

Intersection LOS: C

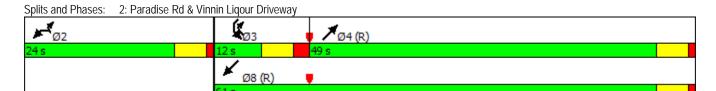
Intersection Signal Delay: 23.6 Intersection Capacity Utilization 71.2%

ICU Level of Service C

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.



	_≉	<b>→</b>	7	<b>/</b>	<b>←</b>	۲	•	×	/	6	K	✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	7	ĵ,		7	f)		*	f.		7	<b></b>	7
Traffic Volume (vph)	68	15	75	57	41	13	72	622	8	22	782	144
Future Volume (vph)	68	15	75	57	41	13	72	622	8	22	782	144
Satd. Flow (prot)	1496	1282	0	1496	1485	0	1496	1507	0	1496	1511	1338
Flt Permitted	0.720			0.695			0.261			0.293		
Satd. Flow (perm)	1084	1282	0	1049	1485	0	409	1507	0	459	1511	1277
Satd. Flow (RTOR)												
Lane Group Flow (vph)	72	95	0	60	57	0	76	663	0	23	823	152
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	Perm
Protected Phases		6			2		7	4		3	8	
Permitted Phases	6			2			4			8		8
Total Split (s)	14.0	14.0		14.0	14.0		12.0	59.0		12.0	59.0	59.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	6.0		5.0	5.0	5.0
Act Effct Green (s)	8.6	8.6		8.6	8.6		61.8	60.8		56.8	56.8	56.8
Actuated g/C Ratio	0.10	0.10		0.10	0.10		0.73	0.72		0.67	0.67	0.67
v/c Ratio	0.66	0.74		0.57	0.38		0.20	0.62		0.06	0.82	0.18
Control Delay	66.1	69.8		58.0	43.2		1.3	3.5		6.1	20.4	6.8
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Delay	66.1	69.8		58.0	43.2		1.3	3.5		6.1	20.4	6.8
LOS	Е	Е		Е	D		Α	Α		Α	С	Α
Approach Delay		68.2			50.8			3.3			18.0	
Approach LOS		Е			D			Α			В	
Queue Length 50th (ft)	38	50		31	29		1	15		4	314	30
Queue Length 95th (ft)	#101	#125		#83	66		m2	m25		12	#605	56
Internal Link Dist (ft)		1622			222			783			407	
Turn Bay Length (ft)	150						500			150		
Base Capacity (vph)	114	135		111	157		386	1077		392	1009	853
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.63	0.70		0.54	0.36		0.20	0.62		0.06	0.82	0.18

Cycle Length: 85

Actuated Cycle Length: 85

Offset: 0 (0%), Referenced to phase 4:NETL and 8:SWTL, Start of Green, Master Intersection

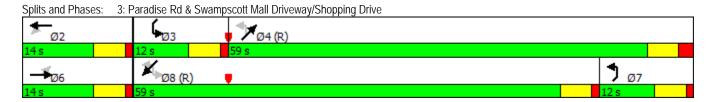
Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.82

Intersection Signal Delay: 18.7 Intersection LOS: B Intersection Capacity Utilization 73.5% ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.



	•	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	4	<b>†</b>	/	-	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	ĵ.		¥	<b>+</b>	7		414	7		414	
Traffic Volume (vph)	25	303	50	327	410	72	32	400	189	76	388	25
Future Volume (vph)	25	303	50	327	410	72	32	400	189	76	388	25
Satd. Flow (prot)	1496	1535	0	1417	1491	1338	0	2979	1285	0	2318	0
Flt Permitted	0.485			0.235				0.886			0.779	
Satd. Flow (perm)	764	1535	0	350	1491	1300	0	2650	1248	0	1820	0
Satd. Flow (RTOR)												
Lane Group Flow (vph)	26	364	0	337	423	74	0	445	195	0	504	0
Turn Type	Perm	NA		pm+pt	NA	Perm	Perm	NA	Perm	pm+pt	NA	
Protected Phases		6		5	2			4		3	8	
Permitted Phases	6			2		2	4		4	8		
Total Split (s)	36.0	36.0		12.0	48.0	48.0	40.0	40.0	40.0	12.0	52.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0		5.0	5.0		5.0	
Act Effct Green (s)	27.4	27.4		39.4	39.4	39.4		35.0	35.0		50.6	
Actuated g/C Ratio	0.27	0.27		0.39	0.39	0.39		0.35	0.35		0.51	
v/c Ratio	0.12	0.87		1.59	0.72	0.14		0.48	0.45		0.52	
Control Delay	23.5	48.3		301.1	27.7	17.6		27.5	29.1		12.8	
Queue Delay	0.0	0.0		0.0	11.7	0.0		0.0	0.0		0.0	
Total Delay	23.5	48.3		301.1	39.4	17.6		27.5	29.1		12.8	
LOS	С	D		F	D	В		С	С		В	
Approach Delay		46.6			143.2			28.0			12.8	
Approach LOS		D			F			С			В	
Queue Length 50th (ft)	15	238		~248	172	23		115	95		46	
Queue Length 95th (ft)	m18	m310		m#450	m291	m39		163	162		82	
Internal Link Dist (ft)		529			213			331			571	
Turn Bay Length (ft)	150								150			
Base Capacity (vph)	236	475		212	641	559		927	436		973	
Starvation Cap Reductn	0	0		0	193	0		0	0		0	
Spillback Cap Reductn	0	0		0	0	0		0	0		0	
Storage Cap Reductn	0	0		0	0	0		0	0		0	
Reduced v/c Ratio	0.11	0.77		1.59	0.94	0.13		0.48	0.45		0.52	

Cycle Length: 100
Actuated Cycle Length: 100

Offset: 0 (0%), Referenced to phase 4:NBTL and 8:SBTL, Start of Green, Master Intersection

Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.59 Intersection Signal Delay: 68.4 Intersection Capacity Utilization 93.5%

Intersection LOS: E
ICU Level of Service F

Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.

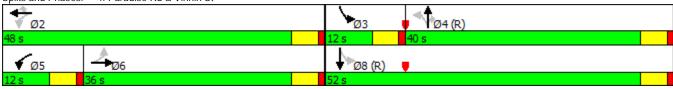
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Paradise Rd & Vinnin St



	•	•	•	<b>†</b>	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ሻ	7	ř	<b></b>	<b></b>	7
Traffic Volume (vph)	263	25	9	499	456	198
Future Volume (vph)	263	25	9	499	456	198
Satd. Flow (prot)	1133	1056	1496	1511	1511	1338
Flt Permitted	0.950		0.335			
Satd. Flow (perm)	1133	1017	527	1511	1511	1270
Satd. Flow (RTOR)						
Lane Group Flow (vph)	271	26	9	514	470	204
Turn Type	Prot	Perm	pm+pt	NA	NA	Perm
Protected Phases	4		5	2	6	
Permitted Phases		4	2			6
Total Split (s)	29.0	29.0	16.0	71.0	55.0	55.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0
Act Effct Green (s)	24.0	24.0	66.0	66.0	50.0	50.0
Actuated g/C Ratio	0.24	0.24	0.66	0.66	0.50	0.50
v/c Ratio	1.00	0.11	0.02	0.52	0.62	0.32
Control Delay	71.6	13.4	3.9	10.7	22.7	16.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	71.6	13.4	3.9	10.7	22.7	16.7
LOS	Е	В	А	В	С	В
Approach Delay	66.5			10.6	20.9	
Approach LOS	Е			В	С	
Queue Length 50th (ft)	188	17	1	91	208	74
Queue Length 95th (ft)	m#266	m12	m3	131	318	126
Internal Link Dist (ft)	691			571	296	
Turn Bay Length (ft)		150				
Base Capacity (vph)	271	244	454	997	755	635
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.00	0.11	0.02	0.52	0.62	0.32
	50					

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 93 (93%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.00 Intersection Signal Delay: 26.3 Intersection Capacity Utilization 58.0%

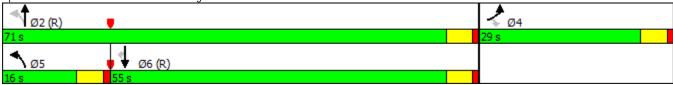
Intersection LOS: C ICU Level of Service B

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Paradise Rd & Loring Ave



	_#	-	7	<b>*</b>	<b>←</b>	۴	•	×	/	6	K	</th
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		- €		7	<b>1</b>		7	•	7	¥	ĵ.	
Traffic Volume (vph)	2	1	2	440	5	17	2	300	330	14	195	2
Future Volume (vph)	2	1	2	440	5	17	2	300	330	14	195	2
Satd. Flow (prot)	0	1362	0	1170	1005	0	1481	1497	1326	1481	1495	0
Flt Permitted		0.968		0.754			0.608			0.242		
Satd. Flow (perm)	0	1345	0	928	1005	0	948	1497	1263	377	1495	0
Satd. Flow (RTOR)									*200			
Lane Group Flow (vph)	0	5	0	543	27	0	2	370	407	17	243	0
Turn Type	Perm	NA		Perm	NA		Perm	NA	Perm	pm+pt	NA	
Protected Phases		6			2			4		3	8	
Permitted Phases	6			2			4		4	8		
Total Split (s)	55.0	55.0		55.0	55.0		33.0	33.0	33.0	12.0	45.0	
Total Lost Time (s)		5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Act Effct Green (s)		56.8		56.8	56.8		28.5	28.5	28.5	33.2	33.2	
Actuated g/C Ratio		0.57		0.57	0.57		0.28	0.28	0.28	0.33	0.33	
v/c Ratio		0.01		1.03	0.05		0.01	0.87	0.81	0.09	0.49	
Control Delay		12.2		61.2	4.5		26.0	55.7	30.6	14.4	19.6	
Queue Delay		0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Delay		12.2		61.2	4.5		26.0	55.7	30.6	14.4	19.6	
LOS		В		Е	Α		С	Е	С	В	В	
Approach Delay		12.3			58.5			42.5			19.3	
Approach LOS		В			Е			D			В	
Queue Length 50th (ft)		1		~185	3		1	207	116	4	61	
Queue Length 95th (ft)		7		#532	m5		6	#326	205	9	65	
Internal Link Dist (ft)		69			529			662			691	
Turn Bay Length (ft)				150			100		150	150		
Base Capacity (vph)		763		526	570		279	440	512	202	598	
Starvation Cap Reductn		0		0	0		0	0	0	0	0	
Spillback Cap Reductn		0		0	0		0	0	0	0	0	
Storage Cap Reductn		0		0	0		0	0	0	0	0	
Reduced v/c Ratio		0.01		1.03	0.05		0.01	0.84	0.79	0.08	0.41	

Cycle Length: 100
Actuated Cycle Length: 100

Offset: 45 (45%), Referenced to phase 2:WBTL and 6:EBTL, Start of Green

Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.03

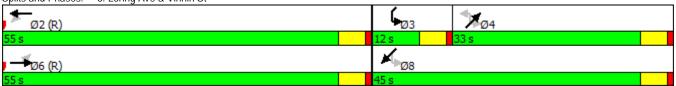
Intersection Signal Delay: 44.3 Intersection Capacity Utilization 66.9% Intersection LOS: D
ICU Level of Service C

Analysis Period (min) 15

- * User Entered Value
- Volume exceeds capacity, queue is theoretically infinite.
   Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.

  Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Loring Ave & Vinnin St



	-	•	•	•	4	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>*</b>	7		41₽	ř	7
Traffic Volume (vph)	423	151	63	654	145	68
Future Volume (vph)	423	151	63	654	145	68
Satd. Flow (prot)	1231	1046	0	1522	1192	1024
Flt Permitted				0.870	0.950	
Satd. Flow (perm)	1231	994	0	1328	1175	1024
Satd. Flow (RTOR)						
Lane Group Flow (vph)	436	156	0	739	149	70
Turn Type	NA	Perm	pm+pt	NA	Prot	Prot
Protected Phases	6		5	2	4	4
Permitted Phases		6	2			
Total Split (s)	62.0	62.0	12.0	74.0	26.0	26.0
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0
Act Effct Green (s)	73.2	73.2		73.2	16.8	16.8
Actuated g/C Ratio	0.73	0.73		0.73	0.17	0.17
v/c Ratio	0.48	0.21		0.76	0.74	0.41
Control Delay	11.4	8.9		16.0	61.1	43.0
Queue Delay	1.4	0.6		0.3	0.4	0.0
Total Delay	12.7	9.5		16.3	61.5	43.0
LOS	В	Α		В	Е	D
Approach Delay	11.9			16.3	55.6	
Approach LOS	В			В	Ε	
Queue Length 50th (ft)	129	43		132	90	40
Queue Length 95th (ft)	m166	m65		261	154	81
Internal Link Dist (ft)	213			312	357	
Turn Bay Length (ft)						150
Base Capacity (vph)	901	727		972	250	215
Starvation Cap Reductn	272	318		0	0	0
Spillback Cap Reductn	0	0		27	8	0
Storage Cap Reductn	0	0		0	0	0
Reduced v/c Ratio	0.69	0.38		0.78	0.62	0.33

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 50 (50%), Referenced to phase 2:WBTL and 6:EBT, Start of Green

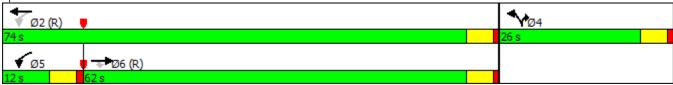
Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.76 Intersection Signal Delay: 20.1 Intersection Capacity Utilization 97.2%

Intersection LOS: C ICU Level of Service F

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 7: Salem St & Vinnin St



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	-	74	4	←	*	4
Lane Group	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	<b>^</b>	7	¥	<b>*</b>	¥	7
Traffic Volume (vph)	621	149	44	666	203	22
Future Volume (vph)	621	149	44	666	203	22
Satd. Flow (prot)	1589	1297	1510	1526	1510	1351
Flt Permitted			0.209		0.950	
Satd. Flow (perm)	1589	1237	332	1526	1510	1351
Satd. Flow (RTOR)						
Lane Group Flow (vph)	647	155	46	694	211	23
Turn Type	NA	Perm	pm+pt	NA	Prot	pt+ov
Protected Phases	4		3	8	2	23
Permitted Phases		4	8			
Total Split (s)	40.0	40.0	15.0	55.0	25.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	
Act Effct Green (s)	31.5	31.5	37.4	37.4	14.2	26.6
Actuated g/C Ratio	0.50	0.50	0.60	0.60	0.23	0.43
v/c Ratio	0.81	0.25	0.14	0.76	0.62	0.04
Control Delay	25.7	12.7	6.4	15.9	33.0	14.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	25.7	12.7	6.4	15.9	33.0	14.5
LOS	С	В	Α	В	С	В
Approach Delay	23.2			15.3	31.2	
Approach LOS	С			В	С	
Queue Length 50th (ft)	229	37	6	169	86	6
Queue Length 95th (ft)	#491	85	20	353	161	21
Internal Link Dist (ft)	1242			517	1622	
Turn Bay Length (ft)		200	100			150
Base Capacity (vph)	973	758	403	1219	523	648
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.66	0.20	0.11	0.57	0.40	0.04

Cycle Length: 80

Actuated Cycle Length: 62.5

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.81

Intersection Signal Delay: 21.0

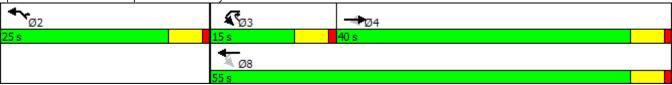
Intersection Capacity Utilization 61.5%

Intersection LOS: C ICU Level of Service B

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 8: Swampscott Mall Driveway & Essex St



	×	À	_	*	ን	~		
Lane Group	SET	SER	NWL	NWT	NEL	NER	Ø9	
Lane Configurations	<b>^</b>	7	ř	<b>*</b>	N/F			
Traffic Volume (vph)	680	120	37	900	15	15		
Future Volume (vph)	680	120	37	900	15	15		
Satd. Flow (prot)	1731	1471	1678	1731	1542	0		
Flt Permitted			0.298		0.976			
Satd. Flow (perm)	1731	1422	525	1731	1507	0		
Satd. Flow (RTOR)								
Lane Group Flow (vph)	739	130	40	978	32	0		
Turn Type	NA	Perm	Perm	NA	Prot			
Protected Phases	6			2	4		9	
Permitted Phases		6	2					
Total Split (s)	50.0	50.0	50.0	50.0	15.0		25.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0			
Act Effct Green (s)	52.6	52.6	52.6	52.6	9.0			
Actuated g/C Ratio	0.80	0.80	0.80	0.80	0.14			
v/c Ratio	0.53	0.11	0.10	0.70	0.15			
Control Delay	13.1	8.4	10.1	18.2	34.5			
Queue Delay	0.0	0.0	0.0	0.0	0.0			
Total Delay	13.2	8.4	10.1	18.2	34.5			
LOS	В	Α	В	В	С			
Approach Delay	12.4			17.9	34.5			
Approach LOS	В			В	С			
Queue Length 50th (ft)	0	0	0	0	8			
Queue Length 95th (ft)	#580	72	31	#883	45			
Internal Link Dist (ft)	497			268	259			
Turn Bay Length (ft)		150	150					
Base Capacity (vph)	1388	1140	421	1388	254			
Starvation Cap Reductn	16	0	0	0	0			
Spillback Cap Reductn	0	0	0	0	0			
Storage Cap Reductn	0	0	0	0	0			
Reduced v/c Ratio	0.54	0.11	0.10	0.70	0.13			

Cycle Length: 90

Actuated Cycle Length: 65.6

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.70

Intersection Signal Delay: 15.7

Intersection Capacity Utilization 62.4%

Intersection LOS: B

ICU Level of Service B

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 9: Harrison Rd & Loring Ave



	*	<b>†</b>	ļ	لر	<b>*</b>	4			
Lane Group	NBL	NBT	SBT	SBR	NEL	NER	Ø9		
Lane Configurations		ર્ન	ą.		**				
Traffic Volume (vph)	5	939	810	12	36	11			
Future Volume (vph)	5	939	810	12	36	11			
Satd. Flow (prot)	0	1731	1726	0	1630	0			
Flt Permitted		0.996			0.963				
Satd. Flow (perm)	0	1724	1726	0	1630	0			
Satd. Flow (RTOR)									
Lane Group Flow (vph)	0	1026	893	0	51	0			
Turn Type	Perm	NA	NA		Prot				
Protected Phases		2	6		4		9		
Permitted Phases	2								
Total Split (s)	41.0	41.0	41.0		25.0		19.0		
Total Lost Time (s)		6.0	5.0		5.0				
Act Effct Green (s)		46.2	46.8		7.7				
Actuated g/C Ratio		0.76	0.77		0.13				
v/c Ratio		0.78	0.67		0.25				
Control Delay		18.1	13.7		28.3				
Queue Delay		0.0	0.0		0.0				
Total Delay		18.1	13.7		28.3				
LOS		В	В		С				
Approach Delay		18.1	13.7		28.3				
Approach LOS		В	В		С				
Queue Length 50th (ft)		217	145		18				
Queue Length 95th (ft)		#844	#693		53				
Internal Link Dist (ft)		497	670		323				
Turn Bay Length (ft)									
Base Capacity (vph)		1312	1330		544				
Starvation Cap Reductn		0	0		0				
Spillback Cap Reductn		0	0		0				
Storage Cap Reductn		0	0		0				
Reduced v/c Ratio		0.78	0.67		0.09				

Cycle Length: 85

Actuated Cycle Length: 60.7 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.78 Intersection Signal Delay: 16.4

Intersection Capacity Utilization 67.6% Analysis Period (min) 15

Intersection LOS: B ICU Level of Service C

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 10: Pickman Rd & Loring Ave



	<b>→</b>	74	4	←	*	4
Movement	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	<b>†</b>	7	*	<b>*</b>	W	
Traffic Volume (veh/h)	785	25	67	628	5	152
Future Volume (Veh/h)	785	25	67	628	5	152
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Hourly flow rate (vph)	818	26	70	654	5	158
Pedestrians	5			5	5	
Lane Width (ft)	11.0			11.0	11.0	
Walking Speed (ft/s)	3.0			3.0	3.0	
Percent Blockage	1			1	1	
Right turn flare (veh)						
Median type	None			None		
Median type  Median storage veh)	140110			140110		
Upstream signal (ft)				914		
pX, platoon unblocked				/17	0.79	
vC, conflicting volume			849		1622	828
vC1, stage 1 conf vol			017		1022	020
vC2, stage 2 conf vol						
vCu, unblocked vol			849		1654	828
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)			1.1		0.1	0.2
tF (s)			2.2		3.5	3.3
p0 queue free %			91		93	57
cM capacity (veh/h)			776		76	364
						304
Direction, Lane # Volume Total	EB 1	EB 2	WB 1 70	WB 2	NW 1	
	818	26		654	163	
Volume Left	0	0	70	0	5	
Volume Right	0	26	0	0	158	
cSH	1700	1700	776	1700	327	
Volume to Capacity	0.48	0.02	0.09	0.38	0.50	
Queue Length 95th (ft)	0	0	7	0	66	
Control Delay (s)	0.0	0.0	10.1	0.0	26.5	
Lane LOS			В		D	
Approach Delay (s)	0.0		1.0		26.5	
Approach LOS					D	
Intersection Summary						
Average Delay			2.9			
Intersection Capacity Utilization			65.7%	IC	U Level of	Service
Analysis Period (min)			15			

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	۶	<b>→</b>	+	4	<b>/</b>	4
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	<b>1</b>		¥/	
Traffic Volume (veh/h)	162	352	617	174	73	62
Future Volume (Veh/h)	162	352	617	174	73	62
Sign Control	102	Free	Free	.,,	Stop	02
Grade		0%	0%		0%	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	178	387	678	191	80	68
Pedestrians	170	10	10	171	10	00
Lane Width (ft)		11.0	11.0		11.0	
Walking Speed (ft/s)		3.0	3.0		3.0	
Percent Blockage		3.0	3.0 1		3.0	
Right turn flare (veh)		ı	ı		ı	
Median type		None	None			
Median storage veh)		None	None			
Upstream signal (ft) pX, platoon unblocked						
vC, conflicting volume	879				1536	794
vC1, stage 1 conf vol	019				1330	/74
vC1, stage 1 conf vol						
vCu, unblocked vol	879				1536	794
	879 4.1				6.4	6.2
tC, single (s)	4.1				0.4	0.2
tC, 2 stage (s) tF (s)	2.2				3.5	3.3
	76				3.5 15	3.3 82
p0 queue free %						
cM capacity (veh/h)	753				94	377
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	565	869	148			
Volume Left	178	0	80			
Volume Right	0	191	68			
cSH	753	1700	144			
Volume to Capacity	0.24	0.51	1.03			
Queue Length 95th (ft)	23	0	192			
Control Delay (s)	5.9	0.0	143.7			
Lane LOS	Α		F			
Approach Delay (s)	5.9	0.0	143.7			
Approach LOS			F			
Intersection Summary						
Average Delay			15.6			
Intersection Capacity Utilization			90.5%	IC	U Level of	Service
Analysis Period (min)			15	10		_ 5
ruary sis i oriou (illiii)			10			

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	<b>₩</b>	لر	<b>*</b>	×	×	₹
Movement	SBL	SBR	NEL	NET	SWT	SWR
Lane Configurations	W			ની	<b>∱</b>	
Traffic Volume (veh/h)	88	28	23	428	666	145
Future Volume (Veh/h)	88	28	23	428	666	145
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	95	30	25	460	716	156
Pedestrians	30			30	30	
Lane Width (ft)	11.0			11.0	11.0	
Walking Speed (ft/s)	3.0			3.0	3.0	
Percent Blockage	3			3	3	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (ft)				706		
pX, platoon unblocked	0.86			, 00		
vC, conflicting volume	1364	854	902			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1342	854	902			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	28	91	97			
cM capacity (veh/h)	131	337	731			
Direction, Lane #	SB 1	NE 1	SW 1			
Volume Total	125	485	872			
Volume Left	95	25	0			
Volume Right	30	0	156			
cSH	154	731	1700			
Volume to Capacity	0.81	0.03	0.51			
Queue Length 95th (ft)	132	3	0.51			
Control Delay (s)	87.9	1.0	0.0			
Lane LOS	67. <del>9</del>	Α	0.0			
Approach Delay (s)	87.9	1.0	0.0			
Approach LOS	67.7 F	1.0	0.0			
	ı					
Intersection Summary						
Average Delay			7.7			
Intersection Capacity Utilization			62.1%	IC	U Level of S	ervice
Analysis Period (min)			15			

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Movement         EBL         EBR         NBL         NBT         SBR           Lane Configurations         Y         4         5           Traffic Volume (veh/h)         8         63         193         212         189         52           Future Volume (Veh/h)         8         63         193         212         189         52           Sign Control         Stop         Free         Free         Free           Grade         0%         0%         0%           Peak Hour Factor         0.92         0.92         0.92         0.92         0.92           Hourly flow rate (vph)         9         68         210         230         205         57           Pedestrians         Lane Width (ft)
Traffic Volume (veh/h)         8         63         193         212         189         52           Future Volume (Veh/h)         8         63         193         212         189         52           Sign Control         Stop         Free         Free         Free           Grade         0%         0%         0%           Peak Hour Factor         0.92         0.92         0.92         0.92         0.92           Hourly flow rate (vph)         9         68         210         230         205         57           Pedestrians
Traffic Volume (veh/h)         8         63         193         212         189         52           Future Volume (Veh/h)         8         63         193         212         189         52           Sign Control         Stop         Free         Free         Free           Grade         0%         0%         0%           Peak Hour Factor         0.92         0.92         0.92         0.92         0.92           Hourly flow rate (vph)         9         68         210         230         205         57           Pedestrians
Future Volume (Veh/h)     8     63     193     212     189     52       Sign Control     Stop     Free     Free     Free       Grade     0%     0%     0%       Peak Hour Factor     0.92     0.92     0.92     0.92     0.92       Hourly flow rate (vph)     9     68     210     230     205     57       Pedestrians
Sign Control         Stop         Free         Free           Grade         0%         0%         0%           Peak Hour Factor         0.92         0.92         0.92         0.92         0.92           Hourly flow rate (vph)         9         68         210         230         205         57           Pedestrians         57         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20         20
Grade         0%         0%         0%           Peak Hour Factor         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92
Peak Hour Factor         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92         0.92
Hourly flow rate (vph) 9 68 210 230 205 57 Pedestrians
Pedestrians
Lane with (ii)
Walking Speed (ft/s)
Percent Blockage
Right turn flare (veh)
Median type None None
Median storage veh)
Upstream signal (ft) 437
pX, platoon unblocked
vC, conflicting volume 884 234 262
vC1, stage 1 conf vol
vC2, stage 2 conf vol
vCu, unblocked vol 884 234 262
tC, single (s) 6.4 6.2 *6.4
tC, 2 stage (s)
tF (s) 3.5 3.3 *3.3
p0 queue free % 96 92 73
cM capacity (veh/h) 230 806 770
Direction, Lane # EB 1 NB 1 SB 1
Volume Total 77 440 262
Volume Left 9 210 0
Volume Right 68 0 57
cSH 623 770 1700
Volume to Capacity 0.12 0.27 0.15
Queue Length 95th (ft) 11 28 0
Lane LOS B A
Approach Delay (s) 11.6 7.3 0.0
Approach LOS B
Intersection Summary
Average Delay 5.3
Intersection Capacity Utilization 59.8% ICU Level of Service
Analysis Period (min) 15
* User Entered Value

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Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		4			4			4			4	
Traffic Volume (vph)	28	20	2	1	8	2	2	625	13	5	420	15
Future Volume (vph)	28	20	2	1	8	2	2	625	13	5	420	15
Satd. Flow (prot)	0	1743	0	0	1752	0	0	1795	0	0	1790	0
Flt Permitted					0.963			0.999			0.994	
Satd. Flow (perm)	0	1792	0	0	1694	0	0	1793	0	0	1781	0
Satd. Flow (RTOR)		2			2			2			3	
Lane Group Flow (vph)	0	54	0	0	12	0	0	688	0	0	473	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		6			2			4			8	
Permitted Phases	6			2			4			8		
Total Split (s)	16.0	16.0		16.0	16.0		36.0	36.0		36.0	36.0	
Total Lost Time (s)		6.0			6.0			6.0			6.0	
Act Effct Green (s)		7.7			7.4			37.3			37.3	
Actuated g/C Ratio		0.17			0.16			0.80			0.80	
v/c Ratio		0.18			0.04			0.48			0.33	
Control Delay		22.7			21.7			11.4			8.3	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		22.7			21.7			11.4			8.3	
LOS		С			С			В			Α	
Approach Delay		22.7			21.7			11.4			8.3	
Approach LOS		С			С			В			Α	
Queue Length 50th (ft)		9			2			0			0	
Queue Length 95th (ft)		54			19			#526			275	
Internal Link Dist (ft)		155			218			904			626	
Turn Bay Length (ft)												
Base Capacity (vph)		419			397			1435			1425	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.13			0.03			0.48			0.33	

Cycle Length: 75

Actuated Cycle Length: 46.6

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.48 Intersection Signal Delay: 10.8

Intersection Capacity Utilization 54.3%

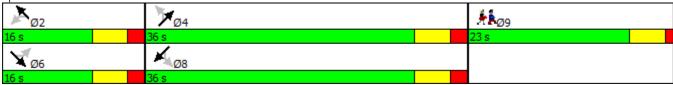
Intersection LOS: B ICU Level of Service A

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Paradise Rd & Ellis Rd



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Lane Group	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	7	7	ĵ.		¥	<b>*</b>
Traffic Volume (vph)	99	100	722	23	145	604
Future Volume (vph)	99	100	722	23	145	604
Satd. Flow (prot)	1540	1378	1274	0	1215	1279
Flt Permitted	0.950				0.176	
Satd. Flow (perm)	1540	1378	1274	0	225	1279
Satd. Flow (RTOR)						
Lane Group Flow (vph)	103	104	776	0	151	629
Turn Type	Prot	pt+ov	NA		pm+pt	NA
Protected Phases	2	23	4		3	8
Permitted Phases					8	
Total Split (s)	27.0		51.0		12.0	63.0
Total Lost Time (s)	5.0		5.0		6.0	5.0
Act Effct Green (s)	11.3	28.7	51.3		67.7	68.7
Actuated g/C Ratio	0.13	0.32	0.57		0.75	0.76
v/c Ratio	0.53	0.24	1.07		0.52	0.64
Control Delay	46.2	22.7	76.1		14.0	6.2
Queue Delay	0.0	0.0	0.0		0.0	0.0
Total Delay	46.2	22.7	76.1		14.0	6.2
LOS	D	С	Е		В	Α
Approach Delay	34.4		76.1			7.7
Approach LOS	С		Е			А
Queue Length 50th (ft)	56	43	~493		14	56
Queue Length 95th (ft)	101	76	#757		m42	m128
Internal Link Dist (ft)	133		711			783
Turn Bay Length (ft)					150	
Base Capacity (vph)	376	439	726		293	976
Starvation Cap Reductn	0	0	0		0	0
Spillback Cap Reductn	0	0	0		0	0
Storage Cap Reductn	0	0	0		0	0
Reduced v/c Ratio	0.27	0.24	1.07		0.52	0.64

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 78 (87%), Referenced to phase 4:NET and 8:SWTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.07

Intersection Signal Delay: 40.9

Intersection LOS: D

ICU Level of Service E

Intersection Capacity Utilization 86.2% Analysis Period (min) 15

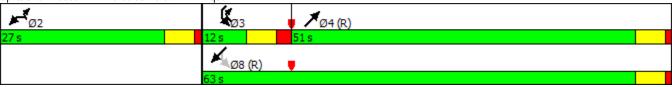
Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Paradise Rd & Vinnin Liqour Dr



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	7	ĵ,		7	ĵ.		ř	ĵ.		ř	<u> </u>	7
Traffic Volume (vph)	196	60	182	90	87	25	181	695	60	39	494	144
Future Volume (vph)	196	60	182	90	87	25	181	695	60	39	494	144
Satd. Flow (prot)	1540	1389	0	1540	1550	0	1215	1234	0	1215	1254	1066
Flt Permitted	0.682			0.386			0.450			0.131		
Satd. Flow (perm)	1105	1389	0	626	1550	0	576	1234	0	168	1254	1018
Satd. Flow (RTOR)												
Lane Group Flow (vph)	202	250	0	93	116	0	187	778	0	40	509	148
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	Perm
Protected Phases		6			2		7	4		3	8	
Permitted Phases	6			2			4			8		8
Total Split (s)	24.0	24.0		24.0	24.0		12.0	54.0		12.0	54.0	54.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	6.0		5.0	5.0	5.0
Act Effct Green (s)	18.2	18.2		18.2	18.2		54.6	53.6		49.8	49.8	49.8
Actuated g/C Ratio	0.20	0.20		0.20	0.20		0.61	0.60		0.55	0.55	0.55
v/c Ratio	0.90	0.89		0.73	0.37		0.47	1.06		0.24	0.73	0.26
Control Delay	76.5	67.9		67.4	34.4		7.6	56.3		13.2	23.4	12.4
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Delay	76.5	67.9		67.4	34.4		7.6	56.3		13.2	23.4	12.4
LOS	Е	Е		Е	С		Α	Е		В	С	В
Approach Delay		71.8			49.1			46.9			20.5	
Approach LOS		Е			D			D			С	
Queue Length 50th (ft)	112	138		49	57		21	~529		10	208	42
Queue Length 95th (ft)	#237	#271		#129	107		m23	m#531		25	350	80
Internal Link Dist (ft)		1630			222			783			390	
Turn Bay Length (ft)	150						500					150
Base Capacity (vph)	233	293		132	327		399	734		173	693	563
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.87	0.85		0.70	0.35		0.47	1.06		0.23	0.73	0.26

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 4:NETL and 8:SWTL, Start of Green, Master Intersection

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.06

Intersection Signal Delay: 44.0

Intersection Capacity Utilization 100.6%

Intersection LOS: D
ICU Level of Service G

Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Paradise Rd & Swampscott Mall Driveway/Shopping Drive

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	4î		7	•	7		41≯	7		4î}∍	
Traffic Volume (vph)	11	376	59	217	310	64	32	520	358	99	441	16
Future Volume (vph)	11	376	59	217	310	64	32	520	358	99	441	16
Satd. Flow (prot)	1215	1251	0	1215	1279	1088	0	2424	1088	0	2398	0
Flt Permitted	0.563			0.143				0.890			0.664	
Satd. Flow (perm)	720	1251	0	183	1279	1073	0	2163	1069	0	1607	0
Satd. Flow (RTOR)									*100			
Lane Group Flow (vph)	12	458	0	228	326	67	0	581	377	0	585	0
Turn Type	Perm	NA		pm+pt	NA	Perm	Perm	NA	Perm	pm+pt	NA	
Protected Phases		6		5	2			4		3	8	
Permitted Phases	6			2		2	4		4	8		
Total Split (s)	34.0	34.0		12.0	46.0	46.0	42.0	42.0	42.0	12.0	54.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0		5.0	5.0		5.0	
Act Effct Green (s)	29.0	29.0		41.0	41.0	41.0		37.0	37.0		49.0	
Actuated g/C Ratio	0.29	0.29		0.41	0.41	0.41		0.37	0.37		0.49	
v/c Ratio	0.06	1.27		1.55	0.62	0.15		0.73	0.82		0.69	
Control Delay	21.5	157.9		299.2	26.6	16.2		33.5	37.5		15.5	
Queue Delay	0.0	0.0		0.0	2.7	0.0		0.0	1.4		0.2	
Total Delay	21.5	157.9		299.2	29.3	16.2		33.5	38.9		15.7	
LOS	С	F		F	С	В		С	D		В	
Approach Delay		154.4			127.0			35.6			15.7	
Approach LOS		F			F			D			В	
Queue Length 50th (ft)	6	~380		~155	178	22		165	163		52	
Queue Length 95th (ft)	m7	m#388		#305	276	m43		233	#335		101	
Internal Link Dist (ft)		529			213			1004			571	
Turn Bay Length (ft)	150								150			
Base Capacity (vph)	208	362		147	524	439		800	458		842	
Starvation Cap Reductn	0	0		0	106	0		0	0		0	
Spillback Cap Reductn	0	0		0	0	0		0	17		20	
Storage Cap Reductn	0	0		0	0	0		0	0		0	
Reduced v/c Ratio	0.06	1.27		1.55	0.78	0.15		0.73	0.85		0.71	

Cycle Length: 100
Actuated Cycle Length: 100

Offset: 0 (0%), Referenced to phase 4:NBTL and 8:SBTL, Start of Green, Master Intersection

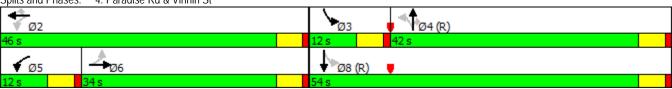
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 1.55
Intersection Signal Delay: 73.9
Intersection Capacity Utilization 110.0%

Intersection LOS: E ICU Level of Service H

Analysis Period (min) 15

- * User Entered Value
- Volume exceeds capacity, queue is theoretically infinite.
   Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Paradise Rd & Vinnin St



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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ሻ	*	ř	<b>†</b>	<b>*</b>	7
Traffic Volume (vph)	287	24	15	573	521	289
Future Volume (vph)	287	24	15	573	521	289
Satd. Flow (prot)	1191	1378	1540	1588	1588	1350
Flt Permitted	0.950		0.272			
Satd. Flow (perm)	1191	1338	441	1588	1588	1286
Satd. Flow (RTOR)						
Lane Group Flow (vph)	315	26	16	630	573	318
Turn Type	Prot	Perm	pm+pt	NA	NA	Perm
Protected Phases	4		5	2	6	
Permitted Phases		4	2			6
Total Split (s)	29.0	29.0	14.0	71.0	57.0	57.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0
Act Effct Green (s)	24.0	24.0	66.0	66.0	52.0	52.0
Actuated g/C Ratio	0.24	0.24	0.66	0.66	0.52	0.52
v/c Ratio	1.11	0.08	0.04	0.60	0.69	0.48
Control Delay	102.9	12.3	4.1	11.6	23.6	18.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	102.9	12.3	4.1	11.6	23.6	18.3
LOS	F	В	Α	В	С	В
Approach Delay	96.0			11.4	21.7	
Approach LOS	F			В	С	
Queue Length 50th (ft)	~240	11	2	71	261	123
Queue Length 95th (ft)	m#359	m13	m4	186	393	199
Internal Link Dist (ft)	691			571	296	
Turn Bay Length (ft)		150				
Base Capacity (vph)	285	321	389	1048	825	668
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.11	0.08	0.04	0.60	0.69	0.48

Cycle Length: 100
Actuated Cycle Length: 100

Offset: 93 (93%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.11 Intersection Signal Delay: 31.6

Intersection LOS: C
ICU Level of Service C

Intersection Capacity Utilization 64.2% Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.

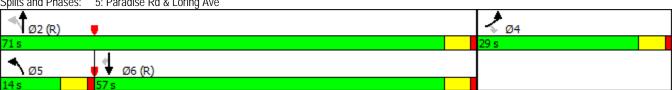
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Oueue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Paradise Rd & Loring Ave



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		€}-		7	ĵ.		7	•	7	¥	₽	
Traffic Volume (vph)	2	1	2	314	6	25	7	298	440	25	292	4
Future Volume (vph)	2	1	2	314	6	25	7	298	440	25	292	4
Satd. Flow (prot)	0	1445	0	1215	1063	0	1296	1337	1160	1296	1334	0
Flt Permitted		0.966		0.754			0.566			0.322		
Satd. Flow (perm)	0	1424	0	965	1063	0	772	1337	1105	439	1334	0
Satd. Flow (RTOR)									*200			
Lane Group Flow (vph)	0	5	0	341	34	0	8	324	478	27	321	0
Turn Type	Perm	NA		Perm	NA		Perm	NA	Perm	pm+pt	NA	
Protected Phases		6			2			4		3	8	
Permitted Phases	6			2			4		4	8		
Total Split (s)	57.0	57.0		57.0	57.0		30.0	30.0	30.0	13.0	43.0	
Total Lost Time (s)		5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Act Effct Green (s)		52.0		52.0	52.0		30.5	30.5	30.5	38.0	38.0	
Actuated g/C Ratio		0.52		0.52	0.52		0.30	0.30	0.30	0.38	0.38	
v/c Ratio		0.01		0.68	0.06		0.03	0.79	1.00	0.12	0.63	
Control Delay		11.6		11.0	2.5		29.0	50.2	65.9	13.9	19.0	
Queue Delay		0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Delay		11.6		11.0	2.5		29.0	50.2	65.9	13.9	19.0	
LOS		В		В	Α		С	D	Е	В	В	
Approach Delay		11.6			10.2			59.3			18.6	
Approach LOS		В			В			Е			В	
Queue Length 50th (ft)		2		29	3		4	202	~245	5	59	
Queue Length 95th (ft)		7		m46	m4		16	#380	#451	m10	83	
Internal Link Dist (ft)		69			529			965			691	
Turn Bay Length (ft)				150			100		150	150		
Base Capacity (vph)		740		501	552		235	408	476	235	506	
Starvation Cap Reductn		0		0	0		0	0	0	0	0	
Spillback Cap Reductn		0		0	0		0	0	0	0	0	
Storage Cap Reductn		0		0	0		0	0	0	0	0	
Reduced v/c Ratio		0.01		0.68	0.06		0.03	0.79	1.00	0.11	0.63	

Cycle Length: 100
Actuated Cycle Length: 100

Offset: 45 (45%), Referenced to phase 2:WBTL and 6:EBTL, Start of Green

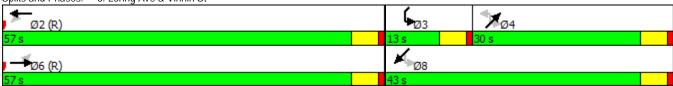
Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.00 Intersection Signal Delay: 37.9 Intersection Capacity Utilization 69.5%

Intersection LOS: D
ICU Level of Service C

Analysis Period (min) 15

- * User Entered Value
- Volume exceeds capacity, queue is theoretically infinite.
   Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Loring Ave & Vinnin St



	-	•	•	←	•	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>*</b>	*		41∱	¥	7
Traffic Volume (vph)	648	197	70	441	121	97
Future Volume (vph)	648	197	70	441	121	97
Satd. Flow (prot)	1588	1378	0	2414	1191	1088
Flt Permitted				0.782	0.950	
Satd. Flow (perm)	1588	1378	0	1901	1191	1088
Satd. Flow (RTOR)						
Lane Group Flow (vph)	682	207	0	538	127	102
Turn Type	NA	Perm	pm+pt	NA	Prot	Prot
Protected Phases	6		5	2	4	4
Permitted Phases		6	2			
Total Split (s)	64.0	64.0	12.0	76.0	24.0	24.0
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0
Act Effct Green (s)	75.0	75.0		75.0	15.0	15.0
Actuated g/C Ratio	0.75	0.75		0.75	0.15	0.15
v/c Ratio	0.57	0.20		0.38	0.71	0.63
Control Delay	8.8	6.1		5.8	61.1	55.9
Queue Delay	30.4	1.1		0.0	0.0	0.0
Total Delay	39.2	7.2		5.8	61.1	55.9
LOS	D	Α		Α	Е	Е
Approach Delay	31.8			5.8	58.8	
Approach LOS	С			Α	Е	
Queue Length 50th (ft)	201	51		54	77	61
Queue Length 95th (ft)	m210	m53		91	136	114
Internal Link Dist (ft)	213			175	347	
Turn Bay Length (ft)						150
Base Capacity (vph)	1190	1033		1424	226	206
Starvation Cap Reductn	535	612		0	0	0
Spillback Cap Reductn	0	0		27	0	0
Storage Cap Reductn	0	0		0	0	0
Reduced v/c Ratio	1.04	0.49		0.39	0.56	0.50

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 50 (50%), Referenced to phase 2:WBTL and 6:EBT, Start of Green

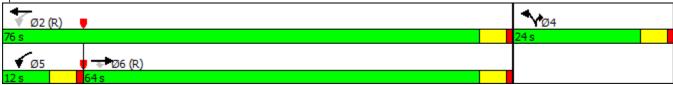
Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.71 Intersection Signal Delay: 27.1 Intersection Capacity Utilization 79.8%

Intersection LOS: C ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 7: Salem St & Vinnin St



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	-	•	•	<b>←</b>	4	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>^</b>	7	7	<b></b>	ሻ	7
Traffic Volume (vph)	693	254	66	631	259	108
Future Volume (vph)	693	254	66	631	259	108
Satd. Flow (prot)	1459	1240	1540	1588	1540	1378
Flt Permitted			0.171		0.950	
Satd. Flow (perm)	1459	1240	277	1588	1540	1378
Satd. Flow (RTOR)						
Lane Group Flow (vph)	745	273	71	678	278	116
Turn Type	NA	Perm	pm+pt	NA	Prot	pt+ov
Protected Phases	4		3	8	2	2 3
Permitted Phases		4	8			
Total Split (s)	55.0	55.0	20.0	75.0	30.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	
Act Effct Green (s)	50.2	50.2	62.3	62.3	21.0	33.2
Actuated g/C Ratio	0.54	0.54	0.67	0.67	0.22	0.36
v/c Ratio	0.95	0.41	0.25	0.64	0.80	0.24
Control Delay	45.7	16.3	8.5	13.3	52.4	22.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	45.7	16.3	8.5	13.3	52.4	22.1
LOS	D	В	Α	В	D	С
Approach Delay	37.8			12.8	43.5	
Approach LOS	D			В	D	
Queue Length 50th (ft)	409	94	14	220	156	48
Queue Length 95th (ft)	#718	171	31	363	#270	88
Internal Link Dist (ft)	1242			509	1630	
Turn Bay Length (ft)		200	100			150
Base Capacity (vph)	783	665	388	1194	413	598
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.95	0.41	0.18	0.57	0.67	0.19

Cycle Length: 105 Actuated Cycle Length: 93.4

Control Type: Actuated-Uncoordinated

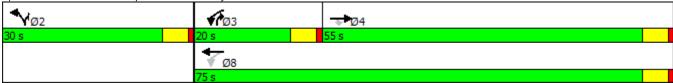
Maximum v/c Ratio: 0.95 Intersection Signal Delay: 30.2 Intersection Capacity Utilization 74.0%

Intersection LOS: C
ICU Level of Service D

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 8: Swampscott Mall Driveway & Essex St



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	$\mathbf{x}$	)	<b>F</b>	×	7	~	
Lane Group	SET	SER	NWL	NWT	NEL	NER	Ø9
Lane Configurations	<b>*</b>	7	7	•	N/A		
Traffic Volume (vph)	972	120	37	826	10	15	
Future Volume (vph)	972	120	37	826	10	15	
Satd. Flow (prot)	1493	1218	1621	1433	1623	0	
Flt Permitted			0.161		0.980		
Satd. Flow (perm)	1493	1182	275	1433	1623	0	
Satd. Flow (RTOR)							
Lane Group Flow (vph)	1057	130	40	898	27	0	
Turn Type	NA	Perm	Perm	NA	Prot		
Protected Phases	6			2	4		9
Permitted Phases		6	2				
Total Split (s)	55.0	55.0	55.0	55.0	20.0		22.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0		
Act Effct Green (s)	55.5	55.5	55.5	55.5	8.5		
Actuated g/C Ratio	0.86	0.86	0.86	0.86	0.13		
v/c Ratio	0.83	0.13	0.17	0.73	0.13		
Control Delay	18.1	5.1	8.6	14.2	32.2		
Queue Delay	0.0	0.0	0.0	0.0	0.0		
Total Delay	18.1	5.1	8.6	14.2	32.2		
LOS	В	Α	Α	В	С		
Approach Delay	16.7			14.0	32.2		
Approach LOS	В			В	С		
Queue Length 50th (ft)	0	0	0	0	8		
Queue Length 95th (ft)	#1022	67	35	#840	40		
Internal Link Dist (ft)	486			296	259		
Turn Bay Length (ft)		150	150				
Base Capacity (vph)	1279	1013	235	1228	390		
Starvation Cap Reductn	2	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0		
Reduced v/c Ratio	0.83	0.13	0.17	0.73	0.07		

Cycle Length: 97

Actuated Cycle Length: 64.8

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.83

Intersection Signal Delay: 15.7

Intersection Capacity Utilization 69.0%

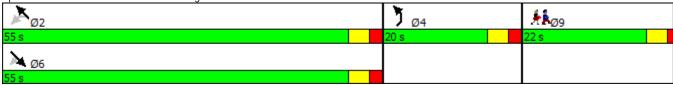
Intersection LOS: B ICU Level of Service C

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 9: Harrison Rd & Loring Ave



	*	<b>†</b>	ļ	لر	<b>*</b>	4		
Lane Group	NBL	NBT	SBT	SBR	NEL	NER	Ø9	
Lane Configurations		ર્સ	ĵ»		N/A			
Traffic Volume (vph)	14	786	1044	25	22	11		
Future Volume (vph)	14	786	1044	25	22	11		
Satd. Flow (prot)	0	1352	1349	0	1665	0		
Flt Permitted		0.788			0.968			
Satd. Flow (perm)	0	1066	1349	0	1665	0		
Satd. Flow (RTOR)								
Lane Group Flow (vph)	0	869	1162	0	36	0		
Turn Type	Perm	NA	NA		Prot			
Protected Phases		2	6		4		9	
Permitted Phases	2							
Total Split (s)	41.0	41.0	41.0		25.0		24.0	
Total Lost Time (s)		6.0	5.0		5.0			
Act Effct Green (s)		49.7	50.1		7.0			
Actuated g/C Ratio		0.83	0.83		0.12			
v/c Ratio		0.99	1.04		0.18			
Control Delay		44.2	52.1		28.5			
Queue Delay		0.0	0.0		0.0			
Total Delay		44.2	52.1		28.5			
LOS		D	D		С			
Approach Delay		44.2	52.1		28.5			
Approach LOS		D	D		С			
Queue Length 50th (ft)		0	0		11			
Queue Length 95th (ft)		#853	#1095		42			
Internal Link Dist (ft)		486	689		323			
Turn Bay Length (ft)								
Base Capacity (vph)		880	1122		561			
Starvation Cap Reductn		0	0		0			
Spillback Cap Reductn		0	0		0			
Storage Cap Reductn		0	0		0			
Reduced v/c Ratio		0.99	1.04		0.06			

Cycle Length: 90

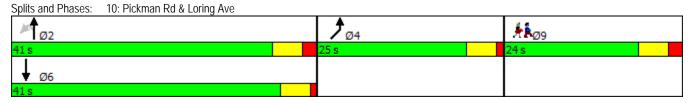
Actuated Cycle Length: 60.2 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 1.04 Intersection Signal Delay: 48.4

Intersection Capacity Utilization 76.4%

Intersection LOS: D ICU Level of Service D

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.



	<b>→</b>	¬₄	4	<b>←</b>	*	4
Movement	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	<b>†</b>	7	ች	<b>*</b>	W	
Traffic Volume (veh/h)	774	46	164	823	12	89
Future Volume (Veh/h)	774	46	164	823	12	89
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	798	47	169	848	12	92
Pedestrians	10				10	
Lane Width (ft)	11.0				11.0	
Walking Speed (ft/s)	3.0				3.0	
Percent Blockage	1				1	
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)				922		
pX, platoon unblocked					0.44	
vC, conflicting volume			855		2004	808
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			855		2633	808
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			78		0	76
cM capacity (veh/h)			777		9	377
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NW 1	
Volume Total	798	47	169	848	104	
Volume Left	0	0	169	0	12	
Volume Right	0	47	0	0	92	
cSH	1700	1700	777	1700	65	
Volume to Capacity	0.47	0.03	0.22	0.50	1.60	
Queue Length 95th (ft)	0	0	21	0	228	
Control Delay (s)	0.0	0.0	10.9	0.0	434.2	
Lane LOS	0.0	0.0	В	0.0	F	
Approach Delay (s)	0.0		1.8		434.2	
Approach LOS	0.0		1.0		F	
Intersection Summary			22.0			
Average Delay			23.9			C '
Intersection Capacity Utilization			66.0%	IC	U Level of	Service
Analysis Period (min)			15			

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		ની	ĵ.		W	
Traffic Volume (veh/h)	133	534	436	100	112	187
Future Volume (Veh/h)	133	534	436	100	112	187
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	143	574	469	108	120	201
Pedestrians		0, .	,,,		.20	
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)		INOLIC	INOLIC			
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	577				1383	523
vC1, stage 1 conf vol	311				1303	523
vC1, stage 1 conf vol						
vCz, stage z coni voi vCu, unblocked vol	577				1383	523
tC, single (s)	4.1				*6.2	6.2
	4.1				0.2	0.2
tC, 2 stage (s)	2.2				3.5	3.3
tF(s)						
p0 queue free %	86				19	64
cM capacity (veh/h)	996				148	554
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	717	577	321			
Volume Left	143	0	120			
Volume Right	0	108	201			
cSH	996	1700	273			
Volume to Capacity	0.14	0.34	1.18			
Queue Length 95th (ft)	13	0	359			
Control Delay (s)	3.4	0.0	150.1			
Lane LOS	Α		F			
Approach Delay (s)	3.4	0.0	150.1			
Approach LOS			F			
Intersection Summary						
Average Delay			31.4			
Intersection Capacity Utilization			92.2%	IC	U Level of	Service
Analysis Period (min)			15			
			- 10			
* User Entered Value						
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10. Calom Ct							
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	=5.	<b>T</b>	,	l unt	<b>V</b>		
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	W			•	<b>*</b>		
Traffic Volume (veh/h)	73	107	83	161	237	86	
Future Volume (Veh/h)	73	107	83	161	237	86	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	79	116	90	175	258	93	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (ft)					427		
pX, platoon unblocked							
vC, conflicting volume	660	304	351				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	660	304	351				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)	0.1	0.2					
tF (s)	3.5	3.3	2.2				
p0 queue free %	80	84	93				
cM capacity (veh/h)	396	735	1208				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	195	265	351				
Volume Left	79	90	0				
Volume Right	116	0	93				
cSH	546	1208	1700				
Volume to Capacity	0.36	0.07	0.21				
Queue Length 95th (ft)	40	6	0				
Control Delay (s)	15.2	3.2	0.0				
Lane LOS	С	Α					
Approach Delay (s)	15.2	3.2	0.0				
Approach LOS	С						
Intersection Summary							
Average Delay			4.7				
Intersection Capacity Utilization			51.4%	IC	U Level of S	Service	
Analysis Period (min)			15	10	2 23.3.01		
			10				

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Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		₩			4			4			44	
Traffic Volume (vph)	19	7	3	0	5	1	0	465	0	1	437	1
Future Volume (vph)	19	7	3	0	5	1	0	465	0	1	437	1
Satd. Flow (prot)	0	1722	0	0	1759	0	0	1801	0	0	1801	0
Flt Permitted											0.999	
Satd. Flow (perm)	0	1777	0	0	1759	0	0	1801	0	0	1799	0
Satd. Flow (RTOR)		3			1							
Lane Group Flow (vph)	0	31	0	0	6	0	0	500	0	0	472	0
Turn Type	Perm	NA			NA			NA		Perm	NA	
Protected Phases		6			2			4			8	
Permitted Phases	6			2			4			8		
Total Split (s)	16.0	16.0		16.0	16.0		36.0	36.0		36.0	36.0	
Total Lost Time (s)		6.0			6.0			6.0			6.0	
Act Effct Green (s)		7.2			6.9			38.5			38.5	
Actuated g/C Ratio		0.16			0.16			0.88			0.88	
v/c Ratio		0.11			0.02			0.32			0.30	
Control Delay		19.9			20.5			6.7			6.6	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		19.9			20.5			6.7			6.6	
LOS		В			С			Α			Α	
Approach Delay		19.9			20.5			6.7			6.6	
Approach LOS		В			С			Α			Α	
Queue Length 50th (ft)		5			1			0			0	
Queue Length 95th (ft)		36			13			291			270	
Internal Link Dist (ft)		155			218			904			594	
Turn Bay Length (ft)												
Base Capacity (vph)		444			439			1586			1584	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.07			0.01			0.32			0.30	

Cycle Length: 75

Actuated Cycle Length: 43.7

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.32 Intersection Signal Delay: 7.2

Intersection Capacity Utilization 43.3%

Intersection LOS: A ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 1: Paradise Rd & Ellis Rd



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Lane Group	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	*	7	f.		ሻ	<b></b>
Traffic Volume (vph)	84	195	654	14	182	664
Future Volume (vph)	84	195	654	14	182	664
Satd. Flow (prot)	1296	1160	1349	0	1296	1354
Flt Permitted	0.950				0.187	
Satd. Flow (perm)	1296	1160	1349	0	255	1354
Satd. Flow (RTOR)						
Lane Group Flow (vph)	88	203	696	0	190	692
Turn Type	Prot	pt+ov	NA		pm+pt	NA
Protected Phases	2	2 3	4		3	8
Permitted Phases					8	
Total Split (s)	30.0		51.0		12.0	63.0
Total Lost Time (s)	5.0		5.0		6.0	5.0
Act Effct Green (s)	17.8	34.5	48.5		64.2	65.2
Actuated g/C Ratio	0.19	0.37	0.52		0.69	0.70
v/c Ratio	0.36	0.47	0.99		0.64	0.73
Control Delay	35.0	25.8	56.6		21.1	16.1
Queue Delay	0.0	0.0	0.0		0.0	0.0
Total Delay	35.0	25.8	56.6		21.1	16.1
LOS	С	С	Е		С	В
Approach Delay	28.6		56.6			17.1
Approach LOS	С		Е			В
Queue Length 50th (ft)	46	85	~448		35	210
Queue Length 95th (ft)	82	150	#662		#133	#548
Internal Link Dist (ft)	133		759			783
Turn Bay Length (ft)					150	
Base Capacity (vph)	348	428	703		296	949
Starvation Cap Reductn	0	0	0		0	0
Spillback Cap Reductn	0	0	0		0	0
Storage Cap Reductn	0	0	0		0	0
Reduced v/c Ratio	0.25	0.47	0.99		0.64	0.73

Cycle Length: 93

Actuated Cycle Length: 93

Offset: 78 (84%), Referenced to phase 4:NET and 8:SWTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.99

Intersection Signal Delay: 33.6

Intersection LOS: C ICU Level of Service D

Intersection Capacity Utilization 81.6%

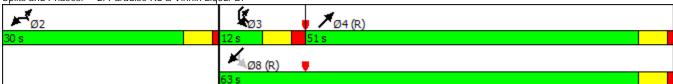
Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 2: Paradise Rd & Vinnin Ligour Dr



	_≉	<b>→</b>	7	<b>*</b>	•	٣	•	×	/	6	×	✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	*	ĵ,		7	f.		ř	ĵ,		ř	<b></b>	7
Traffic Volume (vph)	213	95	212	115	136	27	187	537	43	54	523	229
Future Volume (vph)	213	95	212	115	136	27	187	537	43	54	523	229
Satd. Flow (prot)	1296	1185	0	1296	1313	0	1296	1336	0	1296	1365	1151
Flt Permitted	0.599			0.368			0.304			0.266		
Satd. Flow (perm)	817	1185	0	502	1313	0	415	1336	0	363	1365	1112
Satd. Flow (RTOR)												
Lane Group Flow (vph)	220	317	0	119	168	0	193	598	0	56	539	236
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	Perm
Protected Phases		6			2		7	4		3	8	
Permitted Phases	6			2			4			8		8
Total Split (s)	35.0	35.0		35.0	35.0		12.0	53.0		12.0	53.0	53.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	6.0		5.0	5.0	5.0
Act Effct Green (s)	28.9	28.9		28.9	28.9		57.1	50.7		56.0	49.4	49.4
Actuated g/C Ratio	0.29	0.29		0.29	0.29		0.57	0.51		0.56	0.49	0.49
v/c Ratio	0.93	0.93		0.82	0.44		0.65	0.88		0.21	0.80	0.43
Control Delay	79.8	68.9		74.5	32.9		29.0	41.2		12.5	32.7	19.9
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Delay	79.8	68.9		74.5	32.9		29.0	41.2		12.5	32.7	19.9
LOS	E	Е		Е	С		С	D		В	С	В
Approach Delay		73.4			50.1			38.2			27.7	
Approach LOS		Е			D			D			С	
Queue Length 50th (ft)	133	191		69	85		54	354		14	285	96
Queue Length 95th (ft)	#275	#355		#172	148		#97	#588		30	#485	163
Internal Link Dist (ft)		1673			222			783			1014	
Turn Bay Length (ft)	150						500			150		150
Base Capacity (vph)	246	357		151	395		299	676		269	674	549
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.89	0.89		0.79	0.43		0.65	0.88		0.21	0.80	0.43

Cycle Length: 100
Actuated Cycle Length: 100

Offset: 0 (0%), Referenced to phase 4:NETL and 8:SWTL, Start of Green, Master Intersection

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.93 Intersection Signal Delay: 43.8 Intersection Capacity Utilization 99.2%

Intersection LOS: D
ICU Level of Service F

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Paradise Rd & Swampscott Mall Driveway/Shopping Drive



	٦	<b>→</b>	•	•	<b>←</b>	4	4	<b>†</b>	~	<b>/</b>	<b></b>	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Ť	£		¥	<b>*</b>	7		414	7		474	
Traffic Volume (vph)	22	282	72	279	367	106	42	392	303	101	438	29
Future Volume (vph)	22	282	72	279	367	106	42	392	303	101	438	29
Satd. Flow (prot)	1459	1476	0	1459	1523	1305	0	2891	1305	0	2856	0
Flt Permitted	0.533			0.165				0.849			0.732	
Satd. Flow (perm)	818	1476	0	253	1523	1305	0	2467	1305	0	2110	0
Satd. Flow (RTOR)												
Lane Group Flow (vph)	23	373	0	294	386	112	0	457	319	0	598	0
Turn Type	Perm	NA		pm+pt	NA	Perm	Perm	NA	Perm	pm+pt	NA	
Protected Phases		6		5	2			4		3	8	
Permitted Phases	6			2		2	4		4	8		
Total Split (s)	26.0	26.0		12.0	38.0	38.0	40.0	40.0	40.0	12.0	52.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0		5.0	5.0		5.0	
Act Effct Green (s)	21.0	21.0		33.0	33.0	33.0		35.0	35.0		47.0	
Actuated g/C Ratio	0.23	0.23		0.37	0.37	0.37		0.39	0.39		0.52	
v/c Ratio	0.12	1.08		1.58	0.69	0.23		0.48	0.63		0.52	
Control Delay	20.5	87.8		303.1	26.1	16.8		22.7	29.0		18.8	
Queue Delay	0.0	0.0		0.0	5.3	0.0		0.0	0.0		0.0	
Total Delay	20.5	87.8		303.1	31.4	16.8		22.7	29.0		18.8	
LOS	С	F		F	С	В		С	С		В	
Approach Delay		83.9			130.2			25.3			18.8	
Approach LOS		F			F			С			В	
Queue Length 50th (ft)	12	~250		~196	165	40		100	144		103	
Queue Length 95th (ft)	m16	m#271		m#333	m258	m66		145	238		m158	
Internal Link Dist (ft)		529			213			344			571	
Turn Bay Length (ft)	150								150			
Base Capacity (vph)	190	344		186	558	478		959	507		1159	
Starvation Cap Reductn	0	0		0	116	0		0	0		0	
Spillback Cap Reductn	0	0		0	0	0		0	0		0	
Storage Cap Reductn	0	0		0	0	0		0	0		0	
Reduced v/c Ratio	0.12	1.08		1.58	0.87	0.23		0.48	0.63		0.52	

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 4:NBTL and 8:SBTL, Start of Green, Master Intersection

Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.58

Intersection Signal Delay: 65.3

Intersection LOS: E ICU Level of Service E

Intersection Capacity Utilization 90.2% Analysis Period (min) 15

- Volume exceeds capacity, queue is theoretically infinite.
   Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
- Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

	•	•	•	<b>†</b>	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ሻ	7	ř	<b>*</b>	<b></b>	7
Traffic Volume (vph)	255	32	19	494	529	233
Future Volume (vph)	255	32	19	494	529	233
Satd. Flow (prot)	1286	1151	1296	1354	1365	1160
Flt Permitted	0.950		0.284			
Satd. Flow (perm)	1286	1128	388	1354	1365	1123
Satd. Flow (RTOR)						
Lane Group Flow (vph)	280	35	21	543	581	256
Turn Type	Prot	Perm	pm+pt	NA	NA	Perm
Protected Phases	4		5	2	6	
Permitted Phases		4	2			6
Total Split (s)	23.0	23.0	13.0	67.0	54.0	54.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0
Act Effct Green (s)	18.0	18.0	62.0	62.0	49.0	49.0
Actuated g/C Ratio	0.20	0.20	0.69	0.69	0.54	0.54
v/c Ratio	1.09	0.16	0.06	0.58	0.78	0.42
Control Delay	107.9	22.9	4.8	10.9	25.6	14.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	107.9	22.9	4.8	10.9	25.6	14.8
LOS	F	С	А	В	С	В
Approach Delay	98.4			10.6	22.3	
Approach LOS	F			В	С	
Queue Length 50th (ft)	~187	21	0	83	246	81
Queue Length 95th (ft)	m#317	m32	m0	115	#411	140
Internal Link Dist (ft)	691			571	296	
Turn Bay Length (ft)		150				
Base Capacity (vph)	257	225	348	932	743	611
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.09	0.16	0.06	0.58	0.78	0.42

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 3 (3%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.09 Intersection Signal Delay: 32.4

Intersection LOS: C ICU Level of Service B

Intersection Capacity Utilization 63.7%

Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Paradise Rd & Loring Ave Ø6 (R)

	_#	-	7	<b>*</b>	<b>←</b>	۴	•	×	/	Ĺ	×	</th
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		- 4}-		7	f)		ř	<b></b>	7	¥	ĵ,	
Traffic Volume (vph)	3	5	2	382	3	47	5	254	352	44	236	6
Future Volume (vph)	3	5	2	382	3	47	5	254	352	44	236	6
Satd. Flow (prot)	0	1543	0	1296	1126	0	1296	1354	1160	1296	1347	0
Flt Permitted		0.968		0.751			0.596			0.356		
Satd. Flow (perm)	0	1517	0	1025	1126	0	813	1354	1123	486	1347	0
Satd. Flow (RTOR)									*100			
Lane Group Flow (vph)	0	10	0	415	54	0	5	276	383	48	264	0
Turn Type	Perm	NA		Perm	NA		Perm	NA	Perm	pm+pt	NA	
Protected Phases		6			2			4		3	8	
Permitted Phases	6			2			4		4	8		
Total Split (s)	53.0	53.0		53.0	53.0		25.0	25.0	25.0	12.0	37.0	
Total Lost Time (s)		5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Act Effct Green (s)		48.0		48.0	48.0		24.8	24.8	24.8	32.0	32.0	
Actuated g/C Ratio		0.53		0.53	0.53		0.28	0.28	0.28	0.36	0.36	
v/c Ratio		0.01		0.76	0.09		0.02	0.74	1.01	0.21	0.55	
Control Delay		10.0		15.7	3.3		28.0	46.4	75.8	21.6	24.7	
Queue Delay		0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Delay		10.0		15.7	3.3		28.0	46.4	75.8	21.6	24.7	
LOS		Α		В	Α		С	D	Е	С	С	
Approach Delay		10.0			14.3			63.2			24.2	
Approach LOS		Α			В			Е			С	
Queue Length 50th (ft)		3		49	4		2	154	~215	12	69	
Queue Length 95th (ft)		10		#114	m7		11	#303	#392	35	133	
Internal Link Dist (ft)		69			529			965			691	
Turn Bay Length (ft)				150			100		150	150		
Base Capacity (vph)		809		546	600		224	372	381	235	478	
Starvation Cap Reductn		0		0	0		0	0	0	0	0	
Spillback Cap Reductn		0		0	0		0	0	0	0	0	
Storage Cap Reductn		0		0	0		0	0	0	0	0	
Reduced v/c Ratio		0.01		0.76	0.09		0.02	0.74	1.01	0.20	0.55	

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 41 (46%), Referenced to phase 2:WBTL and 6:EBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.01

Intersection Signal Delay: 38.7

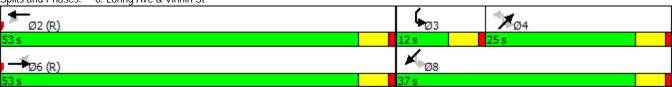
Intersection Capacity Utilization 72.3%

Intersection LOS: D
ICU Level of Service C

Analysis Period (min) 15

- * User Entered Value
- Volume exceeds capacity, queue is theoretically infinite.
   Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Loring Ave & Vinnin St



	<b>→</b>	•	•	<b>←</b>	•	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>*</b>	7		41≯	ř	7
Traffic Volume (vph)	505	183	146	527	185	96
Future Volume (vph)	505	183	146	527	185	96
Satd. Flow (prot)	1450	1232	0	2714	1447	1295
Flt Permitted				0.707	0.950	
Satd. Flow (perm)	1450	1193	0	1939	1419	1295
Satd. Flow (RTOR)						
Lane Group Flow (vph)	532	193	0	709	195	101
Turn Type	NA	Perm	pm+pt	NA	Prot	Prot
Protected Phases	6		5	2	4	4
Permitted Phases		6	2			
Total Split (s)	59.0	59.0	12.0	71.0	19.0	19.0
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0
Act Effct Green (s)	66.3	66.3		66.3	13.7	13.7
Actuated g/C Ratio	0.74	0.74		0.74	0.15	0.15
v/c Ratio	0.50	0.22		0.50	0.89	0.51
Control Delay	7.6	5.9		6.4	76.5	45.1
Queue Delay	2.6	0.8		0.0	0.0	0.0
Total Delay	10.2	6.7		6.4	76.5	45.1
LOS	В	А		Α	Е	D
Approach Delay	9.3			6.4	65.8	
Approach LOS	А			Α	Е	
Queue Length 50th (ft)	122	39		73	110	53
Queue Length 95th (ft)	m152	m59		107	#231	105
Internal Link Dist (ft)	213			175	1023	
Turn Bay Length (ft)						150
Base Capacity (vph)	1068	878		1428	225	201
Starvation Cap Reductn	400	437		0	0	0
Spillback Cap Reductn	0	0		45	0	0
Storage Cap Reductn	0	0		0	0	0
Reduced v/c Ratio	0.80	0.44		0.51	0.87	0.50

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 50 (56%), Referenced to phase 2:WBTL and 6:EBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.89

Intersection Signal Delay: 17.8

Intersection Capacity Utilization 80.9%

Intersection LOS: B ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.





	-	•	•	<b>—</b>	4	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>+</b>	7	ř	<b>*</b>	7	7
Traffic Volume (vph)	535	284	99	575	291	140
Future Volume (vph)	535	284	99	575	291	140
Satd. Flow (prot)	1365	1151	1296	1354	1296	1151
Flt Permitted			0.236		0.950	
Satd. Flow (perm)	1365	1112	322	1354	1296	1151
Satd. Flow (RTOR)						
Lane Group Flow (vph)	575	305	106	618	313	151
Turn Type	NA	Perm	pm+pt	NA	Prot	pt+ov
Protected Phases	4		3	8	2	2 3
Permitted Phases		4	8			
Total Split (s)	47.0	47.0	23.0	70.0	30.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	
Act Effct Green (s)	43.1	43.1	56.7	56.7	24.8	38.5
Actuated g/C Ratio	0.47	0.47	0.62	0.62	0.27	0.42
v/c Ratio	0.90	0.58	0.36	0.74	0.89	0.31
Control Delay	41.7	23.5	10.8	18.7	61.8	20.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	41.7	23.5	10.8	18.7	61.8	20.2
LOS	D	С	В	В	Е	С
Approach Delay	35.4			17.5	48.3	
Approach LOS	D			В	D	
Queue Length 50th (ft)	294	124	23	229	171	56
Queue Length 95th (ft)	#532	221	44	369	#364	113
Internal Link Dist (ft)	1242			539	1673	
Turn Bay Length (ft)		200	100			150
Base Capacity (vph)	642	523	391	962	354	590
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.90	0.58	0.27	0.64	0.88	0.26

Cycle Length: 100 Actuated Cycle Length: 91.6

Control Type: Actuated-Uncoordinated

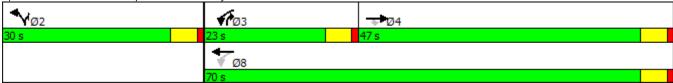
Maximum v/c Ratio: 0.90 Intersection Signal Delay: 32.0 Intersection Capacity Utilization 78.2%

Intersection LOS: C
ICU Level of Service D

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 8: Swampscott Mall Driveway & Essex St



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	$\mathbf{x}$	7	<b>*</b>	×	7	~	
Lane Group	SET	SER	NWL	NWT	NEL	NER	Ø9
Lane Configurations	<b>+</b>	7	ħ	•	N/A		
Traffic Volume (vph)	700	120	37	826	10	15	
Future Volume (vph)	700	120	37	826	10	15	
Satd. Flow (prot)	1801	1531	1711	1801	1623	0	
Flt Permitted			0.284		0.980		
Satd. Flow (perm)	1801	1531	511	1801	1623	0	
Satd. Flow (RTOR)							
Lane Group Flow (vph)	761	130	40	898	27	0	
Turn Type	NA	Perm	Perm	NA	Prot		
Protected Phases	6			2	4		9
Permitted Phases		6	2				
Total Split (s)	55.0	55.0	55.0	55.0	20.0		25.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0		
Act Effct Green (s)	44.2	44.2	44.2	44.2	9.3		
Actuated g/C Ratio	0.85	0.85	0.85	0.85	0.18		
v/c Ratio	0.50	0.10	0.09	0.59	0.09		
Control Delay	7.9	5.0	6.4	9.9	28.8		
Queue Delay	0.0	0.0	0.0	0.0	0.0		
Total Delay	7.9	5.0	6.4	9.9	28.8		
LOS	Α	Α	Α	Α	С		
Approach Delay	7.5			9.7	28.8		
Approach LOS	Α			Α	С		
Queue Length 50th (ft)	0	0	0	0	4		
Queue Length 95th (ft)	481	64	29	#718	40		
Internal Link Dist (ft)	486			296	259		
Turn Bay Length (ft)		150	150				
Base Capacity (vph)	1575	1339	447	1575	539		
Starvation Cap Reductn	36	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0		
Reduced v/c Ratio	0.49	0.10	0.09	0.57	0.05		

Cycle Length: 100 Actuated Cycle Length: 52

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.59 Intersection Signal Delay: 8.9 Intersection Capacity Utilization 58.5%

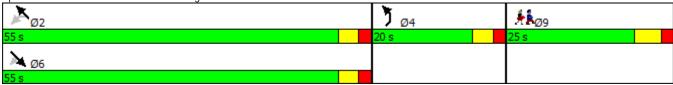
Intersection LOS: A ICU Level of Service B

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 9: Harrison Rd & Loring Ave



	M	<b>†</b>	ļ	لر	<b>*</b>	4		
Lane Group	NBL	NBT	SBT	SBR	NEL	NER	Ø9	
Lane Configurations		ર્ન	ĥ		N/A			
Traffic Volume (vph)	10	826	800	30	35	20		
Future Volume (vph)	10	826	800	30	35	20		
Satd. Flow (prot)	0	1799	1792	0	1658	0		
Flt Permitted		0.988			0.969			
Satd. Flow (perm)	0	1779	1792	0	1658	0		
Satd. Flow (RTOR)								
Lane Group Flow (vph)	0	909	903	0	60	0		
Turn Type	Perm	NA	NA		Prot			
Protected Phases		2	6		4		9	
Permitted Phases	2							
Total Split (s)	41.0	41.0	41.0		25.0		24.0	
Total Lost Time (s)		6.0	5.0		5.0			
Act Effct Green (s)		45.1	45.7		7.8			
Actuated g/C Ratio		0.75	0.76		0.13			
v/c Ratio		0.68	0.66		0.28			
Control Delay		15.3	14.3		29.0			
Queue Delay		0.0	0.0		0.0			
Total Delay		15.3	14.3		29.0			
LOS		В	В		С			
Approach Delay		15.3	14.3		29.0			
Approach LOS		В	В		С			
Queue Length 50th (ft)		159	143		18			
Queue Length 95th (ft)		#786	#765		63			
Internal Link Dist (ft)		486	689		323			
Turn Bay Length (ft)								
Base Capacity (vph)		1330	1358		563			
Starvation Cap Reductn		0	0		0			
Spillback Cap Reductn		0	0		0			
Storage Cap Reductn		0	0		0			
Reduced v/c Ratio		0.68	0.66		0.11			

Cycle Length: 90

Actuated Cycle Length: 60.3 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.68

Intersection Signal Delay: 15.3
Intersection Capacity Utilization 65.6%

Intersection LOS: B
ICU Level of Service C

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 10: Pickman Rd & Loring Ave



Movement         EBT         EBR         WBL         WBT         NWL         NWR           Lane Configurations         1         1         4         7         7         7         7         7         7         7         7         7         7         8         7         642         16         123         123         7         7         7         7         7         7         7         7         7         7         7         7         7         7         7         7         7         7         7         7         7         7         7         7         7         7         7         7         8         7         8         7         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8         8 </th
Lane Configurations  † † † † Traffic Volume (veh/h) 722 23 79 642 16 123
Traffic Volume (veh/h) 722 23 79 642 16 123
1 GLG1 0 V O O O O O O O O O O O O O O O O O O
Sign Control Free Stop
Grade 0% 0% 0%
Peak Hour Factor 0.97 0.97 0.97 0.97 0.97
Hourly flow rate (vph) 744 24 81 662 16 127
Pedestrians
Lane Width (ft)
Walking Speed (ft/s)
Percent Blockage
Right turn flare (veh)
Median type None None
Median storage veh)
Upstream signal (ft) 922
pX, platoon unblocked 0.79
vC, conflicting volume 768 1568 744
vC1, stage 1 conf vol
vC2, stage 2 conf vol
vCu, unblocked vol 768 1586 744
tC, single (s) 4.1 6.4 6.2
tc, 2 stage (s)
tF (s) 2.2 3.5 3.3
p0 queue free % 90 81 69
cM capacity (veh/h) 846 85 415
Direction, Lane # EB 1 EB 2 WB 1 WB 2 NW 1
Volume Total 744 24 81 662 143
Volume Left 0 0 81 0 16
Volume Right 0 24 0 0 127
cSH 1700 1700 846 1700 289
Volume to Capacity 0.44 0.01 0.10 0.39 0.49
Queue Length 95th (ft) 0 0 8 0 64
Control Delay (s) 0.0 0.0 9.7 0.0 29.1
Lane LOS A D
Approach Delay (s) 0.0 1.1 29.1
Approach LOS D
Intersection Summary
Average Delay 3.0
Intersection Capacity Utilization 60.9% ICU Level of Service
Analysis Period (min) 15

		<b>→</b>	<b>—</b>	4	<b>\</b>	4
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	<b>1</b>		¥	05.1
Traffic Volume (veh/h)	60	454	620	80	75	51
Future Volume (Veh/h)	60	454	620	80	75	51
Sign Control	00	Free	Free	00	Stop	J1
Grade		0%	0%		0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	65	488	667	86	81	55
Pedestrians	00	400	007	00	01	33
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)		N.1	N.I.			
Median type		None	None			
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked	750				1000	740
vC, conflicting volume	753				1328	710
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	753				1328	710
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	92				49	87
cM capacity (veh/h)	857				158	434
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	553	753	136			
Volume Left	65	0	81			
Volume Right	0	86	55			
cSH	857	1700	213			
Volume to Capacity	0.08	0.44	0.64			
Queue Length 95th (ft)	6	0	95			
Control Delay (s)	2.0	0.0	47.8			
Lane LOS	A		E			
Approach Delay (s)	2.0	0.0	47.8			
Approach LOS			E			
Intersection Summary						
Average Delay			5.3			
Intersection Capacity Utilization			82.0%	IC	U Level of	Service
Analysis Period (min)			15	ic	O LCVCI UI	JUI VILL
Analysis Penou (IIIII)			10			

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	<b>₩</b>	لر	•	*	×	₹
Movement	SBL	SBR	NEL	NET	SWT	SWR
Lane Configurations	**			स	1>	
Traffic Volume (veh/h)	51	14	12	447	460	65
Future Volume (Veh/h)	51	14	12	447	460	65
Sign Control	Stop	17	12	Free	Free	33
Grade	0%			0%	0%	
Peak Hour Factor	0.95	0.93	0.95	0.95	0.95	0.95
	0.95 54	15	13	471	484	68
Hourly flow rate (vph)	54	15	13	4/1	484	00
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (ft)				674		
pX, platoon unblocked	0.89					
vC, conflicting volume	1015	518	552			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	956	518	552			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.1	0.2	1.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	79	97	99			
	252	558	1018			
cM capacity (veh/h)						
Direction, Lane #	SB 1	NE 1	SW 1			
Volume Total	69	484	552			
Volume Left	54	13	0			
Volume Right	15	0	68			
cSH	286	1018	1700			
Volume to Capacity	0.24	0.01	0.32			
Queue Length 95th (ft)	23	1	0			
Control Delay (s)	21.5	0.4	0.0			
Lane LOS	C	A	0.0			
Approach Delay (s)	21.5	0.4	0.0			
Approach LOS	C C	0.1	0.0			
11	U					
Intersection Summary						
Average Delay			1.5			
Intersection Capacity Utilization			43.5%	IC	U Level of S	Service
Analysis Period (min)			15			

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# Level of Service (LOS) Analysis Alternative 1

	<b>4</b>	×	7	<b>F</b>	×	*	Ť	×	~	Ĺ	×	*
Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		₩			4			4			4	
Traffic Volume (vph)	22	45	4	15	44	7	5	417	8	5	659	31
Future Volume (vph)	22	45	4	15	44	7	5	417	8	5	659	31
Satd. Flow (prot)	0	1759	0	0	1756	0	0	1795	0	0	1790	0
Flt Permitted		0.870			0.899			0.991			0.997	
Satd. Flow (perm)	0	1554	0	0	1596	0	0	1781	0	0	1784	0
Satd. Flow (RTOR)		3			6			2			4	
Lane Group Flow (vph)	0	82	0	0	76	0	0	496	0	0	802	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		6			2			4			8	
Permitted Phases	6			2			4			8		
Total Split (s)	12.0	12.0		12.0	12.0		40.0	40.0		40.0	40.0	
Total Lost Time (s)		6.0			6.0			6.0			6.0	
Act Effct Green (s)		6.5			6.5			37.1			37.1	
Actuated g/C Ratio		0.11			0.11			0.64			0.64	
v/c Ratio		0.47			0.41			0.43			0.70	
Control Delay		40.2			36.5			12.1			19.3	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		40.2			36.5			12.1			19.3	
LOS		D			D			В			В	
Approach Delay		40.2			36.5			12.1			19.3	
Approach LOS		D			D			В			В	
Queue Length 50th (ft)		23			20			57			121	
Queue Length 95th (ft)		#102			#88			264			#604	
Internal Link Dist (ft)		155			218			904			626	
Turn Bay Length (ft)												
Base Capacity (vph)		176			184			1130			1132	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.47			0.41			0.44			0.71	

Cycle Length: 75

Actuated Cycle Length: 57.8

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.70

Intersection Signal Delay: 18.9

Intersection Capacity Utilization 57.6%

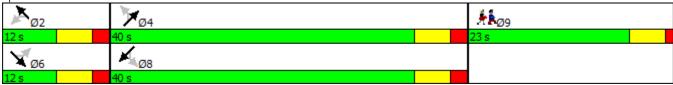
Intersection LOS: B ICU Level of Service B

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Paradise Rd & Ellis Rd



	<b>*</b>	₹	×	~	Ĺ	×
Lane Group	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	*	7	£		*	<b>*</b>
Traffic Volume (vph)	11	28	698	3	37	876
Future Volume (vph)	11	28	698	3	37	876
Satd. Flow (prot)	1496	1338	1510	0	1496	1511
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1450	1338	1510	0	1484	1511
Satd. Flow (RTOR)						
Lane Group Flow (vph)	12	30	759	0	40	948
Turn Type	Prot	pt+ov	NA		Prot	NA
Protected Phases	2	23	4		3	8
Permitted Phases						
Total Split (s)	19.0		54.0		12.0	66.0
Total Lost Time (s)	5.0		5.0		6.0	5.0
Act Effct Green (s)	14.0	26.0	53.8		6.0	61.0
Actuated g/C Ratio	0.16	0.31	0.63		0.07	0.72
v/c Ratio	0.05	0.07	0.79		0.38	0.87
Control Delay	30.7	21.7	21.6		38.8	14.5
Queue Delay	0.0	0.0	0.0		0.0	0.0
Total Delay	30.7	21.7	21.6		38.8	14.5
LOS	С	С	С		D	В
Approach Delay	24.3		21.6			15.5
Approach LOS	С		С			В
Queue Length 50th (ft)	5	11	318		20	204
Queue Length 95th (ft)	20	31	#578		m26	m#651
Internal Link Dist (ft)	133		711			785
Turn Bay Length (ft)					150	
Base Capacity (vph)	246	409	955		105	1084
Starvation Cap Reductn	0	0	0		0	0
Spillback Cap Reductn	0	0	0		0	0
Storage Cap Reductn	0	0	0		0	0
Reduced v/c Ratio	0.05	0.07	0.79		0.38	0.87

Cycle Length: 85

Actuated Cycle Length: 85

Offset: 4 (5%), Referenced to phase 4:NET and 8:SWT, Start of Green

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.87 Intersection Signal Delay: 18.3 Intersection Capacity Utilization 73.8%

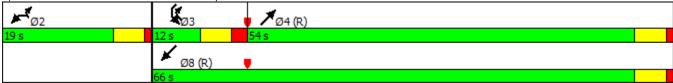
Intersection LOS: B ICU Level of Service D

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.





Noute 17 Villimi Oquale	of flority Corridor Clady
3: Paradise Rd & Swam	pscott Mall Driveway/Shopping Drive

	<b>⊸</b>	-	7	<b>F</b>	<b>←</b>	€_	•	*	/	6	K	✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	*	ą.		¥	ĵ,		¥	ĵ.		ř	<b>*</b>	7
Traffic Volume (vph)	68	15	75	57	41	13	72	622	8	22	782	144
Future Volume (vph)	68	15	75	57	41	13	72	622	8	22	782	144
Satd. Flow (prot)	1496	1283	0	1496	1488	0	1496	1508	0	1496	1511	1338
Flt Permitted	0.719			0.692			0.202			0.339		
Satd. Flow (perm)	1083	1283	0	1045	1488	0	317	1508	0	532	1511	1297
Satd. Flow (RTOR)												
Lane Group Flow (vph)	75	100	0	63	59	0	80	696	0	24	864	159
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	Perm
Protected Phases		6			2		7	4		3	8	
Permitted Phases	6			2			4			8		8
Total Split (s)	14.0	14.0		14.0	14.0		11.0	60.0		11.0	60.0	60.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	6.0		5.0	5.0	5.0
Act Effct Green (s)	8.7	8.7		8.7	8.7		64.3	60.9		62.1	57.5	57.5
Actuated g/C Ratio	0.10	0.10		0.10	0.10		0.76	0.72		0.73	0.68	0.68
v/c Ratio	0.68	0.76		0.59	0.39		0.25	0.64		0.05	0.85	0.18
Control Delay	68.0	73.3		60.2	43.4		4.4	8.6		2.6	21.9	6.4
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Delay	68.0	73.3		60.2	43.4		4.4	8.6		2.6	21.9	6.4
LOS	E	Е		Е	D		Α	Α		Α	С	Α
Approach Delay		71.0			52.1			8.2			19.1	
Approach LOS		Е			D			Α			В	
Queue Length 50th (ft)	39	53		33	30		9	100		2	337	30
Queue Length 95th (ft)	#106	#133		#87	68		m18	m180		6	#644	56
Internal Link Dist (ft)		1622			228			785			1110	
Turn Bay Length (ft)	150						500			150		150
Base Capacity (vph)	114	135		110	157		322	1080		457	1021	877
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.66	0.74		0.57	0.38		0.25	0.64		0.05	0.85	0.18

Cycle Length: 85

Actuated Cycle Length: 85

Offset: 0 (0%), Referenced to phase 4:NETL and 8:SWTL, Start of Green, Master Intersection

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.85

Intersection Signal Delay: 21.3

Intersection Capacity Utilization 76.2%

ICU Level of Service D

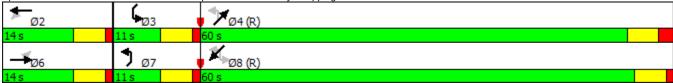
Intersection LOS: C

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Paradise Rd & Swampscott Mall Driveway/Shopping Drive



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	•	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	•	<b>†</b>	/	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	f)		7	<b>+</b>	7		41≯	7		4î.	
Traffic Volume (vph)	25	303	50	327	410	72	32	400	189	76	388	25
Future Volume (vph)	25	303	50	327	410	72	32	400	189	76	388	25
Satd. Flow (prot)	1496	1535	0	1181	1243	1338	0	2979	1285	0	2936	0
Flt Permitted	0.506			0.206				0.874			0.685	
Satd. Flow (perm)	797	1535	0	256	1243	1300	0	2614	1248	0	2028	0
Satd. Flow (RTOR)												
Lane Group Flow (vph)	27	382	0	354	444	78	0	468	205	0	529	0
Turn Type	Perm	NA		pm+pt	NA	Perm	Perm	NA	Perm	pm+pt	NA	
Protected Phases		6		5	2			4		3	8	
Permitted Phases	6			2		2	4		4	8		
Total Split (s)	33.0	33.0		25.0	58.0	58.0	29.0	29.0	29.0	13.0	42.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0		5.0	5.0		5.0	
Act Effct Green (s)	27.0	27.0		52.0	52.0	52.0		24.0	24.0		38.0	
Actuated g/C Ratio	0.27	0.27		0.52	0.52	0.52		0.24	0.24		0.38	
v/c Ratio	0.13	0.92		1.12	0.69	0.12		0.75	0.69		0.62	
Control Delay	17.8	51.7		103.2	24.6	14.9		43.6	47.9		21.9	
Queue Delay	0.0	1.2		0.5	24.1	0.0		0.0	0.0		0.0	
Total Delay	17.8	52.9		103.7	48.6	14.9		43.6	47.9		21.9	
LOS	В	D		F	D	В		D	D		С	
Approach Delay		50.6			67.9			44.9			21.9	
Approach LOS		D			Е			D			С	
Queue Length 50th (ft)	15	254		~196	198	26		145	119		63	
Queue Length 95th (ft)	m14	m#388		m#328	m317	m41		205	#215		121	
Internal Link Dist (ft)		529			213			234			571	
Turn Bay Length (ft)	150								150			
Base Capacity (vph)	223	429		317	658	689		627	299		853	
Starvation Cap Reductn	0	0		13	218	0		0	0		0	
Spillback Cap Reductn	0	7		0	0	0		0	0		0	
Storage Cap Reductn	0	0		0	0	0		0	0		0	
Reduced v/c Ratio	0.12	0.91		1.16	1.01	0.11		0.75	0.69		0.62	

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 0 (0%), Referenced to phase 4:NBTL and 8:SBTL, Start of Green, Master Intersection

Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.12 Intersection Signal Delay: 49.1 Intersection Capacity Utilization 96.8%

Intersection LOS: D
ICU Level of Service F

Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.

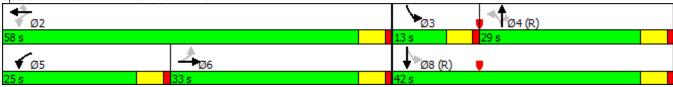
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Paradise Rd & Vinnin St



	•	•	•	<b>†</b>	<b>↓</b>	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	7	7	ř	<b>*</b>	<b>†</b>	7
Traffic Volume (vph)	263	25	9	499	456	198
Future Volume (vph)	263	25	9	499	456	198
Satd. Flow (prot)	1496	1338	1496	1574	1574	1338
Flt Permitted	0.950		0.249			
Satd. Flow (perm)	1496	1295	392	1574	1574	1269
Satd. Flow (RTOR)						
Lane Group Flow (vph)	285	27	10	540	494	214
Turn Type	Prot	Perm	pm+pt	NA	NA	Perm
Protected Phases	4		5	2	6	
Permitted Phases		4	2			6
Total Split (s)	31.0	31.0	23.0	69.0	46.0	46.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0
Act Effct Green (s)	22.7	22.7	67.3	67.3	41.0	41.0
Actuated g/C Ratio	0.23	0.23	0.67	0.67	0.41	0.41
v/c Ratio	0.84	0.09	0.02	0.51	0.77	0.41
Control Delay	33.7	21.7	7.8	19.2	34.8	24.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	33.7	21.7	7.8	19.2	34.8	24.0
LOS	С	С	Α	В	С	С
Approach Delay	32.6			19.0	31.6	
Approach LOS	С			В	С	
Queue Length 50th (ft)	51	2	1	168	265	95
Queue Length 95th (ft)	m99	m10	m4	281	399	159
Internal Link Dist (ft)	691			571	296	
Turn Bay Length (ft)		150				
Base Capacity (vph)	388	336	498	1058	645	520
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.73	0.08	0.02	0.51	0.77	0.41

Cycle Length: 100
Actuated Cycle Length: 100

Offset: 89 (89%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.84 Intersection Signal Delay: 27.4 Intersection Capacity Utilization 56.0%

Intersection LOS: C ICU Level of Service B

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Paradise Rd & Loring Ave



	_#	<b>→</b>	7	<b>*</b>	<b>←</b>	٤	•	×	/	6	×	</th
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		₩		7	f)		ř	<b></b>	*	¥	ĵ,	
Traffic Volume (vph)	2	1	2	440	5	17	2	300	330	14	195	2
Future Volume (vph)	2	1	2	440	5	17	2	300	330	14	195	2
Satd. Flow (prot)	0	1381	0	1481	1320	0	1481	1497	1326	1481	1494	0
Flt Permitted		0.961		0.753			0.601			0.237		
Satd. Flow (perm)	0	1355	0	1174	1320	0	937	1497	1326	370	1494	0
Satd. Flow (RTOR)									*200			
Lane Group Flow (vph)	0	7	0	570	28	0	3	389	428	18	256	0
Turn Type	Perm	NA		Perm	NA		Perm	NA	Perm	pm+pt	NA	
Protected Phases		6			2			4		3	8	
Permitted Phases	6			2			4		4	8		
Total Split (s)	52.0	52.0		52.0	52.0		36.0	36.0	36.0	12.0	48.0	
Total Lost Time (s)		5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Act Effct Green (s)		55.4		55.4	55.4		29.9	29.9	29.9	34.6	34.6	
Actuated g/C Ratio		0.55		0.55	0.55		0.30	0.30	0.30	0.35	0.35	
v/c Ratio		0.01		0.88	0.04		0.01	0.87	0.80	0.09	0.50	
Control Delay		13.7		33.4	7.6		24.0	54.1	29.0	5.5	13.3	
Queue Delay		0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Delay		13.7		33.4	7.6		24.0	54.1	29.0	5.5	13.3	
LOS		В		С	Α		С	D	С	Α	В	
Approach Delay		13.7			32.2			40.9			12.8	
Approach LOS		В			С			D			В	
Queue Length 50th (ft)		2		340	4		1	219	128	2	128	
Queue Length 95th (ft)		9		#518	m11		7	#311	209	m3	77	
Internal Link Dist (ft)		69			529			662			691	
Turn Bay Length (ft)				150			100		150	150		
Base Capacity (vph)		750		650	731		296	473	556	205	642	
Starvation Cap Reductn		0		0	0		0	0	0	0	0	
Spillback Cap Reductn		0		0	0		0	0	0	0	0	
Storage Cap Reductn		0		0	0		0	0	0	0	0	
Reduced v/c Ratio		0.01		0.88	0.04		0.01	0.82	0.77	0.09	0.40	

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 60 (60%), Referenced to phase 2:WBTL and 6:EBTL, Start of Green

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.88

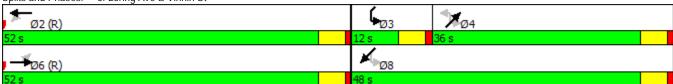
Intersection Signal Delay: 33.2

Intersection LOS: C Intersection Capacity Utilization 61.9% ICU Level of Service B

Analysis Period (min) 15

- User Entered Value
- 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Loring Ave & Vinnin St



	-	$\rightarrow$	•	<b>←</b>	•	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>*</b>	1		414	*	7
Traffic Volume (vph)	423	151	63	654	145	68
Future Volume (vph)	423	151	63	654	145	68
Satd. Flow (prot)	1231	1046	0	1522	1192	1024
Flt Permitted				0.866	0.950	
Satd. Flow (perm)	1231	993	0	1323	1177	1024
Satd. Flow (RTOR)						
Lane Group Flow (vph)	458	163	0	776	157	74
Turn Type	NA	Perm	pm+pt	NA	Prot	Prot
Protected Phases	6		5	2	4	4
Permitted Phases		6	2			
Total Split (s)	58.0	58.0	13.0	71.0	29.0	29.0
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0
Act Effct Green (s)	72.1	72.1		72.1	17.9	17.9
Actuated g/C Ratio	0.72	0.72		0.72	0.18	0.18
v/c Ratio	0.52	0.23		0.81	0.74	0.40
Control Delay	2.6	1.4		20.0	58.0	41.3
Queue Delay	1.1	0.6		0.4	1.1	0.0
Total Delay	3.7	1.9		20.4	59.1	41.3
LOS	А	Α		С	Е	D
Approach Delay	3.3			20.4	53.4	
Approach LOS	А			С	D	
Queue Length 50th (ft)	7	3		153	95	42
Queue Length 95th (ft)	m19	m7		#357	156	81
Internal Link Dist (ft)	213			312	357	
Turn Bay Length (ft)						150
Base Capacity (vph)	887	715		953	286	245
Starvation Cap Reductn	223	292		0	0	0
Spillback Cap Reductn	0	0		21	31	0
Storage Cap Reductn	0	0		0	0	0
Reduced v/c Ratio	0.69	0.39		0.83	0.62	0.30

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 75 (75%), Referenced to phase 2:WBTL and 6:EBT, Start of Green

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.81 Intersection Signal Delay: 18.5 Intersection Capacity Utilization 101.4%

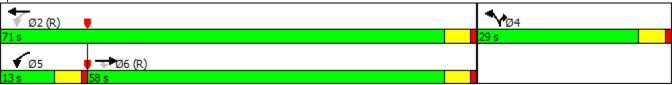
Intersection LOS: B
ICU Level of Service G

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 7: Salem St & Vinnin St



	<b>→</b>	-	4	<b>←</b>	*	4
Lane Group	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	<b>^</b>	7	*	•	¥	7
Traffic Volume (vph)	621	149	44	666	203	22
Future Volume (vph)	621	149	44	666	203	22
Satd. Flow (prot)	1589	1297	1510	1526	1510	1351
Flt Permitted			0.200		0.950	
Satd. Flow (perm)	1589	1238	318	1526	1510	1351
Satd. Flow (RTOR)						
Lane Group Flow (vph)	679	163	48	728	222	24
Turn Type	NA	Perm	pm+pt	NA	Prot	pt+ov
Protected Phases	4		3	8	2	2 3
Permitted Phases		4	8			
Total Split (s)	47.0	47.0	11.0	58.0	22.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	
Act Effct Green (s)	33.5	33.5	38.9	38.9	14.4	26.6
Actuated g/C Ratio	0.52	0.52	0.60	0.60	0.22	0.41
v/c Ratio	0.82	0.25	0.15	0.79	0.66	0.04
Control Delay	24.5	11.2	5.8	16.5	37.8	17.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	24.5	11.2	5.8	16.5	37.8	17.4
LOS	С	В	Α	В	D	В
Approach Delay	21.9			15.8	35.8	
Approach LOS	С			В	D	
Queue Length 50th (ft)	263	42	7	202	100	8
Queue Length 95th (ft)	#478	78	18	342	#197	24
Internal Link Dist (ft)	1242			517	1622	
Turn Bay Length (ft)		200	100			150
Base Capacity (vph)	1100	857	316	1221	444	541
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.62	0.19	0.15	0.60	0.50	0.04

Cycle Length: 80

Actuated Cycle Length: 64.4

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.82

Intersection Signal Delay: 21.2

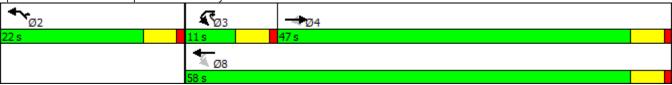
Intersection Capacity Utilization 64.1%

Intersection LOS: C
ICU Level of Service C

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 8: Swampscott Mall Driveway & Essex St



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	*	À	_	×	ን	~			
Lane Group	SET	SER	NWL	NWT	NEL	NER	Ø9		
Lane Configurations	<b>*</b>	7	ሻ	<b>*</b>	W				
Traffic Volume (vph)	680	120	37	900	15	15			
Future Volume (vph)	680	120	37	900	15	15			
Satd. Flow (prot)	1695	1501	1678	1695	1542	0			
Flt Permitted			0.277		0.976				
Satd. Flow (perm)	1695	1451	488	1695	1507	0			
Satd. Flow (RTOR)									
Lane Group Flow (vph)	776	137	42	1027	34	0			
Turn Type	NA	Perm	Perm	NA	Prot				
Protected Phases	6			2	4		9		
Permitted Phases		6	2						
Total Split (s)	50.0	50.0	50.0	50.0	15.0		25.0		
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0				
Act Effct Green (s)	52.7	52.7	52.7	52.7	9.0				
Actuated g/C Ratio	0.80	0.80	0.80	0.80	0.14				
v/c Ratio	0.57	0.12	0.11	0.76	0.16				
Control Delay	14.3	8.4	10.4	20.0	34.5				
Queue Delay	0.0	0.0	0.0	0.0	0.0				
Total Delay	14.3	8.4	10.4	20.0	34.5				
LOS	В	Α	В	С	С				
Approach Delay	13.4			19.6	34.5				
Approach LOS	В			В	С				
Queue Length 50th (ft)	0	0	0	0	9				
Queue Length 95th (ft)	#640	75	33	#956	47				
Internal Link Dist (ft)	497			268	259				
Turn Bay Length (ft)		150	150						
Base Capacity (vph)	1359	1163	391	1359	254				
Starvation Cap Reductn	11	0	0	0	0				
Spillback Cap Reductn	0	0	0	0	0				
Storage Cap Reductn	0	0	0	0	0				
Reduced v/c Ratio	0.58	0.12	0.11	0.76	0.13				

Cycle Length: 90

Actuated Cycle Length: 65.7

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.76

Intersection Signal Delay: 17.1

Intersection Capacity Utilization 64.7%

Intersection LOS: B ICU Level of Service C

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 9: Harrison Rd & Loring Ave



	*	<b>†</b>	<b>↓</b>	لر	<b>*</b>	4	
Lane Group	NBL	NBT	SBT	SBR	NEL	NER	Ø9
Lane Configurations		ર્ન	ĥ		N/		
Traffic Volume (vph)	5	939	810	12	36	11	
Future Volume (vph)	5	939	810	12	36	11	
Satd. Flow (prot)	0	1731	1727	0	1613	0	
Flt Permitted		0.996			0.963		
Satd. Flow (perm)	0	1724	1727	0	1613	0	
Satd. Flow (RTOR)							
Lane Group Flow (vph)	0	1078	938	0	54	0	
Turn Type	Perm	NA	NA		Prot		
Protected Phases		2	6		4		9
Permitted Phases	2						
Total Split (s)	54.0	54.0	54.0		12.0		19.0
Total Lost Time (s)		6.0	5.0		5.0		
Act Effct Green (s)		57.7	58.3		6.8		
Actuated g/C Ratio		0.77	0.78		0.09		
v/c Ratio		0.81	0.70		0.37		
Control Delay		20.2	14.9		42.2		
Queue Delay		0.7	0.0		0.0		
Total Delay		20.9	14.9		42.2		
LOS		С	В		D		
Approach Delay		20.9	14.9		42.2		
Approach LOS		С	В		D		
Queue Length 50th (ft)		241	157		21		
Queue Length 95th (ft)		#900	#722		65		
Internal Link Dist (ft)		497	670		323		
Turn Bay Length (ft)							
Base Capacity (vph)		1329	1345		152		
Starvation Cap Reductn		66	0		0		
Spillback Cap Reductn		0	0		0		
Storage Cap Reductn		0	0		0		
Reduced v/c Ratio		0.85	0.70		0.36		

Cycle Length: 85

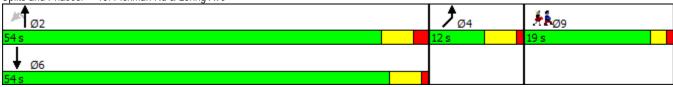
Actuated Cycle Length: 74.8 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.81

Intersection Signal Delay: 18.7 Intersection LOS: B
Intersection Capacity Utilization 70.2% ICU Level of Service C

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 10: Pickman Rd & Loring Ave



	<b>→</b>	¬₄	~	•	*	4
Movement	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	<b>†</b>	7	ሻ	<b>*</b>	14	
Traffic Volume (veh/h)	785	25	67	628	5	152
Future Volume (Veh/h)	785	25	67	628	5	152
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Hourly flow rate (vph)	859	27	73	687	5	166
Pedestrians	5			5	5	
Lane Width (ft)	11.0			11.0	11.0	
Walking Speed (ft/s)	3.0			3.0	3.0	
Percent Blockage	1			1	1	
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)				914		
pX, platoon unblocked					0.76	
vC, conflicting volume			891		1702	869
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			891		1765	869
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			90		92	52
cM capacity (veh/h)			749		62	345
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NW 1	
Volume Total	859	27	73	687	171	
Volume Left	0	0	73	0	5	
Volume Right	0	27	0	0	166	
cSH	1700	1700	749	1700	304	
Volume to Capacity	0.51	0.02	0.10	0.40	0.56	
Queue Length 95th (ft)	0	0.02	8	0.10	81	
Control Delay (s)	0.0	0.0	10.3	0.0	31.0	
Lane LOS	0.0	0.0	В	0.0	D	
Approach Delay (s)	0.0		1.0		31.0	
Approach LOS	0.0		1.0		D D	
Intersection Summary						
Average Delay			3.3			
Intersection Capacity Utilization			68.4%	IC	U Level of	Service
Analysis Period (min)			15			

12: Todocco Ct & Loggo							
	ၨ	_	←	•	_	1	
		<b>→</b>		_	-	•	
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		41	1>		W		
Traffic Volume (veh/h)	162	352	617	174	73	62	
Future Volume (Veh/h)	162	352	617	174	73	62	
Sign Control	102	Free	Free	177	Stop	02	
Grade		0%	0%		0%		
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	
Hourly flow rate (vph)	187	406	712	201	84	72	
Pedestrians	107	10	10	201	10	12	
Lane Width (ft)		11.0	11.0		11.0		
Walking Speed (ft/s)		3.0	3.0		3.0		
Percent Blockage		1	1		1		
Right turn flare (veh)							
Median type		None	None				
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	923				1612	832	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	923				1612	832	
tC, single (s)	4.1				6.4	6.2	
tC, 2 stage (s)							
tF (s)	2.2				3.5	3.3	
p0 queue free %	74				0	80	
cM capacity (veh/h)	724				82	358	
		WD 4	00.4				
Direction, Lane #	EB 1	WB 1	SB 1				
Volume Total	593	913	156				
Volume Left	187	0	84				
Volume Right	0	201	72				
cSH	724	1700	128				
Volume to Capacity	0.26	0.54	1.22				
Queue Length 95th (ft)	26	0	240				
Control Delay (s)	6.4	0.0	216.6				
Lane LOS	Α		F				
Approach Delay (s)	6.4	0.0	216.6				
Approach LOS			F				
Intersection Summary							
Average Delay			22.6				
			94.3%	10	'III ovol of	Convice	
Intersection Capacity Utilization				IC	U Level of	Service	
Analysis Period (min)			15				

	(w	لر	<b>*</b>	×	×	t
Movement	SBL	SBR	NEL	NET	SWT	SWR
Lane Configurations	W			र्स	<b>f</b> a	
Traffic Volume (veh/h)	88	28	23	428	666	145
Future Volume (Veh/h)	88	28	23	428	666	145
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	99	32	26	483	752	164
Pedestrians	30			30	30	
Lane Width (ft)	11.0			11.0	11.0	
Walking Speed (ft/s)	3.0			3.0	3.0	
Percent Blockage	3			3	3	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				110110	110110	
Upstream signal (ft)				706		
pX, platoon unblocked	0.85			700		
vC, conflicting volume	1429	894	946			
vC1, stage 1 conf vol	,	٠,٠	, , , ,			
vC2, stage 2 conf vol						
vCu, unblocked vol	1417	894	946			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	15	90	96			
cM capacity (veh/h)	116	319	703			
Direction, Lane #	SB 1	NE 1	SW 1			
Volume Total	131	509	916			
Volume Left	99	26	0			
Volume Right	32	0	164			
cSH	138	703	1700			
Volume to Capacity	0.95	0.04	0.54			
Queue Length 95th (ft)	165	3	0.54			
	126.5	1.0	0.0			
Control Delay (s) Lane LOS	126.5 F	1.0 A	0.0			
	126.5	1.0	0.0			
Approach LOS	126.5 F	1.0	0.0			
Approach LOS	F					
Intersection Summary						
Average Delay			11.0			
Intersection Capacity Utilization			64.4%	IC	U Level of	Service
Analysis Period (min)			15			

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	•	*	4	<b>†</b>	<b>↓</b>	4	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	W			ર્ની	₽		
Traffic Volume (veh/h)	8	63	193	212	189	52	
Future Volume (Veh/h)	8	63	193	212	189	52	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	9	72	220	242	216	59	
Pedestrians							
Lane Width (ft)							
Valking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median type Median storage veh)				110110	140110		
Jpstream signal (ft)					437		
oX, platoon unblocked					737		
/C, conflicting volume	928	246	275				
/C1, stage 1 conf vol	720	240	210				
C2, stage 2 conf vol							
/Cu, unblocked vol	928	246	275				
C, single (s)	6.4	6.2	*6.4				
C, 2 stage (s)	0.7	0.2	0.7				
F (s)	3.5	3.3	*3.3				
oO queue free %	96	91	71				
cM capacity (veh/h)	211	793	757				
Direction, Lane # /olume Total	EB 1	NB 1	SB 1				
	81	462	275				
/olume Left	9	220	0				
/olume Right	72	0	59				
SH "	607	757	1700				
Volume to Capacity	0.13	0.29	0.16				
Queue Length 95th (ft)	11	30	0				
Control Delay (s)	11.8	7.6	0.0				
Lane LOS	В	A	0.0				
Approach Delay (s)	11.8	7.6	0.0				
Approach LOS	В						
ntersection Summary							
Average Delay			5.5				
Intersection Capacity Utilization			51.2%	IC	CU Level of S	Service	Α
Analysis Period (min)			15				
User Entered Value							

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Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		₩			4			4			4	
Traffic Volume (vph)	28	20	2	1	8	2	2	625	13	5	420	15
Future Volume (vph)	28	20	2	1	8	2	2	625	13	5	420	15
Satd. Flow (prot)	0	1743	0	0	1752	0	0	1795	0	0	1790	0
Flt Permitted		0.976			0.963			0.999			0.992	
Satd. Flow (perm)	0	1749	0	0	1694	0	0	1793	0	0	1777	0
Satd. Flow (RTOR)		2			2			2			3	
Lane Group Flow (vph)	0	57	0	0	12	0	0	723	0	0	497	0
Turn Type	Perm	NA										
Protected Phases		6			2			4			8	
Permitted Phases	6			2			4			8		
Total Split (s)	12.0	12.0		12.0	12.0		40.0	40.0		40.0	40.0	
Total Lost Time (s)		6.0			6.0			6.0			6.0	
Act Effct Green (s)		6.4			6.4			36.1			36.1	
Actuated g/C Ratio		0.13			0.13			0.74			0.74	
v/c Ratio		0.24			0.05			0.54			0.38	
Control Delay		27.1			24.6			11.5			8.3	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		27.1			24.6			11.5			8.3	
LOS		С			С			В			Α	
Approach Delay		27.1			24.6			11.5			8.3	
Approach LOS		С			С			В			Α	
Queue Length 50th (ft)		14			3			101			57	
Queue Length 95th (ft)		60			20			#515			264	
Internal Link Dist (ft)		155			218			904			626	
Turn Bay Length (ft)												
Base Capacity (vph)		233			226			1345			1333	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.24			0.05			0.54			0.37	

Cycle Length: 75

Actuated Cycle Length: 48.7

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.54 Intersection Signal Delay: 11.1

Intersection Capacity Utilization 56.1%

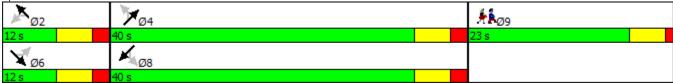
Intersection LOS: B ICU Level of Service B

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.





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Lane Group	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	7	7	ĵ,		¥	<b>+</b>
Traffic Volume (vph)	99	100	722	23	145	604
Future Volume (vph)	99	100	722	23	145	604
Satd. Flow (prot)	1540	1378	1274	0	1215	1279
Flt Permitted	0.950				0.188	
Satd. Flow (perm)	1540	1378	1274	0	241	1279
Satd. Flow (RTOR)						
Lane Group Flow (vph)	108	109	815	0	159	661
Turn Type	Prot	pt+ov	NA		pm+pt	NA
Protected Phases	2	2 3	4		3	8
Permitted Phases					8	
Total Split (s)	19.0		59.0		12.0	71.0
Total Lost Time (s)	5.0		5.0		6.0	5.0
Act Effct Green (s)	11.1	24.3	55.7		67.9	68.9
Actuated g/C Ratio	0.12	0.27	0.62		0.75	0.77
v/c Ratio	0.57	0.29	1.03		0.61	0.68
Control Delay	48.5	27.8	61.7		15.6	6.5
Queue Delay	0.0	0.0	0.0		0.0	0.0
Total Delay	48.5	27.8	61.7		15.6	6.5
LOS	D	С	Е		В	Α
Approach Delay	38.1		61.7			8.2
Approach LOS	D		Е			Α
Queue Length 50th (ft)	58	48	~517		15	71
Queue Length 95th (ft)	108	92	#740		m40	m144
Internal Link Dist (ft)	133		711			783
Turn Bay Length (ft)					150	
Base Capacity (vph)	239	349	788		260	979
Starvation Cap Reductn	0	0	0		0	0
Spillback Cap Reductn	0	0	0		0	0
Storage Cap Reductn	0	0	0		0	0
Reduced v/c Ratio	0.45	0.31	1.03		0.61	0.68

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 70 (78%), Referenced to phase 4:NET and 8:SWTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.03

Intersection Signal Delay: 35.3

Intersection LOS: D ICU Level of Service E

Intersection Capacity Utilization 89.8%

Analysis Period (min) 15

- Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.
- 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Paradise Rd & Vinnin Ligour Dr



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3: Paradise Rd & Swampscott Mall Driveway/Shopping Drive	Route 1A-virini Square Friority Corndor Study	
	3: Paradise Rd & Swampscott Mall Driveway/Shopping Drive	

	_≉	<b>→</b>	7	*	<b>←</b>	٣	•	×	/	6	×	</th
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	7	ĵ,		¥	ą.		¥	ą.		ř	<b>*</b>	7
Traffic Volume (vph)	196	60	182	90	87	25	181	695	60	39	494	144
Future Volume (vph)	196	60	182	90	87	25	181	695	60	39	494	144
Satd. Flow (prot)	1540	1389	0	1540	1552	0	1215	1234	0	1215	1254	1066
Flt Permitted	0.672			0.356			0.427			0.119		
Satd. Flow (perm)	1089	1389	0	577	1552	0	546	1234	0	152	1254	1018
Satd. Flow (RTOR)												
Lane Group Flow (vph)	212	262	0	97	121	0	196	817	0	42	535	156
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	Perm
Protected Phases		6			2		7	4		3	8	
Permitted Phases	6			2			4			8		8
Total Split (s)	23.0	23.0		23.0	23.0		11.0	56.0		11.0	56.0	56.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	6.0		5.0	5.0	5.0
Act Effct Green (s)	18.0	18.0		18.0	18.0		55.4	54.4		51.0	51.0	51.0
Actuated g/C Ratio	0.20	0.20		0.20	0.20		0.62	0.60		0.57	0.57	0.57
v/c Ratio	0.98	0.95		0.84	0.39		0.52	1.10		0.27	0.75	0.27
Control Delay	94.2	79.8		88.3	35.6		8.7	70.0		13.2	23.2	11.5
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Delay	94.2	79.8		88.3	35.6		8.7	70.0		13.2	23.2	11.5
LOS	F	Е		F	D		Α	Е		В	С	В
Approach Delay		86.2			59.1			58.1			20.2	
Approach LOS		F			Е			Е			С	
Queue Length 50th (ft)	121	148		53	60		19	~563		10	214	42
Queue Length 95th (ft)	#261	#297		#145	113		m20	m#587		24	361	79
Internal Link Dist (ft)		1630			222			783			1143	
Turn Bay Length (ft)	150						500			150		150
Base Capacity (vph)	217	277		115	310		380	745		157	710	576
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.98	0.95		0.84	0.39		0.52	1.10		0.27	0.75	0.27

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 4:NETL and 8:SWTL, Start of Green, Master Intersection

Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.10

Intersection Signal Delay: 52.3

Intersection LOS: D
ICU Level of Service G

Intersection Capacity Utilization 104.5%

Analysis Period (min) 15

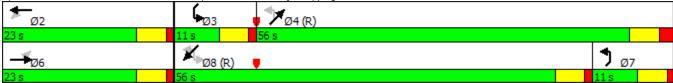
Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Paradise Rd & Swampscott Mall Driveway/Shopping Drive



	•	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	4	<b>†</b>	/	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Ť	f)		7	<b>+</b>	7		41≯	7		4î}∍	
Traffic Volume (vph)	11	376	59	217	310	64	32	520	358	99	441	16
Future Volume (vph)	11	376	59	217	310	64	32	520	358	99	441	16
Satd. Flow (prot)	1215	1251	0	1215	1279	1088	0	2424	1088	0	2398	0
Flt Permitted	0.555			0.187				0.884			0.601	
Satd. Flow (perm)	710	1251	0	239	1279	1073	0	2149	1069	0	1454	0
Satd. Flow (RTOR)									*100			
Lane Group Flow (vph)	12	481	0	240	343	71	0	610	396	0	614	0
Turn Type	Perm	NA		pm+pt	NA	Perm	Perm	NA	Perm	pm+pt	NA	
Protected Phases		6		5	2			4		3	8	
Permitted Phases	6			2		2	4		4	8		
Total Split (s)	39.0	39.0		15.0	54.0	54.0	35.0	35.0	35.0	11.0	46.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0		5.0	5.0		5.0	
Act Effct Green (s)	34.0	34.0		49.0	49.0	49.0		30.0	30.0		41.0	
Actuated g/C Ratio	0.34	0.34		0.49	0.49	0.49		0.30	0.30		0.41	
v/c Ratio	0.05	1.13		1.12	0.55	0.14		0.95	1.02		0.94	
Control Delay	9.6	96.3		122.2	18.7	13.9		60.0	77.4		47.2	
Queue Delay	0.0	0.3		0.0	3.3	0.0		0.0	0.7		0.0	
Total Delay	9.6	96.7		122.2	22.0	13.9		60.0	78.1		47.2	
LOS	А	F		F	С	В		Е	Е		D	
Approach Delay		94.5			57.9			67.2			47.2	
Approach LOS		F			Е			Е			D	
Queue Length 50th (ft)	3	~369		~116	108	19		199	~209		127	
Queue Length 95th (ft)	m3	m#420		#277	190	m46		#313	#406		m#193	
Internal Link Dist (ft)		529			213			193			571	
Turn Bay Length (ft)	150								150			
Base Capacity (vph)	241	425		214	626	525		644	390		652	
Starvation Cap Reductn	0	0		0	189	0		0	0		0	
Spillback Cap Reductn	0	14		0	0	0		0	1		0	
Storage Cap Reductn	0	0		0	0	0		0	0		0	
Reduced v/c Ratio	0.05	1.17		1.12	0.78	0.14		0.95	1.02		0.94	

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 0 (0%), Referenced to phase 4:NBTL and 8:SBTL, Start of Green, Master Intersection

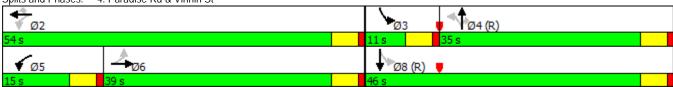
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 1.13
Intersection Signal Delay: 65.4
Intersection Capacity Utilization 114.6%

Intersection LOS: E ICU Level of Service H

Analysis Period (min) 15

- User Entered Value
- Volume exceeds capacity, queue is theoretically infinite.
   Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Paradise Rd & Vinnin St



	•	•	•	<b>†</b>	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	7	7	7	<b>+</b>	•	7
Traffic Volume (vph)	287	24	15	573	521	289
Future Volume (vph)	287	24	15	573	521	289
Satd. Flow (prot)	1191	1378	1540	1588	1588	1350
Flt Permitted	0.950		0.196			
Satd. Flow (perm)	1191	1338	318	1588	1588	1286
Satd. Flow (RTOR)						
Lane Group Flow (vph)	331	28	17	661	601	333
Turn Type	Prot	Perm	pm+pt	NA	NA	Perm
Protected Phases	4		5	2	6	
Permitted Phases		4	2			6
Total Split (s)	39.0	39.0	11.0	61.0	50.0	50.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0
Act Effct Green (s)	30.9	30.9	59.1	59.1	45.0	45.0
Actuated g/C Ratio	0.31	0.31	0.59	0.59	0.45	0.45
v/c Ratio	0.90	0.07	0.06	0.70	0.84	0.58
Control Delay	36.2	5.2	3.3	16.7	37.1	25.3
Queue Delay	0.0	0.0	0.0	0.1	0.0	0.0
Total Delay	36.2	5.2	3.3	16.8	37.1	25.3
LOS	D	А	А	В	D	С
Approach Delay	33.8			16.5	32.9	
Approach LOS	С			В	С	
Queue Length 50th (ft)	19	1	1	109	329	153
Queue Length 95th (ft)	#338	m2	m2	m151	#536	246
Internal Link Dist (ft)	691			571	296	
Turn Bay Length (ft)		150				
Base Capacity (vph)	404	454	299	938	714	578
Starvation Cap Reductn	0	0	0	20	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.82	0.06	0.06	0.72	0.84	0.58

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 99 (99%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

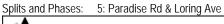
Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.90 Intersection Signal Delay: 27.4 Intersection Capacity Utilization 67.0%

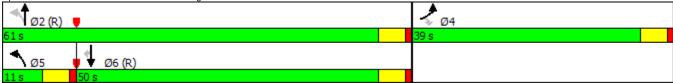
Intersection LOS: C
ICU Level of Service C

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.





	_≉	-	7	<b>*</b>	<b>←</b>	٤	•	×	/	6	×	</th
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		₩		¥	f)		ř	<b></b>	*	¥	ĵ,	
Traffic Volume (vph)	2	1	2	314	6	25	7	298	440	25	292	4
Future Volume (vph)	2	1	2	314	6	25	7	298	440	25	292	4
Satd. Flow (prot)	0	1445	0	1215	1060	0	1296	1337	1160	1296	1334	0
Flt Permitted		0.965		0.754			0.557			0.352		
Satd. Flow (perm)	0	1422	0	965	1060	0	760	1337	1105	480	1334	0
Satd. Flow (RTOR)									*200			
Lane Group Flow (vph)	0	5	0	358	36	0	8	340	502	29	338	0
Turn Type	Perm	NA		Perm	NA		Perm	NA	Perm	pm+pt	NA	
Protected Phases		6			2			4		3	8	
Permitted Phases	6			2			4		4	8		
Total Split (s)	48.0	48.0		48.0	48.0		41.0	41.0	41.0	11.0	52.0	
Total Lost Time (s)		5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Act Effct Green (s)		47.7		47.7	47.7		35.7	35.7	35.7	42.3	42.3	
Actuated g/C Ratio		0.48		0.48	0.48		0.36	0.36	0.36	0.42	0.42	
v/c Ratio		0.01		0.78	0.07		0.03	0.71	0.96	0.12	0.60	
Control Delay		16.4		33.0	11.4		21.3	37.2	51.0	1.8	7.1	
Queue Delay		0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Delay		16.4		33.0	11.4		21.3	37.2	51.0	1.8	7.1	
LOS		В		С	В		С	D	D	Α	Α	
Approach Delay		16.4			31.0			45.2			6.7	
Approach LOS		В			С			D			А	
Queue Length 50th (ft)		2		245	15		3	184	208	1	10	
Queue Length 95th (ft)		9		m#371	m22		14	292	#432	m1	9	
Internal Link Dist (ft)		69			529			965			691	
Turn Bay Length (ft)				150			100		150	150		
Base Capacity (vph)		677		459	505		280	494	534	252	626	
Starvation Cap Reductn		0		0	0		0	0	0	0	0	
Spillback Cap Reductn		0		0	0		0	0	0	0	0	
Storage Cap Reductn		0		0	0		0	0	0	0	0	
Reduced v/c Ratio		0.01		0.78	0.07		0.03	0.69	0.94	0.12	0.54	

Cycle Length: 100
Actuated Cycle Length: 100

Offset: 69 (69%), Referenced to phase 2:WBTL and 6:EBTL, Start of Green

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.96 Intersection Signal Delay: 32.9

Intersection LOS: C ICU Level of Service C

Intersection Capacity Utilization 71.3%

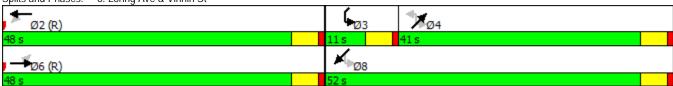
Analysis Period (min) 15

* User Entered Value

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Loring Ave & Vinnin St



	-	•	•	•	•	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>^</b>	7		41⊁	Ť	7
Traffic Volume (vph)	648	197	70	441	121	97
Future Volume (vph)	648	197	70	441	121	97
Satd. Flow (prot)	1588	1378	0	2414	1191	1088
Flt Permitted				0.771	0.950	
Satd. Flow (perm)	1588	1378	0	1874	1191	1088
Satd. Flow (RTOR)						
Lane Group Flow (vph)	716	218	0	564	134	107
Turn Type	NA	Perm	pm+pt	NA	Prot	Prot
Protected Phases	6		5	2	4	4
Permitted Phases		6	2			
Total Split (s)	65.0	65.0	11.0	76.0	24.0	24.0
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0
Act Effct Green (s)	74.6	74.6		74.6	15.4	15.4
Actuated g/C Ratio	0.75	0.75		0.75	0.15	0.15
v/c Ratio	0.60	0.21		0.40	0.73	0.64
Control Delay	2.7	1.3		6.1	62.5	56.5
Queue Delay	11.3	1.0		0.0	0.4	0.0
Total Delay	13.9	2.3		6.1	62.9	56.5
LOS	В	А		Α	Е	Е
Approach Delay	11.2			6.1	60.0	
Approach LOS	В			Α	Е	
Queue Length 50th (ft)	44	6		60	81	64
Queue Length 95th (ft)	m83	m13		97	143	119
Internal Link Dist (ft)	213			175	347	
Turn Bay Length (ft)						150
Base Capacity (vph)	1184	1028		1398	226	206
Starvation Cap Reductn	444	586		0	0	0
Spillback Cap Reductn	0	0		0	7	0
Storage Cap Reductn	0	0		0	0	0
Reduced v/c Ratio	0.97	0.49		0.40	0.61	0.52

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 76 (76%), Referenced to phase 2:WBTL and 6:EBT, Start of Green

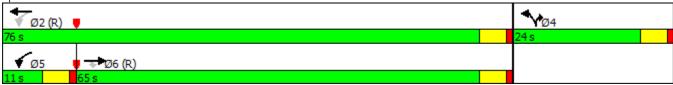
Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.73 Intersection Signal Delay: 16.3 Intersection Capacity Utilization 83.2%

Intersection LOS: B
ICU Level of Service E

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 7: Salem St & Vinnin St



	-	•	•	•	<b>~</b>	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>^</b>	7	*	<b></b>	7	7
Traffic Volume (vph)	693	254	66	631	259	108
Future Volume (vph)	693	254	66	631	259	108
Satd. Flow (prot)	1459	1240	1540	1588	1540	1378
Flt Permitted			0.171		0.950	
Satd. Flow (perm)	1459	1240	277	1588	1540	1378
Satd. Flow (RTOR)						
Lane Group Flow (vph)	782	287	75	712	292	122
Turn Type	NA	Perm	pm+pt	NA	Prot	pt+ov
Protected Phases	4		3	8	2	23
Permitted Phases		4	8			
Total Split (s)	65.0	65.0	11.0	76.0	29.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	
Act Effct Green (s)	56.2	56.2	67.4	67.4	21.8	32.9
Actuated g/C Ratio	0.57	0.57	0.68	0.68	0.22	0.33
v/c Ratio	0.95	0.41	0.28	0.66	0.86	0.27
Control Delay	42.7	14.5	8.5	13.3	63.5	27.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	42.7	14.5	8.5	13.3	63.5	27.4
LOS	D	В	Α	В	Е	С
Approach Delay	35.1			12.9	52.9	
Approach LOS	D			В	D	
Queue Length 50th (ft)	457	101	15	251	188	59
Queue Length 95th (ft)	#743	163	30	377	#330	107
Internal Link Dist (ft)	1242			509	1630	
Turn Bay Length (ft)		200	100			150
Base Capacity (vph)	893	759	265	1150	377	448
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.88	0.38	0.28	0.62	0.77	0.27

Cycle Length: 105
Actuated Cycle Length: 99.3

Control Type: Actuated-Uncoordinated

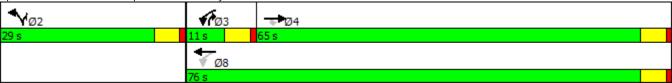
Maximum v/c Ratio: 0.95 Intersection Signal Delay: 30.6 Intersection Capacity Utilization 76.8%

Intersection LOS: C
ICU Level of Service D

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 8: Swampscott Mall Driveway & Essex St



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	*	À	_	×	ን	~			
Lane Group	SET	SER	NWL	NWT	NEL	NER	Ø9		
Lane Configurations	<b>*</b>	7	ሻ	<b>^</b>	W				
Traffic Volume (vph)	972	120	37	826	10	15			
Future Volume (vph)	972	120	37	826	10	15			
Satd. Flow (prot)	1493	1218	1621	1433	1622	0			
Flt Permitted			0.149		0.981				
Satd. Flow (perm)	1493	1183	254	1433	1622	0			
Satd. Flow (RTOR)									
Lane Group Flow (vph)	1109	137	42	943	28	0			
Turn Type	NA	Perm	Perm	NA	Prot				
Protected Phases	6			2	4		9		
Permitted Phases		6	2						
Total Split (s)	62.0	62.0	62.0	62.0	13.0		25.0		
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0				
Act Effct Green (s)	62.5	62.5	62.5	62.5	8.3				
Actuated g/C Ratio	0.87	0.87	0.87	0.87	0.11				
v/c Ratio	0.86	0.13	0.19	0.76	0.15				
Control Delay	19.4	4.9	9.0	14.7	37.0				
Queue Delay	0.1	0.0	0.0	0.0	0.0				
Total Delay	19.5	4.9	9.0	14.7	37.0				
LOS	В	Α	Α	В	D				
Approach Delay	17.9			14.5	37.0				
Approach LOS	В			В	D				
Queue Length 50th (ft)	0	0	0	0	10				
Queue Length 95th (ft)	#1159	73	39	#951	45				
Internal Link Dist (ft)	486			296	259				
Turn Bay Length (ft)		150	150						
Base Capacity (vph)	1293	1024	220	1241	186				
Starvation Cap Reductn	5	0	0	0	0				
Spillback Cap Reductn	0	0	0	0	0				
Storage Cap Reductn	0	0	0	0	0				
Reduced v/c Ratio	0.86	0.13	0.19	0.76	0.15				

Cycle Length: 100 Actuated Cycle Length: 72.2

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.86 Intersection Signal Delay: 16.6 Intersection Capacity Utilization 71.7%

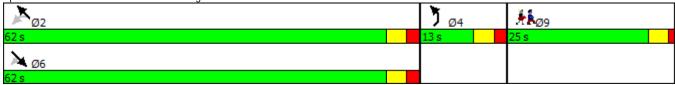
Intersection LOS: B ICU Level of Service C

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 9: Harrison Rd & Loring Ave



	*	<b>†</b>	<b>↓</b>	لر	<b>*</b>	4		
Lane Group	NBL	NBT	SBT	SBR	NEL	NER	Ø9	
Lane Configurations		ર્ન	ĵ.		N/A			
Traffic Volume (vph)	14	786	1044	25	22	11		
Future Volume (vph)	14	786	1044	25	22	11		
Satd. Flow (prot)	0	1511	1508	0	1663	0		
Flt Permitted		0.753			0.968			
Satd. Flow (perm)	0	1139	1508	0	1663	0		
Satd. Flow (RTOR)								
Lane Group Flow (vph)	0	913	1221	0	38	0		
Turn Type	Perm	NA	NA		Prot			
Protected Phases		2	6		4		9	
Permitted Phases	2							
Total Split (s)	54.0	54.0	54.0		12.0		24.0	
Total Lost Time (s)		6.0	5.0		5.0			
Act Effct Green (s)		54.0	54.3		6.8			
Actuated g/C Ratio		0.85	0.86		0.11			
v/c Ratio		0.94	0.94		0.21			
Control Delay		31.7	27.8		33.9			
Queue Delay		0.0	0.0		0.0			
Total Delay		31.7	27.8		33.9			
LOS		С	С		С			
Approach Delay		31.7	27.8		33.9			
Approach LOS		С	С		С			
Queue Length 50th (ft)		0	0		11			
Queue Length 95th (ft)		#931	#1179		51			
Internal Link Dist (ft)		486	689		323			
Turn Bay Length (ft)								
Base Capacity (vph)		973	1296		191			
Starvation Cap Reductn		0	0		0			
Spillback Cap Reductn		0	0		0			
Storage Cap Reductn		0	0		0			
Reduced v/c Ratio		0.94	0.94		0.20			

Cycle Length: 90

Actuated Cycle Length: 63.2

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.94

Intersection Signal Delay: 29.5

Intersection Capacity Utilization 72.6%

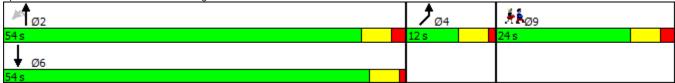
Intersection LOS: C ICU Level of Service C

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.





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	-	¬₄	~	<b>←</b>	•	4
Movement	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	<b>^</b>	7	7	<b>†</b>	W	
Traffic Volume (veh/h)	774	46	164	823	12	89
Future Volume (Veh/h)	774	46	164	823	12	89
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	838	50	178	891	13	96
Pedestrians	10				10	
Lane Width (ft)	11.0				11.0	
Walking Speed (ft/s)	3.0				3.0	
Percent Blockage	1				1	
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)				922		
pX, platoon unblocked					0.40	
vC, conflicting volume			898		2105	848
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			898		3026	848
tC, single (s)			4.1		*6.0	*6.0
tC, 2 stage (s)						
tF (s)			2.2		*3.0	*3.0
p0 queue free %			76		0	76
cM capacity (veh/h)			748		6	403
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NW 1	
Volume Total	838	50	178	891	109	
Volume Left	0	0	178	0	13	
Volume Right	0	50	0	0	96	
cSH	1700	1700	748	1700	47	
Volume to Capacity	0.49	0.03	0.24	0.52	2.31	
Queue Length 95th (ft)	0.49	0.03	23	0.52	2.31	
Control Delay (s)	0.0	0.0	11.3	0.0	781.6	
Lane LOS	0.0	0.0	11.3 B	0.0	701.0 F	
Approach Delay (s)	0.0		1.9		781.6	
Approach LOS	0.0		1.9		781.0 F	
					F	
Intersection Summary						
Average Delay			42.2			
Intersection Capacity Utilization			68.8%	IC	U Level of	Service
Analysis Period (min)			15			
* User Entered Value						

	۶	<b>→</b>	<b>←</b>	4	<b>\</b>	4
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	<b>1</b> >		W	
Traffic Volume (veh/h)	133	534	436	100	112	187
Future Volume (Veh/h)	133	534	436	100	112	187
Sign Control	100	Free	Free	100	Stop	107
Grade		0%	0%		0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
	150	603	492		126	211
Hourly flow rate (vph) Pedestrians	100	003	492	113	120	211
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	605				1452	548
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	605				1452	548
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	85				0	61
cM capacity (veh/h)	973				122	536
		WD.	00.4		122	330
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	753	605	337			
Volume Left	150	0	126			
Volume Right	0	113	211			
cSH	973	1700	236			
Volume to Capacity	0.15	0.36	1.43			
Queue Length 95th (ft)	14	0	481			
Control Delay (s)	3.7	0.0	255.2			
Lane LOS	Α		F			
Approach Delay (s)	3.7	0.0	255.2			
Approach LOS			F			
Intersection Summary						
Average Delay			52.4			
Intersection Capacity Utilization			96.3%	IC	U Level of	Sorvico
				IC	O LEVELOI	JEI VILE
Analysis Period (min)			15			

	<b>→</b>	74	4	<b>←</b>	*	4
Movement	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	f.			4	W	
Traffic Volume (veh/h)	615	139	59	564	47	52
Future Volume (Veh/h)	615	139	59	564	47	52
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	694	157	67	637	53	59
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)	140110			140110		
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			851		1544	772
vC1, stage 1 conf vol			001		1011	772
vC2, stage 2 conf vol						
vCu, unblocked vol			851		1544	772
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)			т. і		0.7	٥.۷
tF (s)			2.2		3.5	3.3
p0 queue free %			91		54	85
cM capacity (veh/h)			788		116	399
					110	377
Direction, Lane #	EB 1	WB 1	NW 1			
Volume Total	851	704	112			
Volume Left	0	67	53			
Volume Right	157	0	59			
cSH	1700	788	185			
Volume to Capacity	0.50	0.09	0.61			
Queue Length 95th (ft)	0	7	84			
Control Delay (s)	0.0	2.2	50.6			
Lane LOS		Α	F			
Approach Delay (s)	0.0	2.2	50.6			
Approach LOS			F			
Intersection Summary						
Average Delay			4.3			
Intersection Capacity Utilization			93.5%	IC	U Level of	Service
Analysis Period (min)			15			
. a.a.jo.o i onoa (iiiii)						

10. Calom Ct							
	ၨ	•	•	<b>†</b>	<b>↓</b>	1	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	W	LDIX	IVDL	<u> </u>	<u> </u>	JDIN	
Traffic Volume (veh/h)	73	107	83	<b>T</b> 161	<b>T</b> 237	86	
Future Volume (Veh/h)	73	107	83	161	237	86	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	83	122	95	184	270	98	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (ft)					427		
pX, platoon unblocked							
vC, conflicting volume	693	319	368				
vC1, stage 1 conf vol	073	317	300				
vC2, stage 2 conf vol							
vCu, unblocked vol	693	319	368				
tC, single (s)	6.4	6.2	4.1				
	0.4	0.2	4.1				
tC, 2 stage (s)	٥٦	0.0	0.0				
tF (s)	3.5	3.3	2.2				
p0 queue free %	78	83	92				
cM capacity (veh/h)	377	722	1191				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	205	279	368				
Volume Left	83	95	0				
Volume Right	122	0	98				
cSH	526	1191	1700				
Volume to Capacity	0.39	0.08	0.22				
Queue Length 95th (ft)	46	6	0.22				
Control Delay (s)	16.1	3.3	0.0				
Lane LOS	C	3.3 A	0.0				
	16.1	3.3	0.0				
Approach LOS	16.1 C	3.3	0.0				
Approach LOS	C						
Intersection Summary							
Average Delay			5.0				
Intersection Capacity Utilization			53.5%	IC	U Level of S	Service	
Analysis Period (min)			15				

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Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		€}-			4			4			4	
Traffic Volume (vph)	19	7	3	0	5	1	0	465	0	1	437	1
Future Volume (vph)	19	7	3	0	5	1	0	465	0	1	437	1
Satd. Flow (prot) Flt Permitted	0	1720	0	0	1766	0	0	1801	0	0	1801 0.999	0
Satd. Flow (perm)	0	1777	0	0	1766	0	0	1801	0	0	1799	0
Satd. Flow (RTOR)	•	3	•	•	1	•	•			· ·		
Lane Group Flow (vph)	0	32	0	0	7	0	0	525	0	0	495	0
Turn Type	Perm	NA	-	-	NA	•	-	NA	•	Perm	NA	-
Protected Phases		6			2			4			8	
Permitted Phases	6			2			4			8		
Total Split (s)	13.0	13.0		13.0	13.0		39.0	39.0		39.0	39.0	
Total Lost Time (s)		6.0			6.0			6.0			6.0	
Act Effct Green (s)		6.9			6.7			38.9			38.9	
Actuated g/C Ratio		0.16			0.15			0.89			0.89	
v/c Ratio		0.11			0.03			0.33			0.31	
Control Delay		21.1			21.6			6.2			6.0	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		21.1			21.6			6.2			6.0	
LOS		С			С			Α			Α	
Approach Delay		21.1			21.6			6.2			6.0	
Approach LOS		С			С			Α			Α	
Queue Length 50th (ft)		5			1			0			0	
Queue Length 95th (ft)		38			14			291			269	
Internal Link Dist (ft)		155			218			904			594	
Turn Bay Length (ft)												
Base Capacity (vph)		311			308			1478			1477	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.10			0.02			0.36			0.34	

Cycle Length: 75

Actuated Cycle Length: 43.8

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.33

Intersection Signal Delay: 6.7
Intersection Capacity Utilization 44.0%

Analysis Period (min) 15

n 44.0% ICU Level of Service A

Splits and Phases: 1: Paradise Rd & Ellis Rd



Intersection LOS: A

	<b>F</b>	(	×	~	Ĺ	×
Lane Group	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	ሻ	7	f)		¥	<b></b>
Traffic Volume (vph)	84	195	654	14	182	664
Future Volume (vph)	84	195	654	14	182	664
Satd. Flow (prot)	1296	1160	1349	0	1296	1354
Flt Permitted	0.950				0.144	
Satd. Flow (perm)	1296	1160	1349	0	197	1354
Satd. Flow (RTOR)						
Lane Group Flow (vph)	92	213	730	0	199	726
Turn Type	Prot	pt+ov	NA		pm+pt	NA
Protected Phases	2	23	4		3	8
Permitted Phases					8	
Total Split (s)	30.0		51.0		12.0	63.0
Total Lost Time (s)	5.0		5.0		6.0	5.0
Act Effct Green (s)	18.0	36.8	46.2		64.0	65.0
Actuated g/C Ratio	0.19	0.40	0.50		0.69	0.70
v/c Ratio	0.37	0.47	1.09		0.70	0.77
Control Delay	35.0	24.9	86.7		27.7	18.1
Queue Delay	0.0	0.0	0.0		0.0	0.0
Total Delay	35.0	24.9	86.7		27.7	18.1
LOS	С	С	F		С	В
Approach Delay	27.9		86.7			20.2
Approach LOS	С		F			С
Queue Length 50th (ft)	47	91	~490		38	239
Queue Length 95th (ft)	85	158	#708		#168	#593
Internal Link Dist (ft)	133		759			783
Turn Bay Length (ft)					150	
Base Capacity (vph)	348	454	670		286	946
Starvation Cap Reductn	0	0	0		0	0
Spillback Cap Reductn	0	0	0		0	0
Storage Cap Reductn	0	0	0		0	0
Reduced v/c Ratio	0.26	0.47	1.09		0.70	0.77

Cycle Length: 93 Actuated Cycle Length: 93

Offset: 0 (0%), Referenced to phase 4:NET and 8:SWTL, Start of Green

Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.09 Intersection Signal Delay: 46.1 Intersection Capacity Utilization 84.8%

Intersection LOS: D ICU Level of Service E

Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 2: Paradise Rd & Vinnin Ligour Dr



	_#	<b>→</b>	7	<b>/</b>	<b>←</b>	٤	•	×	/	6	×	✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	7	£		7	f.		*	ĥ		*	<b>*</b>	7
Traffic Volume (vph)	213	95	212	115	136	27	187	537	43	54	523	229
Future Volume (vph)	213	95	212	115	136	27	187	537	43	54	523	229
Satd. Flow (prot)	1296	1186	0	1296	1313	0	1296	1336	0	1296	1365	1151
Flt Permitted	0.587			0.350			0.281			0.241		
Satd. Flow (perm)	801	1186	0	478	1313	0	383	1336	0	329	1365	1112
Satd. Flow (RTOR)												
Lane Group Flow (vph)	231	332	0	124	176	0	202	628	0	58	566	248
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	Perm
Protected Phases		6			2		7	4		3	8	
Permitted Phases	6			2			4			8		8
Total Split (s)	34.0	34.0		34.0	34.0		12.0	55.0		11.0	54.0	54.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	6.0		5.0	5.0	5.0
Act Effct Green (s)	29.3	29.3		29.3	29.3		57.5	51.1		54.8	49.0	49.0
Actuated g/C Ratio	0.29	0.29		0.29	0.29		0.58	0.51		0.55	0.49	0.49
v/c Ratio	0.99	0.96		0.89	0.46		0.72	0.92		0.25	0.85	0.46
Control Delay	93.6	75.6		89.1	33.7		34.5	45.1		13.1	36.3	20.2
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Delay	93.6	75.6		89.1	33.7		34.5	45.1		13.1	36.3	20.2
LOS	F	Е		F	С		С	D		В	D	С
Approach Delay		83.0			56.6			42.5			30.2	
Approach LOS		F			Е			D			С	
Queue Length 50th (ft)	146	208		75	92		55	371		14	302	100
Queue Length 95th (ft)	#303	#386		#189	158		#113	#615		30	#516	169
Internal Link Dist (ft)		1673			222			783			1077	
Turn Bay Length (ft)	150						500			150		150
Base Capacity (vph)	234	346		139	384		285	682		238	668	544
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.99	0.96		0.89	0.46		0.71	0.92		0.24	0.85	0.46

Cycle Length: 100
Actuated Cycle Length: 100

Offset: 0 (0%), Referenced to phase 4:NETL and 8:SWTL, Start of Green, Master Intersection

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.99 Intersection Signal Delay: 48.9 Intersection Capacity Utilization 103.3%

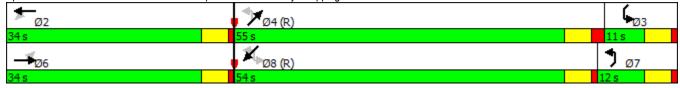
Intersection LOS: D
ICU Level of Service G

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Paradise Rd & Swampscott Mall Driveway/Shopping Drive



 11/24/2016
 Synchro 9 Report

 Seth
 Page 4

	۶	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	<b>/</b>	<b>+</b>	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Ť	£		¥	<b>*</b>	7		41}	7		4î.b	
Traffic Volume (vph)	22	282	72	279	367	106	42	392	303	101	438	29
Future Volume (vph)	22	282	72	279	367	106	42	392	303	101	438	29
Satd. Flow (prot)	1459	1476	0	1459	1523	1305	0	2891	1305	0	2856	0
Flt Permitted	0.524			0.194				0.838			0.672	
Satd. Flow (perm)	804	1476	0	298	1523	1305	0	2435	1305	0	1937	0
Satd. Flow (RTOR)												
Lane Group Flow (vph)	24	392	0	308	406	117	0	479	335	0	628	0
Turn Type	Perm	NA		pm+pt	NA	Perm	Perm	NA	Perm	pm+pt	NA	
Protected Phases		6		5	2			4		3	8	
Permitted Phases	6			2		2	4		4	8		
Total Split (s)	29.0	29.0		17.0	46.0	46.0	33.0	33.0	33.0	11.0	44.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0		5.0	5.0		5.0	
Act Effct Green (s)	24.0	24.0		41.0	41.0	41.0		28.0	28.0		39.0	
Actuated g/C Ratio	0.27	0.27		0.46	0.46	0.46		0.31	0.31		0.43	
v/c Ratio	0.11	1.00		1.06	0.59	0.20		0.63	0.83		0.70	
Control Delay	20.4	65.4		92.4	25.3	19.2		31.1	47.6		12.3	
Queue Delay	0.0	0.0		10.9	5.9	0.0		0.0	0.0		0.1	
Total Delay	20.4	65.4		103.3	31.2	19.2		31.1	47.6		12.4	
LOS	С	Е		F	С	В		С	D		В	
Approach Delay		62.8			56.2			37.9			12.4	
Approach LOS		Е			Е			D			В	
Queue Length 50th (ft)	12	242		~138	180	44		122	176		35	
Queue Length 95th (ft)	m16	m#297		#303	257	m80		176	#323		m65	
Internal Link Dist (ft)		529			213			261			571	
Turn Bay Length (ft)	150								150			
Base Capacity (vph)	214	393		290	693	594		757	406		900	
Starvation Cap Reductn	0	0		8	227	0		0	0		0	
Spillback Cap Reductn	0	0		0	0	0		0	0		12	
Storage Cap Reductn	0	0		0	0	0		0	0		0	
Reduced v/c Ratio	0.11	1.00		1.09	0.87	0.20		0.63	0.83		0.71	

Cycle Length: 90 Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 4:NBTL and 8:SBTL, Start of Green, Master Intersection

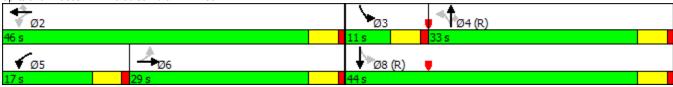
Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.06 Intersection Signal Delay: 41.5 Intersection Capacity Utilization 93.9%

Intersection LOS: D
ICU Level of Service F

Analysis Period (min) 15

- Volume exceeds capacity, queue is theoretically infinite.
  - Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
  - Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Paradise Rd & Vinnin St



	•	•	•	<b>†</b>	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ሻ	7	7	<b>*</b>	<b></b>	7
Traffic Volume (vph)	255	32	19	494	529	233
Future Volume (vph)	255	32	19	494	529	233
Satd. Flow (prot)	1286	1151	1296	1354	1365	1160
Flt Permitted	0.950		0.242			
Satd. Flow (perm)	1286	1128	330	1354	1365	1123
Satd. Flow (RTOR)						
Lane Group Flow (vph)	294	37	22	570	610	269
Turn Type	Prot	Perm	pm+pt	NA	NA	Perm
Protected Phases	4		5	2	6	
Permitted Phases		4	2			6
Total Split (s)	28.0	28.0	11.0	62.0	51.0	51.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0
Act Effct Green (s)	22.3	22.3	57.7	57.7	46.0	46.0
Actuated g/C Ratio	0.25	0.25	0.64	0.64	0.51	0.51
v/c Ratio	0.93	0.13	0.08	0.66	0.88	0.47
Control Delay	48.7	9.6	9.8	25.3	35.6	17.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	48.7	9.6	9.8	25.3	35.6	17.6
LOS	D	Α	Α	С	D	В
Approach Delay	44.3			24.8	30.1	
Approach LOS	D			С	С	
Queue Length 50th (ft)	109	4	5	227	291	93
Queue Length 95th (ft)	#294	m7	m12	332	#515	162
Internal Link Dist (ft)	691			571	296	
Turn Bay Length (ft)		150				
Base Capacity (vph)	328	288	284	868	697	573
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.90	0.13	0.08	0.66	0.88	0.47

Cycle Length: 90 Actuated Cycle Length: 90

Offset: 70 (78%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.93 Intersection Signal Delay: 30.9 Intersection Capacity Utilization 66.5%

Intersection LOS: C ICU Level of Service C

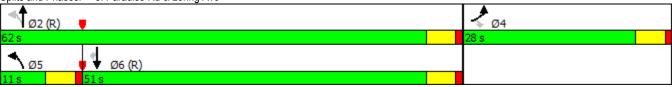
Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Paradise Rd & Loring Ave



	_#	-	7	<b>_</b>	<b>←</b>	٤	•	×	/	6	×	</th
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		₩		7	f)		ř	<b>*</b>	7	¥	f)	
Traffic Volume (vph)	3	5	2	382	3	47	5	254	352	44	236	6
Future Volume (vph)	3	5	2	382	3	47	5	254	352	44	236	6
Satd. Flow (prot)	0	1550	0	1296	1126	0	1296	1354	1160	1296	1347	0
Flt Permitted		0.969		0.750			0.590			0.384		
Satd. Flow (perm)	0	1522	0	1024	1126	0	805	1354	1123	524	1347	0
Satd. Flow (RTOR)									*100			
Lane Group Flow (vph)	0	11	0	436	57	0	6	290	402	50	276	0
Turn Type	Perm	NA		Perm	NA		Perm	NA	Perm	pm+pt	NA	
Protected Phases		6			2			4		3	8	
Permitted Phases	6			2			4		4	8		
Total Split (s)	47.0	47.0		47.0	47.0		32.0	32.0	32.0	11.0	43.0	
Total Lost Time (s)		5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Act Effct Green (s)		44.1		44.1	44.1		29.3	29.3	29.3	35.9	35.9	
Actuated g/C Ratio		0.49		0.49	0.49		0.33	0.33	0.33	0.40	0.40	
v/c Ratio		0.01		0.87	0.10		0.02	0.66	0.93	0.19	0.51	
Control Delay		13.1		26.9	4.4		22.7	35.6	54.0	7.3	11.6	
Queue Delay		0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Delay		13.1		26.9	4.4		22.7	35.6	54.0	7.3	11.6	
LOS		В		С	Α		С	D	D	Α	В	
Approach Delay		13.1			24.3			46.1			11.0	
Approach LOS		В			С			D			В	
Queue Length 50th (ft)		3		215	6		2	146	180	5	27	
Queue Length 95th (ft)		12		#421	m11		11	#259	#374	m11	41	
Internal Link Dist (ft)		69			529			965			691	
Turn Bay Length (ft)				150			100		150	150		
Base Capacity (vph)		746		501	552		261	440	432	260	568	
Starvation Cap Reductn		0		0	0		0	0	0	0	0	
Spillback Cap Reductn		0		0	0		0	0	0	0	0	
Storage Cap Reductn		0		0	0		0	0	0	0	0	
Reduced v/c Ratio		0.01		0.87	0.10		0.02	0.66	0.93	0.19	0.49	

Cycle Length: 90 Actuated Cycle Length: 90

Offset: 42 (47%), Referenced to phase 2:WBTL and 6:EBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.93

Intersection Signal Delay: 31.3

Intersection Capacity Utilization 74.5%

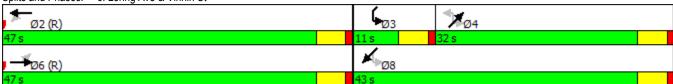
Analysis Period (min) 15

- User Entered Value
- # 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Loring Ave & Vinnin St



Intersection LOS: C

ICU Level of Service D

	-	•	•	<b>←</b>	•	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>*</b>	7		41≯	ř	7
Traffic Volume (vph)	505	183	146	527	185	96
Future Volume (vph)	505	183	146	527	185	96
Satd. Flow (prot)	1450	1232	0	2714	1447	1295
Flt Permitted				0.682	0.950	
Satd. Flow (perm)	1450	1193	0	1871	1427	1295
Satd. Flow (RTOR)						
Lane Group Flow (vph)	558	202	0	743	204	106
Turn Type	NA	Perm	pm+pt	NA	Prot	Prot
Protected Phases	6		5	2	4	4
Permitted Phases		6	2			
Total Split (s)	54.0	54.0	11.0	65.0	25.0	25.0
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0
Act Effct Green (s)	63.3	63.3		63.3	16.7	16.7
Actuated g/C Ratio	0.70	0.70		0.70	0.19	0.19
v/c Ratio	0.55	0.24		0.57	0.76	0.44
Control Delay	4.4	2.4		9.3	52.6	37.6
Queue Delay	1.5	0.6		0.1	0.1	0.0
Total Delay	5.9	3.0		9.4	52.7	37.6
LOS	Α	Α		Α	D	D
Approach Delay	5.1			9.4	47.5	
Approach LOS	Α			Α	D	
Queue Length 50th (ft)	91	18		98	109	53
Queue Length 95th (ft)	m164	m30		160	181	100
Internal Link Dist (ft)	213			175	1023	
Turn Bay Length (ft)	= : •					150
Base Capacity (vph)	1019	838		1315	321	287
Starvation Cap Reductn	274	350		0	0	0
Spillback Cap Reductn	0	0		48	3	0
Storage Cap Reductn	0	0		0	0	0
Reduced v/c Ratio	0.75	0.41		0.59	0.64	0.37

Cycle Length: 90 Actuated Cycle Length: 90

Offset: 82 (91%), Referenced to phase 2:WBTL and 6:EBT, Start of Green

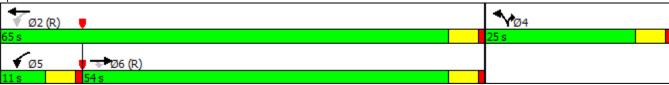
Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.76 Intersection Signal Delay: 14.1 Intersection Capacity Utilization 84.3%

Intersection LOS: B
ICU Level of Service E

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 7: Salem St & Vinnin St



	-	•	•	<b>←</b>	1	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>*</b>	7	7	•	7	7
Traffic Volume (vph)	535	284	99	575	291	140
Future Volume (vph)	535	284	99	575	291	140
Satd. Flow (prot)	1365	1151	1296	1354	1296	1151
Flt Permitted			0.214		0.950	
Satd. Flow (perm)	1365	1112	292	1354	1296	1151
Satd. Flow (RTOR)						
Lane Group Flow (vph)	604	321	112	649	329	158
Turn Type	NA	Perm	pm+pt	NA	Prot	pt+ov
Protected Phases	4		3	8	2	23
Permitted Phases		4	8			
Total Split (s)	47.0	47.0	23.0	70.0	30.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	
Act Effct Green (s)	43.2	43.2	57.0	57.0	25.0	38.8
Actuated g/C Ratio	0.47	0.47	0.62	0.62	0.27	0.42
v/c Ratio	0.94	0.61	0.41	0.77	0.93	0.33
Control Delay	49.3	24.8	11.7	20.5	69.5	20.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	49.3	24.8	11.7	20.5	69.5	20.5
LOS	D	С	В	С	Е	С
Approach Delay	40.8			19.2	53.6	
Approach LOS	D			В	D	
Queue Length 50th (ft)	322	134	25	251	183	59
Queue Length 95th (ft)	#576	238	46	406	#394	119
Internal Link Dist (ft)	1242			539	1673	
Turn Bay Length (ft)		200	100			150
Base Capacity (vph)	640	522	377	957	352	600
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.94	0.61	0.30	0.68	0.93	0.26

Cycle Length: 100
Actuated Cycle Length: 92.1

Control Type: Actuated-Uncoordinated Maximum v/c Ratio: 0.94

Intersection Signal Delay: 36.1 Intersection Capacity Utilization 81.4%

Intersection LOS: D
ICU Level of Service D

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 8: Swampscott Mall Driveway & Essex St



	×	À	_	×	ን	~	
Lane Group	SET	SER	NWL	NWT	NEL	NER	Ø9
Lane Configurations	<b>+</b>	7	7	•	N/F		
Traffic Volume (vph)	700	120	37	826	10	15	
Future Volume (vph)	700	120	37	826	10	15	
Satd. Flow (prot)	1801	1531	1711	1801	1622	0	
Flt Permitted			0.261		0.981		
Satd. Flow (perm)	1801	1531	470	1801	1622	0	
Satd. Flow (RTOR)							
Lane Group Flow (vph)	799	137	42	943	28	0	
Turn Type	NA	Perm	Perm	NA	Prot		
Protected Phases	6			2	4		9
Permitted Phases		6	2				
Total Split (s)	62.0	62.0	62.0	62.0	13.0		25.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0		
Act Effct Green (s)	43.5	43.5	43.5	43.5	10.5		
Actuated g/C Ratio	0.86	0.86	0.86	0.86	0.21		
v/c Ratio	0.52	0.10	0.10	0.61	0.08		
Control Delay	7.5	4.5	6.0	9.6	31.3		
Queue Delay	0.0	0.0	0.0	0.0	0.0		
Total Delay	7.6	4.5	6.0	9.6	31.3		
LOS	Α	Α	Α	Α	С		
Approach Delay	7.1			9.4	31.3		
Approach LOS	Α			Α	С		
Queue Length 50th (ft)	0	0	0	0	6		
Queue Length 95th (ft)	502	64	29	#771	44		
Internal Link Dist (ft)	486			296	259		
Turn Bay Length (ft)		150	150				
Base Capacity (vph)	1621	1378	423	1621	337		
Starvation Cap Reductn	51	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0		
Reduced v/c Ratio	0.51	0.10	0.10	0.58	0.08		

Cycle Length: 100
Actuated Cycle Length: 50.6
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.61

Intersection Signal Delay: 8.6
Intersection Capacity Utilization 60.6%

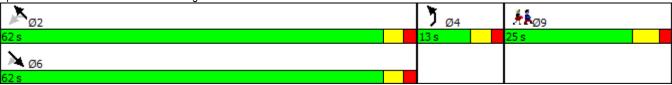
Intersection LOS: A ICU Level of Service B

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 9: Harrison Rd & Loring Ave



	*	<b>†</b>	<b>↓</b>	لر	<b>*</b>	4		
Lane Group	NBL	NBT	SBT	SBR	NEL	NER	Ø9	
Lane Configurations		ર્ન	ĵ.		W			Т
Traffic Volume (vph)	10	826	800	30	35	20		
Future Volume (vph)	10	826	800	30	35	20		
Satd. Flow (prot)	0	1799	1792	0	1659	0		
Flt Permitted		0.988			0.969			
Satd. Flow (perm)	0	1779	1792	0	1659	0		
Satd. Flow (RTOR)								
Lane Group Flow (vph)	0	954	947	0	63	0		
Turn Type	Perm	NA	NA		Prot			
Protected Phases		2	6		4		9	
Permitted Phases	2							
Total Split (s)	54.0	54.0	54.0		12.0		24.0	
Total Lost Time (s)		6.0	5.0		5.0			
Act Effct Green (s)		55.7	56.5		6.9			
Actuated g/C Ratio		0.75	0.76		0.09			
v/c Ratio		0.71	0.69		0.41			
Control Delay		13.8	12.7		41.7			
Queue Delay		0.4	0.0		0.0			
Total Delay		14.3	12.7		41.7			
LOS		В	В		D			
Approach Delay		14.3	12.7		41.7			
Approach LOS		В	В		D			
Queue Length 50th (ft)		172	154		24			
Queue Length 95th (ft)		#805	#779		#77			
Internal Link Dist (ft)		486	689		323			
Turn Bay Length (ft)								
Base Capacity (vph)		1338	1367		158			
Starvation Cap Reductn		100	0		0			
Spillback Cap Reductn		0	0		0			
Storage Cap Reductn		0	0		0			
Reduced v/c Ratio		0.77	0.69		0.40			

Cycle Length: 90

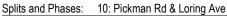
Actuated Cycle Length: 74.1 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.71

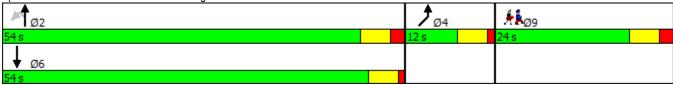
Intersection Signal Delay: 14.4 Intersection Capacity Utilization 68.2%

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.





Intersection LOS: B

ICU Level of Service C

	<b>→</b>	¬₄	4	<b>←</b>	*	4
Movement	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	<b>†</b>	7	ች	<b></b>	W	
Traffic Volume (veh/h)	722	23	79	642	16	123
Future Volume (Veh/h)	722	23	79	642	16	123
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	782	25	86	695	17	133
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)				922		
pX, platoon unblocked					0.75	
vC, conflicting volume			807		1649	782
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			807		1697	782
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			89		75	66
cM capacity (veh/h)			818		69	394
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NW 1	
Volume Total	782	25	86	695	150	
Volume Left	0	0	86	0	17	
Volume Right	0	25	0	0	133	
cSH	1700	1700	818	1700	256	
Volume to Capacity	0.46	0.01	0.11	0.41	0.59	
Queue Length 95th (ft)	0	0	9	0	84	
Control Delay (s)	0.0	0.0	9.9	0.0	37.1	
Lane LOS			А		Е	
Approach Delay (s)	0.0		1.1		37.1	
Approach LOS					Е	
Intersection Summary						
Average Delay			3.7			
Intersection Capacity Utilization			63.4%	IC.	U Level of	Service
Analysis Period (min)			15	10	O LOVOI OI	0011100
rinarysis i crioù (min)			13			

12. Todooco ot a Loggo							 _
	•	<b>→</b>	←	•	<b>\</b>	1	
Mayamant	EDI	r rnt	WDT	WDD	CDI	CDD	
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		र्स	1≽		W		
Traffic Volume (veh/h)	60	454	620	80	75	51	
Future Volume (Veh/h)	60	454	620	80	75	51	
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	
Hourly flow rate (vph)	68	513	700	90	85	58	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type		None	None				
Median storage veh)		140110	110110				
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	790				1394	745	
vC1, stage 1 conf vol	770				1374	743	
vC2, stage 2 conf vol							
vCu, unblocked vol	790				1394	745	
	4.1				6.4	6.2	
tC, single (s)	4.1				0.4	0.2	
tC, 2 stage (s)	0.0				0.5	0.0	
tF (s)	2.2				3.5	3.3	
p0 queue free %	92				41	86	
cM capacity (veh/h)	830				143	414	
Direction, Lane #	EB 1	WB 1	SB 1				
Volume Total	581	790	143				
Volume Left	68	0	85				
Volume Right	0	90	58				
cSH	830	1700	195				
Volume to Capacity	0.08	0.46	0.73				
Queue Length 95th (ft)	7	0	119				
Control Delay (s)	2.1	0.0	62.0				
Lane LOS	A	0.0	F				
Approach Delay (s)	2.1	0.0	62.0				
Approach LOS	2.1	0.0	02.0 F				
•			1				
Intersection Summary							
Average Delay			6.7			_	
Intersection Capacity Utilization			85.6%	IC	U Level of	Service	
Analysis Period (min)			15				

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	<b>L</b>	لر	•	*	×	₹
Movement	SBL	SBR	NEL	NET	SWT	SWR
Lane Configurations	W			सी	1,	
Traffic Volume (veh/h)	51	14	12	447	460	65
Future Volume (Veh/h)	51	14	12	447	460	65
Sign Control	Stop	17	12	Free	Free	03
Grade	0%			0%	0%	
Peak Hour Factor	0.95	0.93	0.95	0.95	0.95	0.95
	56	16	13	494	508	72
Hourly flow rate (vph)	30	10	13	494	508	12
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (ft)				674		
pX, platoon unblocked	0.88					
vC, conflicting volume	1064	544	580			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1005	544	580			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	76	97	99			
cM capacity (veh/h)	233	539	994			
Direction, Lane #	SB 1	NE 1	SW 1			
Volume Total	72	507	580			
Volume Left	56	13	0			
Volume Right	16	0	72			
cSH	266	994	1700			
Volume to Capacity	0.27	0.01	0.34			
Queue Length 95th (ft)	27	1	0			
Control Delay (s)	23.5	0.4	0.0			
Lane LOS	С	Α				
Approach Delay (s)	23.5	0.4	0.0			
Approach LOS	С					
Intersection Summary						
Average Delay			1.6			
Intersection Capacity Utilization			45.4%	IC	CU Level of S	Corvico
				IC	O LEVELUI S	oei vice
Analysis Period (min)			15			

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Movement		<b>→</b>	-	4	<b>←</b>	*	4
Lane Configurations	Movement	EBT	EBR	WBL	WBT	NWL	NWR
Traffic Volume (veh/h)         483         99         39         632         51         31           Future Volume (Veh/h)         483         99         39         632         51         31           Sign Control         Free         Free         Stop         Gas         51         31           Sign Control         Free         Free         Stop         Gas         51         31           Grade         0%         0%         0%         0%         0%           Peak Hour Factor         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93							
Future Volume (Veh/h)		483	99	39			31
Sign Control         Free Grade         Free Own							
Grade 0% 0,93 0,93 0,93 0,93 0,93 0,93 0,93 0,93			,,	0,			01
Peak Hour Factor   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.							
Hourly flow rate (vph)   545   112   44   714   58   35     Pedestrians   Lane Width (ft)     Walking Speed (ft/s)     Percent Blockage     Right turn flare (veh)     Median type   None   None     Median storage veh)     Upstream signal (ft)     VX, platoon unblocked     VC, conflicting volume   657   1403   601     VC1, stage 1 conf vol     VC2, stage 2 conf vol     VC2, stage 2 conf vol     VC2, stage (s)     IF (s)   2.2   3.5   3.3     Po queue free %   95   60   93     CM capacity (veh/h)   931   147   500     Direction, Lane #   EB 1   WB 1   NW 1     Volume Total   657   758   93     Volume Left   0   44   58     Volume Left   0   44   58     Volume Right   112   0   35     CSH   1700   931   200     Volume Logacity   0.39   0.05   0.47     Queue Length 95th (ft)   0   4   56     Control Delay (s)   0.0   1.2   37.8     Approach LOS   E     Intersection Summary     Average Delay   10   10     Intersection Capacity Utilization   80.1%   ICU Level of Service			0.93	0.93			0.93
Pedestrians   Lane Width (ft)   Walking Speed (ft/s)   Percent Blockage   Right turn flare (veh)   Median type   None   None   Median storage veh   Upstream signal (ft)   pX, platoon unblocked   VC, conflicting volume   657							
Lane Width (ft)  Walking Speed (ft/s)  Percent Blockage Right turn flare (veh)  Median type None  Median storage veh)  Upstream signal (ft) pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC4, single (s) tC, single (s) tF (s) 2.2 3.5 3.3 p0 queue free % 95 60 93 cM capacity (veh/h)  Pirection, Lane # EB 1 WB 1 NW 1  Volume Total 657 758 93 Volume Left 0 44 58 Volume Right 112 0 35 cSH 1700 931 200 Volume to Capacity 0.39 0.05 0.47 Queue Length 95th (ft) 0 4 56 Control Delay (s) 0.0 1.2 37.8 Approach LOS E  Intersection Summary  Average Delay Intersection Capacity Utilization  Intersection Capacity Utilization  Intersection Capacity Utilization  Intersection Capacity Utilization  INDICULE Velor of Service		010	112		, , ,	00	00
Walking Speed (ft/s)         Percent Blockage       Right turn flare (veh)         Median type       None       None         Median storage veh)       Upstream signal (ft)       V. platoon unblocked         VC, conflicting volume       657       1403       601         VC1, stage 1 conf vol       657       1403       601         VC2, stage 2 conf vol       657       1403       601         VC, single (s)       4.1       6.4       6.2         C, 2 stage (s)       4.1       6.4       6.2         IF (s)       2.2       3.5       3.3         p0 queue free %       95       60       93         M capacity (veh/h)       931       147       500         Direction, Lane #       EB1       WB 1       NW 1         Volume Total       657       758       93         Volume Left       0       44       58         Volume Right       112       0       35         cSH       1700       931       200         Volume Post (ft)       0       4       56         Control Delay (s)       0.0       1.2       37.8         Lane LOS       A       E							
Percent Blockage   Right turn flare (veh)							
Right turn flare (veh)  Median type None None  Median storage veh)  Upstream signal (ft) pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC2, stage (s) tF (s) 4.1 6.4 6.2 tC, 2 stage (s) tF (s) 2.2 3.5 3.3 p0 queue free % 95 60 93 cM capacity (veh/h) 931 147 500  Direction, Lane # EB 1 WB 1 NW 1  Volume Total 657 758 93 Volume Left 0 44 58 Volume Right 112 0 35 cSH 1700 931 200 Volume to Capacity 0 0.39 0.05 0.47 Queue Length 95th (ft) 0 4 56 Control Delay (s) 0.0 1.2 37.8 Lane LOS A E Approach Delay (s) 0.0 1.2 37.8 Approach LOS E  Intersection Summary  Average Delay Intersection Capacity Utilization  None  A 54  5 2 4 403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1404 6.2  1409 6.57  1403 601  1403 601  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2							
Median type         None         None           Median storage veh)         Upstream signal (ft)           pX, platoon unblocked         VC, conflicting volume         657         1403         601           vC1, stage 1 conf vol         VC2, stage 2 conf vol         VC2, stage 2 conf vol         VC2, stage (s)         4.1         6.4         6.2           tC, 2 stage (s)         TEST (SS)         2.2         3.5         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.3         3.4         4.4         4.5         4.5 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>							
Median storage veh)       Upstream signal (ft)         pX, platoon unblocked       vC, conflicting volume       657       1403       601         vC1, stage 1 conf vol       vC2, stage 2 conf vol       vCu, unblocked vol       657       1403       601         tC, single (s)       4.1       6.4       6.2       6.2         tC, 2 stage (s)       tF (s)       2.2       3.5       3.3         p0 queue free %       95       60       93         cM capacity (veh/h)       931       147       500         Direction, Lane #       EB 1       WB 1       NW 1         Volume Total       657       758       93         Volume Left       0       44       58         Volume Right       112       0       35         CSH       1700       931       200         Volume to Capacity       0.39       0.05       0.47         Queue Length 95th (ft)       0       4       56         Control Delay (s)       0.0       1.2       37.8         Lane LOS       A       E         Approach LOS       E         Intersection Summary         Average		Mono			Mono		
Upstream signal (ft) pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC2, unblocked vol tC, single (s) tF (s) 2.2 3.5 3.3 p0 queue free % 95 60 93 cM capacity (veh/h) 931 147 500  Direction, Lane # EB 1 WB 1 Volume Total Volume Total Volume Right 112 0 35 cSH 1700 931 200 Volume to Capacity 0.39 0.05 0.47 Queue Length 95th (ft) 0 4 56 Control Delay (s) 1.2 37.8 Approach LOS E Intersection Summary Average Delay Intersection Capacity Utilization  657 1403 601 40. 40. 41 6.4 6.2 42. 42 3.5 3.3 401 47 500  80.1% ICU Level of Service		INOTIC			None		
pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol tC, single (s) tF (s) p0 queue free % p1 queue free % p1 queue free % p2 queue free % p2 queue free % p3 queue free % p4 queue free % p5 queue free % p6 queue fre							
vC, conflicting volume       657       1403       601         vC1, stage 1 conf vol       vC2, stage 2 conf vol         vCu, unblocked vol       657       1403       601         tC, single (s)       4.1       6.4       6.2         tC, 2 stage (s)       tF (s)       2.2       3.5       3.3         p0 queue free %       95       60       93         cM capacity (veh/h)       931       147       500         Direction, Lane #       EB 1       WB 1       NW 1         Volume Total       657       758       93         Volume Left       0       44       58         Volume Right       112       0       35         cSH       1700       931       200         Volume to Capacity       0.39       0.05       0.47         Queue Length 95th (ft)       0       4       56         Control Delay (s)       0.0       1.2       37.8         Lane LOS       A       E         Approach LOS       E         Intersection Summary         Average Delay       2.9         Intersection Capacity Utilization       80.1% <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>							
VC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol tC, single (s) tF, (s) tF (s) tF (s) tC, 2 stage (s) tF (s) tF (s) tC, 2 stage (s) tF (s) tF (s) tC, 2 stage (s) tC, 3 stage (s) tC, 4 stage (s				457		1402	601
vC2, stage 2 conf vol         vCu, unblocked vol       657       1403       601         tC, single (s)       4.1       6.4       6.2         tC, 2 stage (s)       tF (s)       2.2       3.5       3.3         p0 queue free %       95       60       93         cM capacity (veh/h)       931       147       500         Direction, Lane #       EB 1       WB 1       NW 1         Volume Total       657       758       93         Volume Left       0       44       58         Volume Right       112       0       35         CSH       1700       931       200         Volume to Capacity       0.39       0.05       0.47         Queue Length 95th (ft)       0       4       56         Control Delay (s)       0.0       1.2       37.8         Lane LOS       A       E         Approach LOS       E         Intersection Summary         Average Delay       2.9         Intersection Capacity Utilization       80.1%       ICU Level of Service				037		1403	001
vCu, unblocked vol       657       1403       601         tC, single (s)       4.1       6.4       6.2         tC, 2 stage (s)       tF (s)       2.2       3.5       3.3         p0 queue free %       95       60       93         cM capacity (veh/h)       931       147       500         Direction, Lane #       EB 1       WB 1       NW 1         Volume Total       657       758       93         Volume Left       0       44       58         Volume Right       112       0       35         CSH       1700       931       200         Volume to Capacity       0.39       0.05       0.47         Queue Length 95th (ft)       0       4       56         Control Delay (s)       0.0       1.2       37.8         Lane LOS       A       E         Approach Delay (s)       0.0       1.2       37.8         Approach LOS       E         Intersection Summary         Average Delay       2.9         Intersection Capacity Utilization       80.1%       ICU Level of Service							
tC, single (s) 4.1 6.4 6.2 tC, 2 stage (s) tF (s) 2.2 3.5 3.3 p0 queue free % 95 60 93 cM capacity (veh/h) 931 147 500  Direction, Lane # EB 1 WB 1 NW 1  Volume Total 657 758 93 Volume Left 0 44 58 Volume Right 112 0 35 cSH 1700 931 200 Volume to Capacity 0.39 0.05 0.47 Queue Length 95th (ft) 0 4 56 Control Delay (s) 0.0 1.2 37.8 Lane LOS A E Approach Delay (s) 0.0 1.2 37.8 Approach LOS  Intersection Summary  Average Delay Intersection Capacity Utilization 80.1% ICU Level of Service				457		1402	<b>601</b>
tC, 2 stage (s)  tF (s)							
tF (s) 2.2 3.5 3.3 p0 queue free % 95 60 93 cM capacity (veh/h) 931 147 500  Direction, Lane # EB 1 WB 1 NW 1  Volume Total 657 758 93  Volume Left 0 44 58  Volume Right 112 0 35 cSH 1700 931 200  Volume to Capacity 0.39 0.05 0.47 Queue Length 95th (ft) 0 4 56 Control Delay (s) 0.0 1.2 37.8 Lane LOS A E Approach Delay (s) 0.0 1.2 37.8 Approach LOS  Intersection Summary  Average Delay Intersection Capacity Utilization 80.1%  ICU Level of Service				4.1		0.4	0.2
p0 queue free %       95       60       93         cM capacity (veh/h)       931       147       500         Direction, Lane #       EB 1       WB 1       NW 1         Volume Total       657       758       93         Volume Left       0       44       58         Volume Right       112       0       35         cSH       1700       931       200         Volume to Capacity       0.39       0.05       0.47         Queue Length 95th (ft)       0       4       56         Control Delay (s)       0.0       1.2       37.8         Lane LOS       A       E         Approach Delay (s)       0.0       1.2       37.8         Approach LOS       E         Intersection Summary         Average Delay       2.9         Intersection Capacity Utilization       80.1%       ICU Level of Service				2.2		2 E	2.2
CM capacity (veh/h)         931         147         500           Direction, Lane #         EB 1         WB 1         NW 1           Volume Total         657         758         93           Volume Left         0         44         58           Volume Right         112         0         35           cSH         1700         931         200           Volume to Capacity         0.39         0.05         0.47           Queue Length 95th (ft)         0         4         56           Control Delay (s)         0.0         1.2         37.8           Lane LOS         A         E           Approach Delay (s)         0.0         1.2         37.8           Approach LOS         E           Intersection Summary           Average Delay         2.9           Intersection Capacity Utilization         80.1%         ICU Level of Service							
Direction, Lane #         EB 1         WB 1         NW 1           Volume Total         657         758         93           Volume Left         0         44         58           Volume Right         112         0         35           cSH         1700         931         200           Volume to Capacity         0.39         0.05         0.47           Queue Length 95th (ft)         0         4         56           Control Delay (s)         0.0         1.2         37.8           Lane LOS         A         E           Approach Delay (s)         0.0         1.2         37.8           Approach LOS         E           Intersection Summary           Average Delay         2.9           Intersection Capacity Utilization         80.1%         ICU Level of Service							
Volume Total         657         758         93           Volume Left         0         44         58           Volume Right         112         0         35           cSH         1700         931         200           Volume to Capacity         0.39         0.05         0.47           Queue Length 95th (ft)         0         4         56           Control Delay (s)         0.0         1.2         37.8           Lane LOS         A         E           Approach Delay (s)         0.0         1.2         37.8           Approach LOS         E           Intersection Summary           Average Delay         2.9           Intersection Capacity Utilization         80.1%         ICU Level of Service						147	500
Volume Left         0         44         58           Volume Right         112         0         35           cSH         1700         931         200           Volume to Capacity         0.39         0.05         0.47           Queue Length 95th (ft)         0         4         56           Control Delay (s)         0.0         1.2         37.8           Lane LOS         A         E           Approach Delay (s)         0.0         1.2         37.8           Approach LOS         E           Intersection Summary           Average Delay         2.9           Intersection Capacity Utilization         80.1%         ICU Level of Service							
Volume Right         112         0         35           cSH         1700         931         200           Volume to Capacity         0.39         0.05         0.47           Queue Length 95th (ft)         0         4         56           Control Delay (s)         0.0         1.2         37.8           Lane LOS         A         E           Approach Delay (s)         0.0         1.2         37.8           Approach LOS         E           Intersection Summary         E           Intersection Capacity Utilization         80.1%         ICU Level of Service							
1700   931   200							
Volume to Capacity         0.39         0.05         0.47           Queue Length 95th (ft)         0         4         56           Control Delay (s)         0.0         1.2         37.8           Lane LOS         A         E           Approach Delay (s)         0.0         1.2         37.8           Approach LOS         E           Intersection Summary           Average Delay         2.9           Intersection Capacity Utilization         80.1%         ICU Level of Service							
Queue Length 95th (ft)         0         4         56           Control Delay (s)         0.0         1.2         37.8           Lane LOS         A         E           Approach Delay (s)         0.0         1.2         37.8           Approach LOS         E           Intersection Summary         E           Average Delay         2.9           Intersection Capacity Utilization         80.1%         ICU Level of Service							
Control Delay (s)         0.0         1.2         37.8           Lane LOS         A         E           Approach Delay (s)         0.0         1.2         37.8           Approach LOS         E           Intersection Summary           Average Delay         2.9           Intersection Capacity Utilization         80.1%         ICU Level of Service							
Lane LOS A E Approach Delay (s) 0.0 1.2 37.8 Approach LOS E  Intersection Summary Average Delay Intersection Capacity Utilization 80.1% ICU Level of Service							
Approach Delay (s) 0.0 1.2 37.8 Approach LOS E  Intersection Summary  Average Delay 2.9 Intersection Capacity Utilization 80.1% ICU Level of Service		0.0		37.8			
Approach LOS E  Intersection Summary  Average Delay 2.9 Intersection Capacity Utilization 80.1% ICU Level of Service				_			
Intersection Summary  Average Delay Intersection Capacity Utilization  2.9 Intersection Capacity Utilization  80.1%  ICU Level of Service	Approach Delay (s)	0.0	1.2	37.8			
Average Delay  2.9 Intersection Capacity Utilization  80.1%  ICU Level of Service	Approach LOS			Е			
Average Delay  Intersection Capacity Utilization  2.9  ICU Level of Service	Intersection Summary						
Intersection Capacity Utilization 80.1% ICU Level of Service				2.9			
Analysis Doring (min)					IC	U Level of	Service
AUGIVAIA ECHUU IIIIIIII	Analysis Period (min)			15	.0		

# Level of Service (LOS) Analysis Alternatives 2

	<b>4</b>	×	7	<b>F</b>	×	*	Ť	×	~	Ĺ	×	*
Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		₩			4			4			4	
Traffic Volume (vph)	22	45	4	15	44	7	5	417	8	5	659	31
Future Volume (vph)	22	45	4	15	44	7	5	417	8	5	659	31
Satd. Flow (prot)	0	1759	0	0	1756	0	0	1795	0	0	1790	0
Flt Permitted		0.870			0.899			0.991			0.997	
Satd. Flow (perm)	0	1554	0	0	1596	0	0	1781	0	0	1784	0
Satd. Flow (RTOR)		3			6			2			4	
Lane Group Flow (vph)	0	82	0	0	76	0	0	496	0	0	802	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		6			2			4			8	
Permitted Phases	6			2			4			8		
Total Split (s)	12.0	12.0		12.0	12.0		40.0	40.0		40.0	40.0	
Total Lost Time (s)		6.0			6.0			6.0			6.0	
Act Effct Green (s)		6.5			6.5			37.1			37.1	
Actuated g/C Ratio		0.11			0.11			0.64			0.64	
v/c Ratio		0.47			0.41			0.43			0.70	
Control Delay		40.2			36.5			12.1			19.3	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		40.2			36.5			12.1			19.3	
LOS		D			D			В			В	
Approach Delay		40.2			36.5			12.1			19.3	
Approach LOS		D			D			В			В	
Queue Length 50th (ft)		23			20			57			121	
Queue Length 95th (ft)		#102			#88			264			#604	
Internal Link Dist (ft)		155			218			904			626	
Turn Bay Length (ft)												
Base Capacity (vph)		176			184			1130			1132	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.47			0.41			0.44			0.71	

Cycle Length: 75

Actuated Cycle Length: 57.8

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.70 Intersection Signal Delay: 18.9

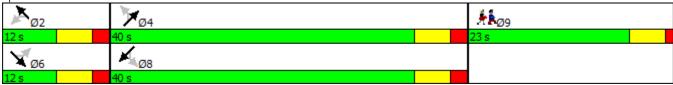
Intersection Capacity Utilization 57.6%

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Paradise Rd & Ellis Rd



Intersection LOS: B

ICU Level of Service B

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	<b>*</b>	₹	×	~	Ĺ	×
Lane Group	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	*	7	ĵ.		*	<b>^</b>
Traffic Volume (vph)	11	28	698	3	37	876
Future Volume (vph)	11	28	698	3	37	876
Satd. Flow (prot)	1496	1338	1510	0	1496	2931
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1468	1338	1510	0	1484	2931
Satd. Flow (RTOR)						
Lane Group Flow (vph)	12	30	759	0	40	948
Turn Type	Prot	pt+ov	NA		Prot	NA
Protected Phases	2	23	4		3	8
Permitted Phases						
Total Split (s)	19.0		54.0		12.0	66.0
Total Lost Time (s)	5.0		5.0		6.0	5.0
Act Effct Green (s)	14.0	26.0	53.8		6.0	61.0
Actuated g/C Ratio	0.16	0.31	0.63		0.07	0.72
v/c Ratio	0.05	0.07	0.79		0.38	0.45
Control Delay	30.7	21.7	21.6		41.5	6.9
Queue Delay	0.0	0.0	0.0		0.0	0.0
Total Delay	30.7	21.7	21.6		41.5	6.9
LOS	С	С	С		D	А
Approach Delay	24.3		21.6			8.3
Approach LOS	С		С			А
Queue Length 50th (ft)	5	11	318		21	74
Queue Length 95th (ft)	20	31	#578		m43	177
Internal Link Dist (ft)	133		351			785
Turn Bay Length (ft)					150	
Base Capacity (vph)	246	409	955		105	2103
Starvation Cap Reductn	0	0	0		0	0
Spillback Cap Reductn	0	0	0		0	0
Storage Cap Reductn	0	0	0		0	0
Reduced v/c Ratio	0.05	0.07	0.79		0.38	0.45

Cycle Length: 85

Actuated Cycle Length: 85

Offset: 69 (81%), Referenced to phase 4:NET and 8:SWT, Start of Green

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.79

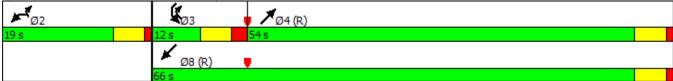
Intersection Signal Delay: 14.3 Intersection Capacity Utilization 63.1% Intersection LOS: B
ICU Level of Service B

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.





	_#	<b>→</b>	7	<b>/</b>	<b>←</b>	٤	•	×	/	4	×	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	7	f)		ř	f.		ř	<b>1</b> 13		Ť	<b>↑</b> ₽	
Traffic Volume (vph)	68	15	75	57	41	13	72	622	8	22	782	144
Future Volume (vph)	68	15	75	57	41	13	72	622	8	22	782	144
Satd. Flow (prot)	1496	1338	0	1496	1505	0	1496	2924	0	1496	2850	0
Flt Permitted	0.719			0.692			0.227			0.387		
Satd. Flow (perm)	1114	1338	0	1073	1505	0	357	2924	0	608	2850	0
Satd. Flow (RTOR)												
Lane Group Flow (vph)	75	100	0	63	59	0	80	696	0	24	1023	0
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		6			2		7	4		3	8	
Permitted Phases	6			2			4			8		
Total Split (s)	20.0	20.0		20.0	20.0		11.0	54.0		11.0	54.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	6.0		5.0	5.0	
Act Effct Green (s)	11.3	11.3		11.1	11.1		64.3	61.8		61.5	57.8	
Actuated g/C Ratio	0.13	0.13		0.13	0.13		0.76	0.73		0.72	0.68	
v/c Ratio	0.51	0.56		0.45	0.30		0.23	0.33		0.05	0.53	
Control Delay	45.6	46.3		43.4	36.0		1.7	1.6		3.8	10.8	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	45.6	46.3		43.4	36.0		1.7	1.6		3.8	10.8	
LOS	D	D		D	D		Α	Α		Α	В	
Approach Delay		46.0			39.8			1.6			10.7	
Approach LOS		D			D			Α			В	
Queue Length 50th (ft)	38	51		31	29		2	11		3	159	
Queue Length 95th (ft)	78	96		68	62		m3	m20		9	236	
Internal Link Dist (ft)		1622			228			785			1423	
Turn Bay Length (ft)	150						500			150		
Base Capacity (vph)	196	236		189	265		356	2126		503	1938	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.38	0.42		0.33	0.22		0.22	0.33		0.05	0.53	

Cycle Length: 85

Actuated Cycle Length: 85

Offset: 0 (0%), Referenced to phase 4:NETL and 8:SWTL, Start of Green, Master Intersection

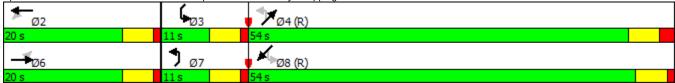
Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.56 Intersection Signal Delay: 11.9 Intersection Capacity Utilization 59.0%

Intersection LOS: B
ICU Level of Service B

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.





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	۶	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<b>/</b>	<b>/</b>	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ř	<b>∱</b>		¥	<b>*</b>	7		41≯	7		4î∌	
Traffic Volume (vph)	25	303	50	327	410	72	32	400	189	76	388	25
Future Volume (vph)	25	303	50	327	410	72	32	400	189	76	388	25
Satd. Flow (prot)	1496	1535	0	1181	1243	1338	0	2979	1285	0	2936	0
Flt Permitted	0.506			0.197				0.873			0.656	
Satd. Flow (perm)	797	1535	0	245	1243	1300	0	2611	1248	0	1942	0
Satd. Flow (RTOR)												
Lane Group Flow (vph)	27	382	0	354	444	78	0	468	205	0	529	0
Turn Type	Perm	NA		pm+pt	NA	Perm	Perm	NA	Perm	pm+pt	NA	
Protected Phases		6		5	2			4		3	8	
Permitted Phases	6			2		2	4		4	8		
Total Split (s)	32.0	32.0		30.0	62.0	62.0	27.0	27.0	27.0	11.0	38.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0		5.0	5.0		5.0	
Act Effct Green (s)	26.4	26.4		56.4	56.4	56.4		22.0	22.0		33.6	
Actuated g/C Ratio	0.26	0.26		0.56	0.56	0.56		0.22	0.22		0.34	
v/c Ratio	0.13	0.94		0.95	0.63	0.11		0.82	0.75		0.74	
Control Delay	27.9	62.7		49.5	15.0	8.5		49.9	54.8		21.2	
Queue Delay	0.0	0.0		1.7	6.1	0.0		0.0	0.0		0.0	
Total Delay	27.9	62.7		51.2	21.1	8.5		49.9	54.8		21.2	
LOS	С	Е		D	С	Α		D	D		С	
Approach Delay		60.4			32.2			51.4			21.2	
Approach LOS		Е			С			D			С	
Queue Length 50th (ft)	16	255		147	135	16		150	123		41	
Queue Length 95th (ft)	m22	m#381		m#298	m223	m24		#229	#232		98	
Internal Link Dist (ft)		529			213			1423			571	
Turn Bay Length (ft)	150								150			
Base Capacity (vph)	215	414		372	708	741		574	274		718	
Starvation Cap Reductn	0	0		4	207	0		0	0		0	
Spillback Cap Reductn	0	0		0	0	0		0	0		0	
Storage Cap Reductn	0	0		0	0	0		0	0		0	
Reduced v/c Ratio	0.13	0.92		0.96	0.89	0.11		0.82	0.75		0.74	

Cycle Length: 100
Actuated Cycle Length: 100

Offset: 0 (0%), Referenced to phase 4:NBTL and 8:SBTL, Start of Green, Master Intersection

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.95 Intersection Signal Delay: 39.7 Intersection Capacity Utilization 96.8%

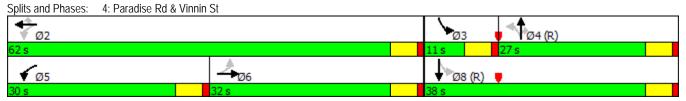
Intersection LOS: D
ICU Level of Service F

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Oueue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.



	•	•	4	<b>†</b>	<b>↓</b>	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	7	7	¥	<b>*</b>	<b>†</b>	7
Traffic Volume (vph)	263	25	9	499	456	198
Future Volume (vph)	263	25	9	499	456	198
Satd. Flow (prot)	1496	1338	1496	1574	1574	1338
Flt Permitted	0.950		0.249			
Satd. Flow (perm)	1496	1295	392	1574	1574	1269
Satd. Flow (RTOR)						
Lane Group Flow (vph)	285	27	10	540	494	214
Turn Type	Prot	Perm	pm+pt	NA	NA	Perm
Protected Phases	4		5	2	6	
Permitted Phases		4	2			6
Total Split (s)	31.0	31.0	23.0	69.0	46.0	46.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0
Act Effct Green (s)	22.7	22.7	67.3	67.3	41.0	41.0
Actuated g/C Ratio	0.23	0.23	0.67	0.67	0.41	0.41
v/c Ratio	0.84	0.09	0.02	0.51	0.77	0.41
Control Delay	22.0	5.0	13.0	24.8	34.8	24.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	22.0	5.0	13.0	24.8	34.8	24.0
LOS	С	Α	В	С	С	С
Approach Delay	20.6			24.6	31.6	
Approach LOS	С			С	С	
Queue Length 50th (ft)	170	5	2	235	265	95
Queue Length 95th (ft)	m17	m2	m6	m328	399	159
Internal Link Dist (ft)	691			571	296	
Turn Bay Length (ft)		150				
Base Capacity (vph)	388	336	498	1058	645	520
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.73	0.08	0.02	0.51	0.77	0.41

Cycle Length: 100
Actuated Cycle Length: 100

Offset: 83 (83%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

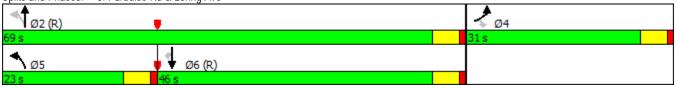
Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.84 Intersection Signal Delay: 26.9 Intersection Capacity Utilization 56.0%

Intersection LOS: C ICU Level of Service B

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Paradise Rd & Loring Ave



	_#	-	7	<b>*</b>	<b>←</b>	٤	•	×	/	6	K	</th
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		₩		7	<b>1</b>		ř	<b></b>	*	¥	f)	
Traffic Volume (vph)	2	1	2	440	5	17	2	300	330	14	195	2
Future Volume (vph)	2	1	2	440	5	17	2	300	330	14	195	2
Satd. Flow (prot)	0	1381	0	1481	1320	0	1481	1497	1326	1481	1494	0
Flt Permitted		0.961		0.753			0.601			0.242		
Satd. Flow (perm)	0	1355	0	1174	1320	0	937	1497	1326	377	1494	0
Satd. Flow (RTOR)									*200			
Lane Group Flow (vph)	0	7	0	570	28	0	3	389	428	18	256	0
Turn Type	Perm	NA		Perm	NA		Perm	NA	Perm	pm+pt	NA	
Protected Phases		6			2			4		3	8	
Permitted Phases	6			2			4		4	8		
Total Split (s)	57.0	57.0		57.0	57.0		32.0	32.0	32.0	11.0	43.0	
Total Lost Time (s)		5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Act Effct Green (s)		55.3		55.3	55.3		30.3	30.3	30.3	34.7	34.7	
Actuated g/C Ratio		0.55		0.55	0.55		0.30	0.30	0.30	0.35	0.35	
v/c Ratio		0.01		0.88	0.04		0.01	0.86	0.79	0.09	0.49	
Control Delay		11.7		27.8	5.2		26.5	53.3	29.2	7.2	11.8	
Queue Delay		0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Delay		11.7		27.8	5.2		26.5	53.3	29.2	7.2	11.8	
LOS		В		С	Α		С	D	С	Α	В	
Approach Delay		11.7			26.7			40.6			11.5	
Approach LOS		В			С			D			В	
Queue Length 50th (ft)		2		331	4		1	209	122	2	31	
Queue Length 95th (ft)		8		#465	m6		8	#361	#232	m5	37	
Internal Link Dist (ft)		69			529			662			691	
Turn Bay Length (ft)				150			100		150	150		
Base Capacity (vph)		748		648	729		284	454	541	197	567	
Starvation Cap Reductn		0		0	0		0	0	0	0	0	
Spillback Cap Reductn		0		0	0		0	0	0	0	0	
Storage Cap Reductn		0		0	0		0	0	0	0	0	
Reduced v/c Ratio		0.01		0.88	0.04		0.01	0.86	0.79	0.09	0.45	

Cycle Length: 100
Actuated Cycle Length: 100

Offset: 43 (43%), Referenced to phase 2:WBTL and 6:EBTL, Start of Green

Control Type: Actuated-Coordinated

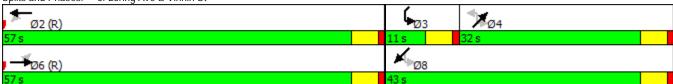
Maximum v/c Ratio: 0.88

Intersection Signal Delay: 30.9 Intersection Capacity Utilization 61.9% Intersection LOS: C
ICU Level of Service B

Analysis Period (min) 15

- * User Entered Value
- # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Loring Ave & Vinnin St



	-	•	•	←	4	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b>	7		414	*	7
Traffic Volume (vph)	423	151	63	654	145	68
Future Volume (vph)	423	151	63	654	145	68
Satd. Flow (prot)	1231	1046	0	1522	1192	1024
Flt Permitted				0.866	0.950	
Satd. Flow (perm)	1231	994	0	1322	1173	1024
Satd. Flow (RTOR)						
Lane Group Flow (vph)	458	163	0	776	157	74
Turn Type	NA	Perm	pm+pt	NA	Prot	Prot
Protected Phases	6		5	2	4	4
Permitted Phases		6	2			
Total Split (s)	65.0	65.0	11.0	76.0	24.0	24.0
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0
Act Effct Green (s)	73.3	73.3		73.3	16.7	16.7
Actuated g/C Ratio	0.73	0.73		0.73	0.17	0.17
v/c Ratio	0.51	0.22		0.80	0.79	0.44
Control Delay	7.4	4.6		17.7	66.8	44.9
Queue Delay	2.8	0.7		0.5	0.0	0.0
Total Delay	10.2	5.3		18.2	66.8	44.9
LOS	В	А		В	E	D
Approach Delay	8.9			18.2	59.8	
Approach LOS	A			В	E	
Queue Length 50th (ft)	148	41		156	95	42
Queue Length 95th (ft)	m177	m42		#288	#186	87
Internal Link Dist (ft)	213			312	357	
Turn Bay Length (ft)	2.0			Ŭ. <u>E</u>	00.	150
Base Capacity (vph)	902	728		969	226	194
Starvation Cap Reductn	324	319		0	0	0
Spillback Cap Reductn	0	0		33	0	0
Storage Cap Reductn	0	0		0	0	0
Reduced v/c Ratio	0.79	0.40		0.83	0.69	0.38

Cycle Length: 100
Actuated Cycle Length: 100

Offset: 31 (31%), Referenced to phase 2:WBTL and 6:EBT, Start of Green

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.80 Intersection Signal Delay: 20.6 Intersection Capacity Utilization 101.4%

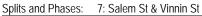
Intersection LOS: C
ICU Level of Service G

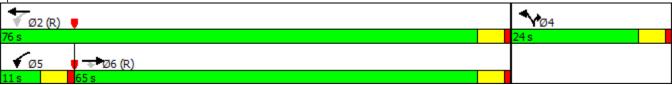
Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.





	-	-	4	<b>←</b>	*	4
Lane Group	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	<b>*</b>	7	¥	<b>*</b>	7	7
Traffic Volume (vph)	621	149	44	666	203	22
Future Volume (vph)	621	149	44	666	203	22
Satd. Flow (prot)	1589	1297	1510	1526	1510	1351
Flt Permitted			0.200		0.950	
Satd. Flow (perm)	1589	1238	318	1526	1510	1351
Satd. Flow (RTOR)						
Lane Group Flow (vph)	679	163	48	728	222	24
Turn Type	NA	Perm	pm+pt	NA	Prot	pt+ov
Protected Phases	4		3	8	2	2 3
Permitted Phases		4	8			
Total Split (s)	47.0	47.0	11.0	58.0	22.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	
Act Effct Green (s)	33.5	33.5	38.9	38.9	14.4	26.6
Actuated g/C Ratio	0.52	0.52	0.60	0.60	0.22	0.41
v/c Ratio	0.82	0.25	0.15	0.79	0.66	0.04
Control Delay	24.5	11.2	5.8	16.5	37.8	17.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	24.5	11.2	5.8	16.5	37.8	17.4
LOS	С	В	Α	В	D	В
Approach Delay	21.9			15.8	35.8	
Approach LOS	С			В	D	
Queue Length 50th (ft)	263	42	7	202	100	8
Queue Length 95th (ft)	#478	78	18	342	#197	24
Internal Link Dist (ft)	1242			517	1622	
Turn Bay Length (ft)		200	100			150
Base Capacity (vph)	1100	857	316	1221	444	541
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.62	0.19	0.15	0.60	0.50	0.04

Cycle Length: 80

Actuated Cycle Length: 64.4

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.82 Intersection Signal Delay: 21.2

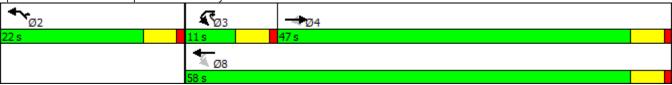
Intersection Capacity Utilization 64.1%

Intersection LOS: C ICU Level of Service C

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 8: Swampscott Mall Driveway & Essex St



	*	Ì	<b>F</b>	×	ን	~			
Lane Group	SET	SER	NWL	NWT	NEL	NER	Ø9		
Lane Configurations	•	7	7	•	W				
Traffic Volume (vph)	680	120	37	900	15	15			
Future Volume (vph)	680	120	37	900	15	15			
Satd. Flow (prot)	1695	1501	1678	1695	1530	0			
Flt Permitted			0.282		0.976				
Satd. Flow (perm)	1695	1452	497	1695	1487	0			
Satd. Flow (RTOR)									
Lane Group Flow (vph)	776	137	42	1027	34	0			
Turn Type	NA	Perm	Perm	NA	Prot				
Protected Phases	6			2	4		9		
Permitted Phases		6	2						
Total Split (s)	52.0	52.0	52.0	52.0	13.0		25.0		
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0				
Act Effct Green (s)	54.4	54.4	54.4	54.4	8.6				
Actuated g/C Ratio	0.81	0.81	0.81	0.81	0.13				
v/c Ratio	0.57	0.12	0.10	0.75	0.17				
Control Delay	13.4	7.8	9.6	19.1	36.2				
Queue Delay	0.0	0.0	0.0	0.0	0.0				
Total Delay	13.4	7.8	9.6	19.1	36.2				
LOS	В	Α	Α	В	D				
Approach Delay	12.6			18.8	36.2				
Approach LOS	В			В	D				
Queue Length 50th (ft)	0	0	0	0	9				
Queue Length 95th (ft)	#617	71	31	#934	48				
Internal Link Dist (ft)	497			268	259				
Turn Bay Length (ft)		150	150						
Base Capacity (vph)	1371	1174	402	1371	195				
Starvation Cap Reductn	17	0	0	0	0				
Spillback Cap Reductn	0	0	0	0	0				
Storage Cap Reductn	0	0	0	0	0				
Reduced v/c Ratio	0.57	0.12	0.10	0.75	0.17				

Cycle Length: 90

Actuated Cycle Length: 67.2

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.75

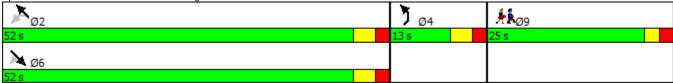
Intersection Signal Delay: 16.3 Intersection Capacity Utilization 64.7% Intersection LOS: B ICU Level of Service C

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 9: Harrison Rd & Loring Ave



	M	<b>†</b>	<b>↓</b>	لر	Ť	4		
Lane Group	NBL	NBT	SBT	SBR	NEL	NER	Ø9	
Lane Configurations		ર્ન	ĥ		W			
Traffic Volume (vph)	5	939	810	12	36	11		
Future Volume (vph)	5	939	810	12	36	11		
Satd. Flow (prot)	0	1731	1727	0	1613	0		
Flt Permitted		0.996			0.963			
Satd. Flow (perm)	0	1724	1727	0	1613	0		
Satd. Flow (RTOR)								
Lane Group Flow (vph)	0	1078	938	0	54	0		
Turn Type	Perm	NA	NA		Prot			
Protected Phases		2	6		4		9	
Permitted Phases	2							
Total Split (s)	54.0	54.0	54.0		12.0		19.0	
Total Lost Time (s)		6.0	5.0		5.0			
Act Effct Green (s)		57.7	58.3		6.8			
Actuated g/C Ratio		0.77	0.78		0.09			
v/c Ratio		0.81	0.70		0.37			
Control Delay		20.2	14.9		42.2			
Queue Delay		0.7	0.0		0.0			
Total Delay		20.9	14.9		42.2			
LOS		С	В		D			
Approach Delay		20.9	14.9		42.2			
Approach LOS		С	В		D			
Queue Length 50th (ft)		241	157		21			
Queue Length 95th (ft)		#900	#722		65			
Internal Link Dist (ft)		497	670		323			
Turn Bay Length (ft)								
Base Capacity (vph)		1329	1345		152			
Starvation Cap Reductn		66	0		0			
Spillback Cap Reductn		0	0		0			
Storage Cap Reductn		0	0		0			
Reduced v/c Ratio		0.85	0.70		0.36			

Cycle Length: 85

Actuated Cycle Length: 74.8 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.81

 Intersection Signal Delay: 18.7
 Intersection LOS: B

 Intersection Capacity Utilization 70.2%
 ICU Level of Service C

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 10: Pickman Rd & Loring Ave



	-	¬₄	~	•	<b>~</b>	4
Movement	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	<b>*</b>	7	ች	<b>+</b>	W	
Traffic Volume (veh/h)	785	25	67	628	5	152
Future Volume (Veh/h)	785	25	67	628	5	152
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Hourly flow rate (vph)	859	27	73	687	5	166
Pedestrians	5			5	5	
Lane Width (ft)	11.0			11.0	11.0	
Walking Speed (ft/s)	3.0			3.0	3.0	
Percent Blockage	1			1	1	
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)				914		
pX, platoon unblocked					0.77	
vC, conflicting volume			891		1702	869
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			891		1763	869
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			90		92	52
cM capacity (veh/h)			749		63	345
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NW 1	
Volume Total	859	27	73	687	171	
Volume Left	0	0	73	0	5	
Volume Right	0	27	0	0	166	
cSH	1700	1700	749	1700	305	
Volume to Capacity	0.51	0.02	0.10	0.40	0.56	
Queue Length 95th (ft)	0	0	8	0	80	
Control Delay (s)	0.0	0.0	10.3	0.0	30.9	
Lane LOS			В		D	
Approach Delay (s)	0.0		1.0		30.9	
Approach LOS					D	
Intersection Summary						
Average Delay			3.3			
Intersection Capacity Utilization			68.4%	IC	U Level of	Sorvico
			15	IC	o Level Ol	Service
Analysis Period (min)			15			

	۶	<b>→</b>	<b>←</b>	4	<b>\</b>	4
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	<b>1</b> >		W	
Traffic Volume (veh/h)	162	352	617	174	73	62
Future Volume (Veh/h)	162	352	617	174	73	62
Sign Control	102	Free	Free	177	Stop	02
Grade		0%	0%		0%	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	187	406	712	201	84	72
Pedestrians	107	10	10	201	10	12
Lane Width (ft)		11.0	11.0		11.0	
Walking Speed (ft/s)		3.0	3.0		3.0	
		3.0	3.0 1		3.0	
Percent Blockage Right turn flare (veh)		ı	ı		ı	
		None	Mone			
Median type		None	None			
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked	000				1/10	022
vC, conflicting volume	923				1612	832
vC1, stage 1 conf vol						
vC2, stage 2 conf vol					4440	
vCu, unblocked vol	923				1612	832
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	74				0	80
cM capacity (veh/h)	724				82	358
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	593	913	156			
Volume Left	187	0	84			
Volume Right	0	201	72			
cSH	724	1700	128			
Volume to Capacity	0.26	0.54	1.22			
Queue Length 95th (ft)	26	0	240			
Control Delay (s)	6.4	0.0	216.6			
Lane LOS	Α		F			
Approach Delay (s)	6.4	0.0	216.6			
Approach LOS			F			
Intersection Summary						
Average Delay			22.6			
Intersection Capacity Utilization			94.3%	IC	CU Level of	Service
Analysis Period (min)			15	10	, o Lovel O	JOI VICC
Analysis Fellou (IIIIII)			10			

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Movement	SBL	SBR	NEL	NET	SWT	SWR
Lane Configurations	W			ર્ન	<b>f</b> a	
Traffic Volume (veh/h)	88	28	23	428	666	145
Future Volume (Veh/h)	88	28	23	428	666	145
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	99	32	26	483	752	164
Pedestrians	30			30	30	
Lane Width (ft)	11.0			11.0	11.0	
Walking Speed (ft/s)	3.0			3.0	3.0	
Percent Blockage	3			3	3	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				110110	110110	
Upstream signal (ft)				706		
pX, platoon unblocked	0.85			700		
vC, conflicting volume	1429	894	946			
vC1, stage 1 conf vol	,	٠,٠	, , , ,			
vC2, stage 2 conf vol						
vCu, unblocked vol	1417	894	946			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	15	90	96			
cM capacity (veh/h)	116	319	703			
Direction, Lane #	SB 1	NE 1	SW 1			
Volume Total	131	509	916			
Volume Left	99	26	0			
Volume Right	32	0	164			
cSH	138	703	1700			
Volume to Capacity	0.95	0.04	0.54			
Queue Length 95th (ft)	165	3	0.54			
	126.5	1.0	0.0			
Control Delay (s) Lane LOS	126.5 F	1.0 A	0.0			
	126.5	1.0	0.0			
Approach LOS	126.5 F	1.0	0.0			
Approach LOS	F					
Intersection Summary						
Average Delay			11.0			
Intersection Capacity Utilization			64.4%	IC	U Level of	Service
Analysis Period (min)			15			

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	<b>→</b>	-	~	<b>←</b>	*	4	
Movement	EBT	EBR	WBL	WBT	NWL	NWR	
Lane Configurations	<b>1</b> 2			€Î	W		
Traffic Volume (veh/h)	438	77	32	647	73	76	
Future Volume (Veh/h)	438	77	32	647	73	76	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	
Hourly flow rate (vph)	505	89	37	747	84	88	
Pedestrians	10			10	10		
Lane Width (ft)	11.0			11.0	11.0		
Walking Speed (ft/s)	3.0			3.0	3.0		
Percent Blockage	1			1	1		
Right turn flare (veh)							
Median type	None			None			
Median storage veh)							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume			604		1390	570	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			604		1390	570	
tC, single (s)			*6.2		6.4	6.2	
tC, 2 stage (s)							
tF (s)			*3.3		3.5	3.3	
p0 queue free %			93		41	83	
cM capacity (veh/h)			497		142	511	
	FD 1	WD 1					
Direction, Lane # Volume Total	EB 1 594	WB 1 784	NW 1 172				
		784 37					
Volume Left	0 89		84				
Volume Right		0	88				
CSH Valume to Conneity	1700	497	225				
Volume to Capacity	0.35	0.07	0.76				
Queue Length 95th (ft)	0	6	134				
Control Delay (s)	0.0	2.2	58.9				
Lane LOS		A	F				
Approach Delay (s)	0.0	2.2	58.9				
Approach LOS			F				
Intersection Summary							
Average Delay			7.7				
Intersection Capacity Utilization			80.5%	IC	U Level of	Service	
Analysis Period (min)			15				
, ,							
* User Entered Value							

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			ર્ન	₽	
Traffic Volume (veh/h)	8	63	193	212	189	52
Future Volume (Veh/h)	8	63	193	212	189	52
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	72	220	242	216	59
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (ft)					437	
pX, platoon unblocked						
vC, conflicting volume	928	246	275			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	928	246	275			
tC, single (s)	6.4	6.2	*6.4			
tC, 2 stage (s)						
tF (s)	3.5	3.3	*3.3			
p0 queue free %	96	91	71			
cM capacity (veh/h)	211	793	757			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	81	462	275			
Volume Left	9	220	0			
Volume Right	72	0	59			
cSH	607	757	1700			
Volume to Capacity	0.13	0.29	0.16			
Queue Length 95th (ft)	11	30	0.10			
Control Delay (s)	11.8	7.6	0.0			
Lane LOS	В	Α.	0.0			
Approach Delay (s)	11.8	7.6	0.0			
Approach LOS	В	7.0	0.0			
Intersection Summary						
Average Delay			5.5			
Intersection Capacity Utilization			51.2%	IC	U Level of S	Service
Analysis Period (min)			15			
* User Entered Value						

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Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		₩			4			4			4	
Traffic Volume (vph)	28	20	2	1	8	2	2	625	13	5	420	15
Future Volume (vph)	28	20	2	1	8	2	2	625	13	5	420	15
Satd. Flow (prot)	0	1743	0	0	1752	0	0	1795	0	0	1790	0
Flt Permitted		0.976			0.963			0.999			0.992	
Satd. Flow (perm)	0	1749	0	0	1694	0	0	1793	0	0	1777	0
Satd. Flow (RTOR)		2			2			2			3	
Lane Group Flow (vph)	0	57	0	0	12	0	0	723	0	0	497	0
Turn Type	Perm	NA										
Protected Phases		6			2			4			8	
Permitted Phases	6			2			4			8		
Total Split (s)	12.0	12.0		12.0	12.0		40.0	40.0		40.0	40.0	
Total Lost Time (s)		6.0			6.0			6.0			6.0	
Act Effct Green (s)		6.4			6.4			36.1			36.1	
Actuated g/C Ratio		0.13			0.13			0.74			0.74	
v/c Ratio		0.24			0.05			0.54			0.38	
Control Delay		27.1			24.6			11.5			8.3	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		27.1			24.6			11.5			8.3	
LOS		С			С			В			Α	
Approach Delay		27.1			24.6			11.5			8.3	
Approach LOS		С			С			В			Α	
Queue Length 50th (ft)		14			3			101			57	
Queue Length 95th (ft)		60			20			#515			264	
Internal Link Dist (ft)		155			218			904			626	
Turn Bay Length (ft)												
Base Capacity (vph)		233			226			1345			1333	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.24			0.05			0.54			0.37	

Cycle Length: 75

Actuated Cycle Length: 48.7

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.54

Intersection Signal Delay: 11.1 Intersection Capacity Utilization 56.1%

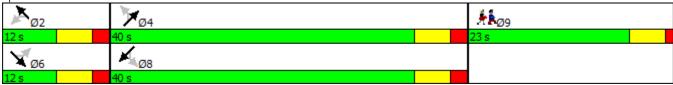
Intersection LOS: B ICU Level of Service B

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.





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Lane Group	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	7	7	<b>∱</b> 1≽		*	<b>^</b>
Traffic Volume (vph)	99	100	722	23	145	604
Future Volume (vph)	99	100	722	23	145	604
Satd. Flow (prot)	1540	1378	2419	0	1215	2431
Flt Permitted	0.950				0.273	
Satd. Flow (perm)	1540	1378	2419	0	349	2431
Satd. Flow (RTOR)						
Lane Group Flow (vph)	108	109	815	0	159	661
Turn Type	Prot	pt+ov	NA		pm+pt	NA
Protected Phases	2	2 3	4		3	8
Permitted Phases					8	
Total Split (s)	19.0		52.0		19.0	71.0
Total Lost Time (s)	5.0		5.0		6.0	5.0
Act Effct Green (s)	11.1	26.2	53.8		67.9	68.9
Actuated g/C Ratio	0.12	0.29	0.60		0.75	0.77
v/c Ratio	0.57	0.27	0.56		0.45	0.36
Control Delay	48.5	24.9	13.9		5.5	0.5
Queue Delay	0.0	0.0	0.0		0.0	0.0
Total Delay	48.5	24.9	13.9		5.5	0.5
LOS	D	С	В		А	А
Approach Delay	36.7		13.9			1.5
Approach LOS	D		В			А
Queue Length 50th (ft)	58	47	137		1	1
Queue Length 95th (ft)	108	82	226		m15	1
Internal Link Dist (ft)	133		258			783
Turn Bay Length (ft)					150	
Base Capacity (vph)	239	437	1446		388	1861
Starvation Cap Reductn	0	0	0		0	0
Spillback Cap Reductn	0	0	0		0	0
Storage Cap Reductn	0	0	0		0	0
Reduced v/c Ratio	0.45	0.25	0.56		0.41	0.36

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 84 (93%), Referenced to phase 4:NET and 8:SWTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.57

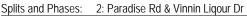
Intersection Signal Delay: 11.1

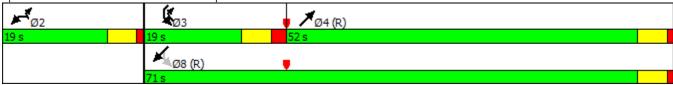
Intersection LOS: B
ICU Level of Service B

Intersection Capacity Utilization 62.2%

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.





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3: Paradise Rd & Swampscott Mall Driveway/Shopping Dri	ve

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	75	f)		Ŋ.	f)		ň	<b>∱</b> β		¥	<b>∱</b> β	
Traffic Volume (vph)	196	60	182	90	87	25	181	695	60	39	494	144
Future Volume (vph)	196	60	182	90	87	25	181	695	60	39	494	144
Satd. Flow (prot)	1540	1409	0	1540	1558	0	1215	2369	0	1215	2301	0
Flt Permitted	0.679			0.413			0.389			0.224		
Satd. Flow (perm)	1100	1409	0	669	1558	0	498	2369	0	287	2301	0
Satd. Flow (RTOR)												
Lane Group Flow (vph)	212	262	0	97	121	0	196	817	0	42	691	0
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		6			2		7	4		3	8	
Permitted Phases	6			2			4			8		
Total Split (s)	31.0	31.0		31.0	31.0		18.0	48.0		11.0	41.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	6.0		5.0	5.0	
Act Effct Green (s)	21.6	21.6		21.6	21.6		51.5	50.5		40.4	40.4	
Actuated g/C Ratio	0.24	0.24		0.24	0.24		0.57	0.56		0.45	0.45	
v/c Ratio	0.80	0.78		0.61	0.32		0.51	0.61		0.22	0.67	
Control Delay	54.3	47.3		45.8	29.2		15.8	11.3		19.1	24.7	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	54.3	47.3		45.8	29.2		15.8	11.3		19.1	24.7	
LOS	D	D		D	С		В	В		В	С	
Approach Delay		50.4			36.6			12.1			24.4	
Approach LOS		D			D			В			С	
Queue Length 50th (ft)	111	136		48	55		35	82		14	164	
Queue Length 95th (ft)	#201	215		98	99		56	104		36	244	
Internal Link Dist (ft)		1630			222			783			1420	
Turn Bay Length (ft)	150						500			150		
Base Capacity (vph)	317	407		193	450		388	1330		193	1032	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.67	0.64		0.50	0.27		0.51	0.61		0.22	0.67	

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 4:NETL and 8:SWTL, Start of Green, Master Intersection

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.80

Intersection Signal Delay: 25.5

Intersection Capacity Utilization 82.0%

Intersection LOS: C

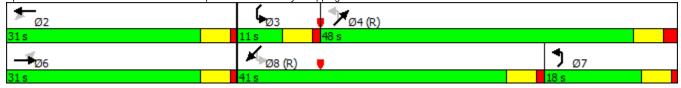
ICU Level of Service E

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Paradise Rd & Swampscott Mall Driveway/Shopping Drive



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 Synchro 9 Report

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	•	<b>→</b>	$\rightarrow$	•	<b>←</b>	•	•	<b>†</b>	/	<b>&gt;</b>	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	f)		7	<b>+</b>	7		41≯	7		4î.	
Traffic Volume (vph)	11	376	59	217	310	64	32	520	358	99	441	16
Future Volume (vph)	11	376	59	217	310	64	32	520	358	99	441	16
Satd. Flow (prot)	1215	1251	0	1215	1279	1088	0	3070	1378	0	3037	0
Flt Permitted	0.555			0.187				0.884			0.601	
Satd. Flow (perm)	710	1251	0	239	1279	1073	0	2722	1354	0	1842	0
Satd. Flow (RTOR)									*100			
Lane Group Flow (vph)	12	481	0	240	343	71	0	610	396	0	614	0
Turn Type	Perm	NA		pm+pt	NA	Perm	Perm	NA	Perm	pm+pt	NA	
Protected Phases		6		5	2			4		3	8	
Permitted Phases	6			2		2	4		4	8		
Total Split (s)	39.0	39.0		15.0	54.0	54.0	35.0	35.0	35.0	11.0	46.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0		5.0	5.0		5.0	
Act Effct Green (s)	34.0	34.0		49.0	49.0	49.0		30.0	30.0		41.0	
Actuated g/C Ratio	0.34	0.34		0.49	0.49	0.49		0.30	0.30		0.41	
v/c Ratio	0.05	1.13		1.12	0.55	0.14		0.75	0.83		0.74	
Control Delay	9.6	96.3		122.2	18.7	13.9		38.3	40.9		30.8	
Queue Delay	0.0	0.3		0.0	3.3	0.0		0.0	0.1		0.0	
Total Delay	9.6	96.6		122.2	22.0	13.9		38.3	41.0		30.8	
LOS	Α	F		F	С	В		D	D		С	
Approach Delay		94.5			57.9			39.4			30.8	
Approach LOS		F			Е			D			С	
Queue Length 50th (ft)	3	~369		~116	108	19		184	180		120	
Queue Length 95th (ft)	m3	m#420		#277	190	m46		251	#346		m163	
Internal Link Dist (ft)		529			213			1420			571	
Turn Bay Length (ft)	150								150			
Base Capacity (vph)	241	425		214	626	525		816	476		826	
Starvation Cap Reductn	0	0		0	189	0		0	0		0	
Spillback Cap Reductn	0	13		0	0	0		0	1		0	
Storage Cap Reductn	0	0		0	0	0		0	0		0	
Reduced v/c Ratio	0.05	1.17		1.12	0.78	0.14		0.75	0.83		0.74	

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 0 (0%), Referenced to phase 4:NBTL and 8:SBTL, Start of Green, Master Intersection

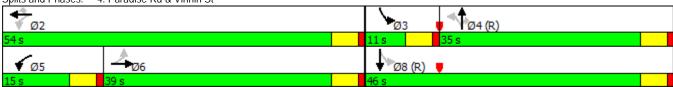
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 1.13
Intersection Signal Delay: 51.7
Intersection Capacity Utilization 105.0%

Intersection LOS: D
ICU Level of Service G

Analysis Period (min) 15

- * User Entered Value
- Volume exceeds capacity, queue is theoretically infinite.
   Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 4: Paradise Rd & Vinnin St



	•	•	4	<b>†</b>	<b>↓</b>	1
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Ť	7	ř	<b>*</b>	<b>†</b>	7
Traffic Volume (vph)	287	24	15	573	521	289
Future Volume (vph)	287	24	15	573	521	289
Satd. Flow (prot)	1191	1378	1540	1588	1588	1350
Flt Permitted	0.950		0.196			
Satd. Flow (perm)	1191	1338	318	1588	1588	1286
Satd. Flow (RTOR)						
Lane Group Flow (vph)	331	28	17	661	601	333
Turn Type	Prot	Perm	pm+pt	NA	NA	Perm
Protected Phases	4		5	2	6	
Permitted Phases		4	2			6
Total Split (s)	39.0	39.0	11.0	61.0	50.0	50.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0
Act Effct Green (s)	30.9	30.9	59.1	59.1	45.0	45.0
Actuated g/C Ratio	0.31	0.31	0.59	0.59	0.45	0.45
v/c Ratio	0.90	0.07	0.06	0.70	0.84	0.58
Control Delay	36.2	5.2	3.4	19.1	37.1	25.3
Queue Delay	0.0	0.0	0.0	0.1	0.0	0.0
Total Delay	36.2	5.2	3.4	19.2	37.1	25.3
LOS	D	Α	А	В	D	С
Approach Delay	33.7			18.8	32.9	
Approach LOS	С			В	С	
Queue Length 50th (ft)	19	1	1	169	329	153
Queue Length 95th (ft)	#338	m2	m2	226	#536	246
Internal Link Dist (ft)	691			571	296	
Turn Bay Length (ft)		150				
Base Capacity (vph)	404	454	299	938	714	578
Starvation Cap Reductn	0	0	0	20	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.82	0.06	0.06	0.72	0.84	0.58

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 99 (99%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

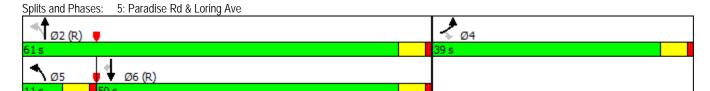
Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.90 Intersection Signal Delay: 28.2 Intersection Capacity Utilization 67.0%

Intersection LOS: C
ICU Level of Service C

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.



	_#	-	7	<b>F</b>	<b>←</b>	۲	•	×	/	6	K	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		€}-		7	f)		ř	•	7	¥	ĵ.	
Traffic Volume (vph)	2	1	2	314	6	25	7	298	440	25	292	4
Future Volume (vph)	2	1	2	314	6	25	7	298	440	25	292	4
Satd. Flow (prot)	0	1445	0	1215	1060	0	1296	1337	1160	1296	1334	0
Flt Permitted		0.965		0.754			0.557			0.352		
Satd. Flow (perm)	0	1422	0	965	1060	0	760	1337	1105	480	1334	0
Satd. Flow (RTOR)									*200			
Lane Group Flow (vph)	0	5	0	358	36	0	8	340	502	29	338	0
Turn Type	Perm	NA		Perm	NA		Perm	NA	Perm	pm+pt	NA	
Protected Phases		6			2			4		3	8	
Permitted Phases	6			2			4		4	8		
Total Split (s)	48.0	48.0		48.0	48.0		41.0	41.0	41.0	11.0	52.0	
Total Lost Time (s)		5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Act Effct Green (s)		47.7		47.7	47.7		35.7	35.7	35.7	42.3	42.3	
Actuated g/C Ratio		0.48		0.48	0.48		0.36	0.36	0.36	0.42	0.42	
v/c Ratio		0.01		0.78	0.07		0.03	0.71	0.96	0.12	0.60	
Control Delay		16.4		33.3	11.5		21.3	37.2	51.0	1.8	7.1	
Queue Delay		0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Delay		16.4		33.3	11.5		21.3	37.2	51.0	1.8	7.1	
LOS		В		С	В		С	D	D	Α	Α	
Approach Delay		16.4			31.3			45.2			6.7	
Approach LOS		В			С			D			Α	
Queue Length 50th (ft)		2		244	14		3	184	208	1	10	
Queue Length 95th (ft)		9		m#390	m23		14	292	#432	m1	9	
Internal Link Dist (ft)		69			529			965			691	
Turn Bay Length (ft)				150			100		150	150		
Base Capacity (vph)		677		459	505		280	494	534	252	626	
Starvation Cap Reductn		0		0	0		0	0	0	0	0	
Spillback Cap Reductn		0		0	0		0	0	0	0	0	
Storage Cap Reductn		0		0	0		0	0	0	0	0	
Reduced v/c Ratio		0.01		0.78	0.07		0.03	0.69	0.94	0.12	0.54	

Cycle Length: 100
Actuated Cycle Length: 100

Offset: 69 (69%), Referenced to phase 2:WBTL and 6:EBTL, Start of Green

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.96 Intersection Signal Delay: 32.9

Intersection LOS: C ICU Level of Service C

Intersection Capacity Utilization 71.3%

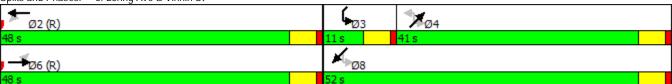
Analysis Period (min) 15

* User Entered Value

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Loring Ave & Vinnin St



	-	•	•	•	•	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>^</b>	7		41₽	*	7
Traffic Volume (vph)	648	197	70	441	121	97
Future Volume (vph)	648	197	70	441	121	97
Satd. Flow (prot)	1588	1378	0	2414	1191	1088
Flt Permitted				0.771	0.950	
Satd. Flow (perm)	1588	1378	0	1874	1191	1088
Satd. Flow (RTOR)						
Lane Group Flow (vph)	716	218	0	564	134	107
Turn Type	NA	Perm	pm+pt	NA	Prot	Prot
Protected Phases	6		5	2	4	4
Permitted Phases		6	2			
Total Split (s)	65.0	65.0	11.0	76.0	24.0	24.0
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0
Act Effct Green (s)	74.6	74.6		74.6	15.4	15.4
Actuated g/C Ratio	0.75	0.75		0.75	0.15	0.15
v/c Ratio	0.60	0.21		0.40	0.73	0.64
Control Delay	3.0	1.3		6.1	62.5	56.5
Queue Delay	3.3	0.9		0.0	0.4	0.0
Total Delay	6.3	2.1		6.1	62.9	56.5
LOS	А	Α		Α	Е	Е
Approach Delay	5.3			6.1	60.0	
Approach LOS	А			Α	Е	
Queue Length 50th (ft)	36	5		60	81	64
Queue Length 95th (ft)	m105	m14		97	143	119
Internal Link Dist (ft)	213			175	347	
Turn Bay Length (ft)						150
Base Capacity (vph)	1184	1028		1398	226	206
Starvation Cap Reductn	359	559		0	0	0
Spillback Cap Reductn	0	0		0	7	0
Storage Cap Reductn	0	0		0	0	0
Reduced v/c Ratio	0.87	0.46		0.40	0.61	0.52

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 76 (76%), Referenced to phase 2:WBTL and 6:EBT, Start of Green

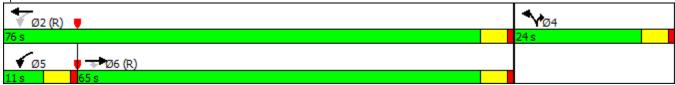
Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.73 Intersection Signal Delay: 13.2 Intersection Capacity Utilization 83.2%

Intersection LOS: B ICU Level of Service E

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 7: Salem St & Vinnin St



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	-	•	•	←	<b>1</b>	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b>	7	7	<b>*</b>	*	7
Traffic Volume (vph)	693	254	66	631	259	108
Future Volume (vph)	693	254	66	631	259	108
Satd. Flow (prot)	1459	1240	1540	1588	1540	1378
Flt Permitted			0.171		0.950	
Satd. Flow (perm)	1459	1240	277	1588	1540	1378
Satd. Flow (RTOR)						
Lane Group Flow (vph)	782	287	75	712	292	122
Turn Type	NA	Perm	pm+pt	NA	Prot	pt+ov
Protected Phases	4		3	8	2	2 3
Permitted Phases		4	8			
Total Split (s)	65.0	65.0	11.0	76.0	29.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	
Act Effct Green (s)	56.2	56.2	67.4	67.4	21.8	32.9
Actuated g/C Ratio	0.57	0.57	0.68	0.68	0.22	0.33
v/c Ratio	0.95	0.41	0.28	0.66	0.86	0.27
Control Delay	42.7	14.5	8.5	13.3	63.5	27.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	42.7	14.5	8.5	13.3	63.5	27.4
LOS	D	В	Α	В	Е	С
Approach Delay	35.1			12.9	52.9	
Approach LOS	D			В	D	
Queue Length 50th (ft)	457	101	15	251	188	59
Queue Length 95th (ft)	#743	163	30	377	#330	107
Internal Link Dist (ft)	1242			509	1630	
Turn Bay Length (ft)		200	100			150
Base Capacity (vph)	893	759	265	1150	377	448
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.88	0.38	0.28	0.62	0.77	0.27

Cycle Length: 105
Actuated Cycle Length: 99.3

Control Type: Actuated-Uncoordinated

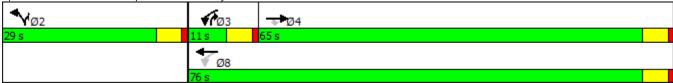
Maximum v/c Ratio: 0.95 Intersection Signal Delay: 30.6 Intersection Capacity Utilization 76.8%

Intersection LOS: C
ICU Level of Service D

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 8: Swampscott Mall Driveway & Essex St



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Lane Group	SET	SER	NWL	NWT	NEL	NER	Ø9		
Lane Configurations	<b>*</b>	7	ሻ	<b>^</b>	W				
Traffic Volume (vph)	972	120	37	826	10	15			
Future Volume (vph)	972	120	37	826	10	15			
Satd. Flow (prot)	1493	1218	1621	1433	1622	0			
Flt Permitted			0.149		0.981				
Satd. Flow (perm)	1493	1183	254	1433	1622	0			
Satd. Flow (RTOR)									
Lane Group Flow (vph)	1109	137	42	943	28	0			
Turn Type	NA	Perm	Perm	NA	Prot				
Protected Phases	6			2	4		9		
Permitted Phases		6	2						
Total Split (s)	62.0	62.0	62.0	62.0	13.0		25.0		
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0				
Act Effct Green (s)	62.5	62.5	62.5	62.5	8.3				
Actuated g/C Ratio	0.87	0.87	0.87	0.87	0.11				
v/c Ratio	0.86	0.13	0.19	0.76	0.15				
Control Delay	19.4	4.9	9.0	14.7	37.0				
Queue Delay	0.1	0.0	0.0	0.0	0.0				
Total Delay	19.5	4.9	9.0	14.7	37.0				
LOS	В	Α	Α	В	D				
Approach Delay	17.9			14.5	37.0				
Approach LOS	В			В	D				
Queue Length 50th (ft)	0	0	0	0	10				
Queue Length 95th (ft)	#1159	73	39	#951	45				
Internal Link Dist (ft)	486			296	259				
Turn Bay Length (ft)		150	150						
Base Capacity (vph)	1293	1024	220	1241	186				
Starvation Cap Reductn	5	0	0	0	0				
Spillback Cap Reductn	0	0	0	0	0				
Storage Cap Reductn	0	0	0	0	0				
Reduced v/c Ratio	0.86	0.13	0.19	0.76	0.15				

Cycle Length: 100 Actuated Cycle Length: 72.2

Control Type: Actuated-Uncoordinated Maximum v/c Ratio: 0.86

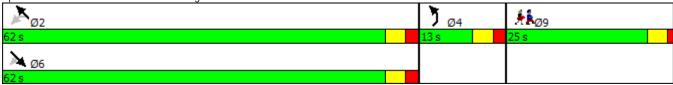
Intersection Signal Delay: 16.6 Intersection Capacity Utilization 71.7% Intersection LOS: B ICU Level of Service C

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 9: Harrison Rd & Loring Ave



	*	<b>†</b>	<b>↓</b>	لر	<b>*</b>	4		
Lane Group	NBL	NBT	SBT	SBR	NEL	NER	Ø9	
Lane Configurations		ર્ન	ĵ.		N/A			
Traffic Volume (vph)	14	786	1044	25	22	11		
Future Volume (vph)	14	786	1044	25	22	11		
Satd. Flow (prot)	0	1511	1508	0	1663	0		
Flt Permitted		0.753			0.968			
Satd. Flow (perm)	0	1139	1508	0	1663	0		
Satd. Flow (RTOR)								
Lane Group Flow (vph)	0	913	1221	0	38	0		
Turn Type	Perm	NA	NA		Prot			
Protected Phases		2	6		4		9	
Permitted Phases	2							
Total Split (s)	54.0	54.0	54.0		12.0		24.0	
Total Lost Time (s)		6.0	5.0		5.0			
Act Effct Green (s)		54.0	54.3		6.8			
Actuated g/C Ratio		0.85	0.86		0.11			
v/c Ratio		0.94	0.94		0.21			
Control Delay		31.7	27.8		33.9			
Queue Delay		0.0	0.0		0.0			
Total Delay		31.7	27.8		33.9			
LOS		С	С		С			
Approach Delay		31.7	27.8		33.9			
Approach LOS		С	С		С			
Queue Length 50th (ft)		0	0		11			
Queue Length 95th (ft)		#931	#1179		51			
Internal Link Dist (ft)		486	689		323			
Turn Bay Length (ft)								
Base Capacity (vph)		973	1296		191			
Starvation Cap Reductn		0	0		0			
Spillback Cap Reductn		0	0		0			
Storage Cap Reductn		0	0		0			
Reduced v/c Ratio		0.94	0.94		0.20			

Cycle Length: 90

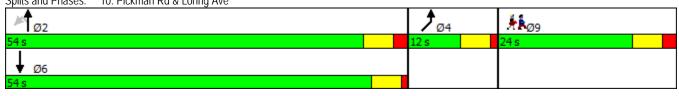
Actuated Cycle Length: 63.2 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.94

Intersection Signal Delay: 29.5 Intersection Capacity Utilization 72.6% Intersection LOS: C
ICU Level of Service C

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 10: Pickman Rd & Loring Ave



	-	¬₄	~	<b>←</b>	•	4
Movement	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	<b>^</b>	7	7	<b>†</b>	W	
Traffic Volume (veh/h)	774	46	164	823	12	89
Future Volume (Veh/h)	774	46	164	823	12	89
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	838	50	178	891	13	96
Pedestrians	10				10	
Lane Width (ft)	11.0				11.0	
Walking Speed (ft/s)	3.0				3.0	
Percent Blockage	1				1	
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)				922		
pX, platoon unblocked					0.40	
vC, conflicting volume			898		2105	848
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			898		3026	848
tC, single (s)			4.1		*6.0	*6.0
tC, 2 stage (s)						
tF (s)			2.2		*3.0	*3.0
p0 queue free %			76		0	76
cM capacity (veh/h)			748		6	403
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NW 1	
Volume Total	838	50	178	891	109	
Volume Left	0	0	178	0	13	
Volume Right	0	50	0	0	96	
cSH	1700	1700	748	1700	47	
Volume to Capacity	0.49	0.03	0.24	0.52	2.31	
Queue Length 95th (ft)	0.49	0.03	23	0.52	2.31	
Control Delay (s)	0.0	0.0	11.3	0.0	781.6	
Lane LOS	0.0	0.0	11.3 B	0.0	701.0 F	
Approach Delay (s)	0.0		1.9		781.6	
Approach LOS	0.0		1.9		781.0 F	
					F	
Intersection Summary						
Average Delay			42.2			
Intersection Capacity Utilization			68.8%	IC	U Level of	Service
Analysis Period (min)			15			
* User Entered Value						

	ၨ	<b>→</b>	•	•	<b>\</b>	4	
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		ર્ન	<b>1</b> 2		W		
Traffic Volume (veh/h)	133	534	436	100	112	187	
Future Volume (Veh/h)	133	534	436	100	112	187	
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	
Hourly flow rate (vph)	150	603	492	113	126	211	
Pedestrians							
_ane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type		None	None				
Median storage veh)							
Jpstream signal (ft)							
X, platoon unblocked							
C, conflicting volume	605				1452	548	
C1, stage 1 conf vol							
C2, stage 2 conf vol							
Cu, unblocked vol	605				1452	548	
C, single (s)	4.1				*6.2	6.2	
C, 2 stage (s)							
F(s)	2.2				3.5	3.3	
00 queue free %	85				5	61	
cM capacity (veh/h)	973				133	536	
Direction, Lane #	EB 1	WB 1	SB 1				
/olume Total	753	605	337				
/olume Left	150	0	126				
/olume Right	0	113	211				
SH	973	1700	251				
olume to Capacity	0.15	0.36	1.34				
Queue Length 95th (ft)	14	0	445				
Control Delay (s)	3.7	0.0	216.7				
Lane LOS	Α		F				
Approach Delay (s)	3.7	0.0	216.7				
Approach LOS			F				
ntersection Summary							
Average Delay			44.7				
ntersection Capacity Utilization			96.3%	I	CU Level of	Service	F
Analysis Period (min)			15				
User Entered Value							

	<b>→</b>	74	4	•	*	4
Movement	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	₽			4	W	
Traffic Volume (veh/h)	615	139	59	564	47	52
Future Volume (Veh/h)	615	139	59	564	47	52
Sign Control	Free		0,	Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	694	157	67	637	53	59
Pedestrians	071	107	07	007	00	0,
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)	INOTIC			None		
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			851		1544	772
vC1, stage 1 conf vol			001		1344	112
vC2, stage 2 conf vol						
vCu, unblocked vol			851		1544	772
			4.1			6.2
tC, single (s)			4.1		6.4	0.2
tC, 2 stage (s)			2.2		2.5	2.2
tF (s)					3.5	3.3
p0 queue free %			91		54	85
cM capacity (veh/h)			788		116	399
Direction, Lane #	EB 1	WB 1	NW 1			
Volume Total	851	704	112			
Volume Left	0	67	53			
Volume Right	157	0	59			
cSH	1700	788	185			
Volume to Capacity	0.50	0.09	0.61			
Queue Length 95th (ft)	0	7	84			
Control Delay (s)	0.0	2.2	50.6			
Lane LOS		Α	F			
Approach Delay (s)	0.0	2.2	50.6			
Approach LOS			F			
Intersection Summary						
Average Delay			4.3			
Intersection Capacity Utilization			93.5%	IC	U Level of	Service
Analysis Period (min)			15	10		2 3 100
Analysis Period (min)			15			

	ၨ	•	•	<b>†</b>	<del> </del>	<b>√</b>
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		EBK	NDL			SDK
Traffic Volume (veh/h)	<b>₩</b> 73	107	83	141	<b>↑</b>	86
Future Volume (Veh/h)	73	107	83	161 161	237 237	86
		107	83			80
Sign Control Grade	Stop			Free	Free	
	0%	0.00	0.00	0%	0%	0.00
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	83	122	95	184	270	98
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (ft)					427	
pX, platoon unblocked						
vC, conflicting volume	693	319	368			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	693	319	368			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	78	83	92			
cM capacity (veh/h)	377	722	1191			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	205	279	368			
Volume Left	83	95	300			
	122	95	98			
Volume Right cSH	526		1700			
		1191				
Volume to Capacity	0.39	0.08	0.22			
Queue Length 95th (ft)	46	6	0			
Control Delay (s)	16.1	3.3	0.0			
Lane LOS	С	А				
Approach Delay (s)	16.1	3.3	0.0			
Approach LOS	С					
Intersection Summary						
Average Delay			5.0			
Intersection Capacity Utilization			53.5%	IC	CU Level of S	Service
Analysis Period (min)			15			

	<b>y</b>	×	Ì	~	×	₹	ን	×	~	Ĺ	×	*
Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		€}-			€}-			4			4	
Traffic Volume (vph)	19	7	3	0	5	1	0	465	0	1	437	1
Future Volume (vph)	19	7	3	0	5	1	0	465	0	1	437	1
Satd. Flow (prot)	0	1720	0	0	1766	0	0	1801	0	0	1801	0
Flt Permitted											0.999	
Satd. Flow (perm)	0	1777	0	0	1766	0	0	1801	0	0	1799	0
Satd. Flow (RTOR)		3			1							
Lane Group Flow (vph)	0	32	0	0	7	0	0	525	0	0	495	0
Turn Type	Perm	NA			NA			NA		Perm	NA	
Protected Phases		6			2			4			8	
Permitted Phases	6			2			4			8		
Total Split (s)	13.0	13.0		13.0	13.0		39.0	39.0		39.0	39.0	
Total Lost Time (s)		6.0			6.0			6.0			6.0	
Act Effct Green (s)		6.9			6.7			38.9			38.9	
Actuated g/C Ratio		0.16			0.15			0.89			0.89	
v/c Ratio		0.11			0.03			0.33			0.31	
Control Delay		21.1			21.6			6.2			6.0	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		21.1			21.6			6.2			6.0	
LOS		С			С			Α			Α	
Approach Delay		21.1			21.6			6.2			6.0	
Approach LOS		С			С			Α			Α	
Queue Length 50th (ft)		5			1			0			0	
Queue Length 95th (ft)		38			14			291			269	
Internal Link Dist (ft)		155			218			904			594	
Turn Bay Length (ft)												
Base Capacity (vph)		311			308			1478			1477	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	
Reduced v/c Ratio		0.10			0.02			0.36			0.34	

Cycle Length: 75

Actuated Cycle Length: 43.8

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.33

Intersection Signal Delay: 6.7 Intersection Capacity Utilization 44.0%

Analysis Period (min) 15

Intersection LOS: A ICU Level of Service A



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Lane Group	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations	*	7	<b>∱</b> Ъ		ሻ	<b>^</b>
Traffic Volume (vph)	84	195	654	14	182	664
Future Volume (vph)	84	195	654	14	182	664
Satd. Flow (prot)	1296	1160	2573	0	1296	2583
Flt Permitted	0.950				0.280	
Satd. Flow (perm)	1296	1160	2573	0	382	2583
Satd. Flow (RTOR)						
Lane Group Flow (vph)	92	213	730	0	199	726
Turn Type	Prot	pt+ov	NA		pm+pt	NA
Protected Phases	2	2 3	4		3	8
Permitted Phases					8	
Total Split (s)	30.0		42.0		21.0	63.0
Total Lost Time (s)	5.0		5.0		6.0	5.0
Act Effct Green (s)	18.7	35.8	47.2		63.3	64.3
Actuated g/C Ratio	0.20	0.38	0.51		0.68	0.69
v/c Ratio	0.35	0.48	0.56		0.54	0.41
Control Delay	34.2	23.9	19.6		12.3	7.7
Queue Delay	0.0	0.0	0.0		0.0	0.0
Total Delay	34.2	23.9	19.6		12.3	7.7
LOS	С	С	В		В	Α
Approach Delay	27.0		19.6			8.7
Approach LOS	С		В			Α
Queue Length 50th (ft)	47	93	149		39	85
Queue Length 95th (ft)	85	129	255		85	146
Internal Link Dist (ft)	133		173			783
Turn Bay Length (ft)					150	
Base Capacity (vph)	348	492	1306		407	1784
Starvation Cap Reductn	0	0	0		0	0
Spillback Cap Reductn	0	0	0		0	0
Storage Cap Reductn	0	0	0		0	0
Reduced v/c Ratio	0.26	0.43	0.56		0.49	0.41

Cycle Length: 93

Actuated Cycle Length: 93

Offset: 0 (0%), Referenced to phase 4:NET and 8:SWTL, Start of Green

Control Type: Actuated-Coordinated

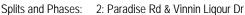
Maximum v/c Ratio: 0.56 Intersection Signal Delay: 15.6

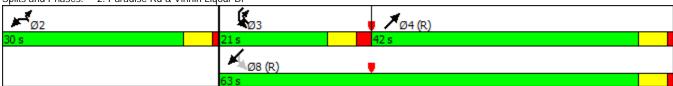
Intersection Capacity Utilization 61.6%

Analysis Period (min) 15

Intersection LOS: B

ICU Level of Service B





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Notice 1A-villilli Square Fliolity Collidor Study	
3: Paradise Rd & Swampscott Mall Driveway/Shopping	Drive

	_#	-	7	<b>*</b>	<b>←</b>	€.	•	×	/	6	×	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	75	f)		7	ĵ.		7	<b>∱</b> ∱≽		7	<b>∱</b> ∱≽	
Traffic Volume (vph)	213	95	212	115	136	27	187	537	43	54	523	229
Future Volume (vph)	213	95	212	115	136	27	187	537	43	54	523	229
Satd. Flow (prot)	1296	1197	0	1296	1316	0	1296	2548	0	1296	2448	0
Flt Permitted	0.595			0.370			0.253			0.359		
Satd. Flow (perm)	812	1197	0	505	1316	0	345	2548	0	490	2448	0
Satd. Flow (RTOR)												
Lane Group Flow (vph)	231	332	0	124	176	0	202	628	0	58	814	0
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		6			2		7	4		3	8	
Permitted Phases	6			2			4			8		
Total Split (s)	39.0	39.0		39.0	39.0		16.0	49.0		12.0	45.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	6.0		5.0	5.0	
Act Effct Green (s)	31.1	31.1		31.1	31.1		58.1	48.7		50.6	44.3	
Actuated g/C Ratio	0.31	0.31		0.31	0.31		0.58	0.49		0.51	0.44	
v/c Ratio	0.92	0.89		0.79	0.43		0.69	0.51		0.19	0.75	
Control Delay	72.6	59.5		65.3	30.2		36.5	21.0		13.5	30.2	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	72.6	59.5		65.3	30.2		36.5	21.0		13.5	30.2	
LOS	Е	Е		Е	С		D	С		В	С	
Approach Delay		64.9			44.7			24.8			29.1	
Approach LOS		Е			D			С			С	
Queue Length 50th (ft)	133	189		68	84		64	155		16	240	
Queue Length 95th (ft)	#273	#344		#167	146		#117	212		35	#330	
Internal Link Dist (ft)		1673			222			783			1428	
Turn Bay Length (ft)	150						500			150		
Base Capacity (vph)	277	408		172	449		309	1240		307	1084	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.83	0.81		0.72	0.39		0.65	0.51		0.19	0.75	

Cycle Length: 100 Actuated Cycle Length: 100

Offset: 0 (0%), Referenced to phase 4:NETL and 8:SWTL, Start of Green, Master Intersection

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.92 Intersection Signal Delay: 37.4 Intersection Capacity Utilization 95.5%

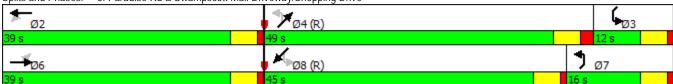
Intersection LOS: D ICU Level of Service F

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Paradise Rd & Swampscott Mall Driveway/Shopping Drive



	٠	<b>→</b>	•	•	<b>←</b>	•	•	†	<b>/</b>	<b>/</b>	Ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ħ	£		ř	<b>*</b>	7		414	7		4î}∍	
Traffic Volume (vph)	22	282	72	279	367	106	42	392	303	101	438	29
Future Volume (vph)	22	282	72	279	367	106	42	392	303	101	438	29
Satd. Flow (prot)	1459	1476	0	1459	1523	1305	0	2891	1305	0	2856	0
Flt Permitted	0.329			0.298				0.825			0.725	
Satd. Flow (perm)	505	1476	0	458	1523	1305	0	2397	1305	0	2090	0
Satd. Flow (RTOR)												
Lane Group Flow (vph)	24	392	0	308	406	117	0	479	335	0	628	0
Turn Type	Perm	NA		pm+pt	NA	Perm	Perm	NA	Perm	pm+pt	NA	
Protected Phases		6		5	2			4		3	8	
Permitted Phases	6			2		2	4		4	8		
Total Split (s)	30.0	30.0		16.0	46.0	46.0	33.0	33.0	33.0	11.0	44.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0		5.0	5.0		5.0	
Act Effct Green (s)	24.9	24.9		41.0	41.0	41.0		28.0	28.0		39.0	
Actuated g/C Ratio	0.28	0.28		0.46	0.46	0.46		0.31	0.31		0.43	
v/c Ratio	0.17	0.96		0.93	0.59	0.20		0.64	0.83		0.66	
Control Delay	9.6	43.1		59.5	20.5	15.5		31.4	47.6		13.8	
Queue Delay	0.0	0.8		0.0	6.1	0.0		0.0	0.0		0.0	
Total Delay	9.6	43.9		59.5	26.6	15.5		31.4	47.6		13.8	
LOS	А	D		Е	С	В		С	D		В	
Approach Delay		41.9			37.2			38.1			13.8	
Approach LOS		D			D			D			В	
Queue Length 50th (ft)	3	164		83	124	31		122	176		45	
Queue Length 95th (ft)	m4	m#282		#288	251	m67		177	#323		m75	
Internal Link Dist (ft)		529			213			1428			571	
Turn Bay Length (ft)	150								150			
Base Capacity (vph)	140	410		331	693	594		745	406		956	
Starvation Cap Reductn	0	0		0	229	0		0	0		0	
Spillback Cap Reductn	0	2		0	0	0		0	0		0	
Storage Cap Reductn	0	0		0	0	0		0	0		0	
Reduced v/c Ratio	0.17	0.96		0.93	0.88	0.20		0.64	0.83		0.66	

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 4:NBTL and 8:SBTL, Start of Green, Master Intersection

Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.96

Intersection Signal Delay: 32.7

Intersection Capacity Utilization 93.9%

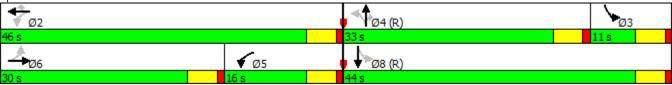
Intersection LOS: C
ICU Level of Service F

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.





	•	•	4	<b>†</b>	<b>↓</b>	1
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	7	7	ř	<b>*</b>	<b>†</b>	7
Traffic Volume (vph)	255	32	19	494	529	233
Future Volume (vph)	255	32	19	494	529	233
Satd. Flow (prot)	1286	1151	1296	1354	1365	1160
Flt Permitted	0.950		0.242			
Satd. Flow (perm)	1286	1128	330	1354	1365	1123
Satd. Flow (RTOR)						
Lane Group Flow (vph)	294	37	22	570	610	269
Turn Type	Prot	Perm	pm+pt	NA	NA	Perm
Protected Phases	4		5	2	6	
Permitted Phases		4	2			6
Total Split (s)	28.0	28.0	11.0	62.0	51.0	51.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0
Act Effct Green (s)	22.3	22.3	57.7	57.7	46.0	46.0
Actuated g/C Ratio	0.25	0.25	0.64	0.64	0.51	0.51
v/c Ratio	0.93	0.13	0.08	0.66	0.88	0.47
Control Delay	49.2	11.7	3.0	14.0	35.6	17.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	49.2	11.7	3.0	14.0	35.6	17.6
LOS	D	В	Α	В	D	В
Approach Delay	45.0			13.5	30.1	
Approach LOS	D			В	С	
Queue Length 50th (ft)	34	4	2	93	291	93
Queue Length 95th (ft)	#294	m10	m5	194	#515	162
Internal Link Dist (ft)	691			571	296	
Turn Bay Length (ft)		150				
Base Capacity (vph)	328	288	284	868	697	573
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.90	0.13	0.08	0.66	0.88	0.47

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 83 (92%), Referenced to phase 2:NBTL and 6:SBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.93

Intersection LOS: C

Intersection Signal Delay: 27.4 Intersection Capacity Utilization 66.5%

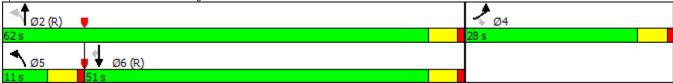
ICU Level of Service C

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Paradise Rd & Loring Ave



	_#	<b>→</b>	7	<b>*</b>	<b>←</b>	٤	•	×	/	6	×	</th
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations		₩		7	f)		ň	<b></b>	7	ř	f)	
Traffic Volume (vph)	3	5	2	382	3	47	5	254	352	44	236	6
Future Volume (vph)	3	5	2	382	3	47	5	254	352	44	236	6
Satd. Flow (prot)	0	1550	0	1296	1126	0	1296	1354	1160	1296	1347	0
Flt Permitted		0.969		0.750			0.590			0.384		
Satd. Flow (perm)	0	1522	0	1024	1126	0	805	1354	1123	524	1347	0
Satd. Flow (RTOR)									*100			
Lane Group Flow (vph)	0	11	0	436	57	0	6	290	402	50	276	0
Turn Type	Perm	NA		Perm	NA		Perm	NA	Perm	pm+pt	NA	
Protected Phases		6			2			4		3	8	
Permitted Phases	6			2			4		4	8		
Total Split (s)	47.0	47.0		47.0	47.0		32.0	32.0	32.0	11.0	43.0	
Total Lost Time (s)		5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Act Effct Green (s)		44.1		44.1	44.1		29.3	29.3	29.3	35.9	35.9	
Actuated g/C Ratio		0.49		0.49	0.49		0.33	0.33	0.33	0.40	0.40	
v/c Ratio		0.01		0.87	0.10		0.02	0.66	0.93	0.19	0.51	
Control Delay		13.1		28.5	5.3		22.7	35.6	54.0	6.1	10.6	
Queue Delay		0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Delay		13.1		28.5	5.3		22.7	35.6	54.0	6.1	10.6	
LOS		В		С	Α		С	D	D	Α	В	
Approach Delay		13.1			25.8			46.1			9.9	
Approach LOS		В			С			D			Α	
Queue Length 50th (ft)		3		201	6		2	146	180	3	20	
Queue Length 95th (ft)		12		#419	m10		11	#259	#374	m8	49	
Internal Link Dist (ft)		69			529			965			691	
Turn Bay Length (ft)				150			100		150	150		
Base Capacity (vph)		746		501	552		261	440	432	260	568	
Starvation Cap Reductn		0		0	0		0	0	0	0	0	
Spillback Cap Reductn		0		0	0		0	0	0	0	0	
Storage Cap Reductn		0		0	0		0	0	0	0	0	
Reduced v/c Ratio		0.01		0.87	0.10		0.02	0.66	0.93	0.19	0.49	

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 57 (63%), Referenced to phase 2:WBTL and 6:EBTL, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.93

Intersection Signal Delay: 31.6

Intersection Capacity Utilization 74.5%

Intersection LOS: C ICU Level of Service D

Analysis Period (min) 15

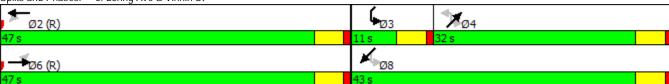
User Entered Value

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Loring Ave & Vinnin St



	-	•	•	<b>←</b>	4	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>^</b>	7		414	7	7
Traffic Volume (vph)	505	183	146	527	185	96
Future Volume (vph)	505	183	146	527	185	96
Satd. Flow (prot)	1450	1232	0	2714	1447	1295
Flt Permitted				0.682	0.950	
Satd. Flow (perm)	1450	1193	0	1871	1427	1295
Satd. Flow (RTOR)						
Lane Group Flow (vph)	558	202	0	743	204	106
Turn Type	NA	Perm	pm+pt	NA	Prot	Prot
Protected Phases	6		5	2	4	4
Permitted Phases		6	2			
Total Split (s)	54.0	54.0	11.0	65.0	25.0	25.0
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0
Act Effct Green (s)	63.3	63.3		63.3	16.7	16.7
Actuated g/C Ratio	0.70	0.70		0.70	0.19	0.19
v/c Ratio	0.55	0.24		0.57	0.76	0.44
Control Delay	5.6	3.3		9.3	52.6	37.6
Queue Delay	1.7	0.6		0.0	1.0	0.0
Total Delay	7.3	3.9		9.3	53.6	37.6
LOS	Α	Α		Α	D	D
Approach Delay	6.4			9.3	48.1	
Approach LOS	Α			Α	D	
Queue Length 50th (ft)	123	24		98	109	53
Queue Length 95th (ft)	m179	m36		160	181	100
Internal Link Dist (ft)	213			175	1023	
Turn Bay Length (ft)						150
Base Capacity (vph)	1019	838		1315	321	287
Starvation Cap Reductn	291	351		0	0	0
Spillback Cap Reductn	0	0		9	24	0
Storage Cap Reductn	0	0		0	0	0
Reduced v/c Ratio	0.77	0.41		0.57	0.69	0.37

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 76 (84%), Referenced to phase 2:WBTL and 6:EBT, Start of Green

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.76

Intersection Signal Delay: 14.7

Intersection LOS: B
ICU Level of Service E

Intersection Capacity Utilization 84.3%

Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 7: Salem St & Vinnin St



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	-	•	•	<b>—</b>	<b>~</b>	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>*</b>	7	ሻ	<b></b>	ሻ	7
Traffic Volume (vph)	535	284	99	575	291	140
Future Volume (vph)	535	284	99	575	291	140
Satd. Flow (prot)	1365	1151	1296	1354	1296	1151
Flt Permitted			0.214		0.950	
Satd. Flow (perm)	1365	1112	292	1354	1296	1151
Satd. Flow (RTOR)						
Lane Group Flow (vph)	604	321	112	649	329	158
Turn Type	NA	Perm	pm+pt	NA	Prot	pt+ov
Protected Phases	4		3	8	2	2 3
Permitted Phases		4	8			
Total Split (s)	47.0	47.0	23.0	70.0	30.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	
Act Effct Green (s)	43.2	43.2	57.0	57.0	25.0	38.8
Actuated g/C Ratio	0.47	0.47	0.62	0.62	0.27	0.42
v/c Ratio	0.94	0.61	0.41	0.77	0.93	0.33
Control Delay	49.3	24.8	11.7	20.5	69.5	20.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	49.3	24.8	11.7	20.5	69.5	20.5
LOS	D	С	В	С	Е	С
Approach Delay	40.8			19.2	53.6	
Approach LOS	D			В	D	
Queue Length 50th (ft)	322	134	25	251	183	59
Queue Length 95th (ft)	#576	238	46	406	#394	119
Internal Link Dist (ft)	1242			539	1673	
Turn Bay Length (ft)		200	100			150
Base Capacity (vph)	640	522	377	957	352	600
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.94	0.61	0.30	0.68	0.93	0.26

Cycle Length: 100 Actuated Cycle Length: 92.1

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.94 Intersection Signal Delay: 36.1 Intersection Capacity Utilization 81.4%

Intersection LOS: D
ICU Level of Service D

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 8: Swampscott Mall Driveway & Essex St



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	*	Ž	<b>~</b>	×	ን	~			
Lane Group	SET	SER	NWL	NWT	NEL	NER	Ø9		
Lane Configurations	<b>^</b>	7	ř	<b>*</b>	W				
Traffic Volume (vph)	700	120	37	826	10	15			
Future Volume (vph)	700	120	37	826	10	15			
Satd. Flow (prot)	1801	1531	1711	1801	1622	0			
Flt Permitted			0.261		0.981				
Satd. Flow (perm)	1801	1531	470	1801	1622	0			
Satd. Flow (RTOR)									
Lane Group Flow (vph)	799	137	42	943	28	0			
Turn Type	NA	Perm	Perm	NA	Prot				
Protected Phases	6			2	4		9		
Permitted Phases		6	2						
Total Split (s)	62.0	62.0	62.0	62.0	13.0		25.0		
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0				
Act Effct Green (s)	43.5	43.5	43.5	43.5	10.5				
Actuated g/C Ratio	0.86	0.86	0.86	0.86	0.21				
v/c Ratio	0.52	0.10	0.10	0.61	0.08				
Control Delay	7.5	4.5	6.0	9.6	31.3				
Queue Delay	0.0	0.0	0.0	0.0	0.0				
Total Delay	7.6	4.5	6.0	9.6	31.3				
LOS	Α	Α	Α	Α	С				
Approach Delay	7.1			9.4	31.3				
Approach LOS	А			Α	С				
Queue Length 50th (ft)	0	0	0	0	6				
Queue Length 95th (ft)	502	64	29	#771	44				
Internal Link Dist (ft)	486			296	259				
Turn Bay Length (ft)		150	150						
Base Capacity (vph)	1621	1378	423	1621	337				
Starvation Cap Reductn	51	0	0	0	0				
Spillback Cap Reductn	0	0	0	0	0				
Storage Cap Reductn	0	0	0	0	0				
Reduced v/c Ratio	0.51	0.10	0.10	0.58	0.08				

Cycle Length: 100 Actuated Cycle Length: 50.6

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.61 Intersection Signal Delay: 8.6

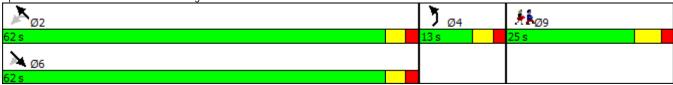
Intersection Signal Delay: 8.6 Intersection LOS: A Intersection Capacity Utilization 60.6% ICU Level of Service B

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 9: Harrison Rd & Loring Ave



	M	<b>†</b>	<b>↓</b>	لر	<b>*</b>	4		
Lane Group	NBL	NBT	SBT	SBR	NEL	NER	Ø9	
Lane Configurations		ર્ન	ĥ		N/			
Traffic Volume (vph)	10	826	800	30	35	20		
Future Volume (vph)	10	826	800	30	35	20		
Satd. Flow (prot)	0	1799	1792	0	1659	0		
Flt Permitted		0.988			0.969			
Satd. Flow (perm)	0	1779	1792	0	1659	0		
Satd. Flow (RTOR)								
Lane Group Flow (vph)	0	954	947	0	63	0		
Turn Type	Perm	NA	NA		Prot			
Protected Phases		2	6		4		9	
Permitted Phases	2							
Total Split (s)	54.0	54.0	54.0		12.0		24.0	
Total Lost Time (s)		6.0	5.0		5.0			
Act Effct Green (s)		55.6	56.4		6.9			
Actuated g/C Ratio		0.75	0.76		0.09			
v/c Ratio		0.71	0.69		0.41			
Control Delay		13.9	12.7		41.6			
Queue Delay		0.5	0.0		0.0			
Total Delay		14.3	12.7		41.6			
LOS		В	В		D			
Approach Delay		14.3	12.7		41.6			
Approach LOS		В	В		D			
Queue Length 50th (ft)		172	154		24			
Queue Length 95th (ft)		#805	#779		#77			
Internal Link Dist (ft)		486	689		323			
Turn Bay Length (ft)								
Base Capacity (vph)		1337	1366		159			
Starvation Cap Reductn		100	0		0			
Spillback Cap Reductn		0	0		0			
Storage Cap Reductn		0	0		0			
Reduced v/c Ratio		0.77	0.69		0.40			

Cycle Length: 90

Actuated Cycle Length: 73.9 Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.71 Intersection Signal Delay: 14.4

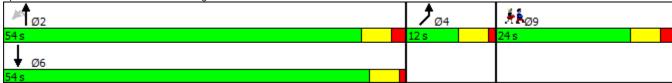
Intersection Signal Delay: 14.4 Intersection Capacity Utilization 68.2% Intersection LOS: B
ICU Level of Service C

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.





	-	74	~	←	•	4
Movement	EBT	EBR	WBL	WBT	NWL	NWR
Lane Configurations	<b>†</b>	7	ሻ	<b>†</b>	W	
Traffic Volume (veh/h)	722	23	79	642	16	123
Future Volume (Veh/h)	722	23	79	642	16	123
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	782	25	86	695	17	133
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)	INOTIC			INOTIC		
Upstream signal (ft)				922		
pX, platoon unblocked				722	0.75	
vC, conflicting volume			807		1649	782
vC1, stage 1 conf vol			007		1047	702
vC2, stage 2 conf vol						
vCu, unblocked vol			807		1697	782
			4.1		6.4	6.2
tC, single (s) tC, 2 stage (s)			4.1		0.4	0.2
			2.2		2.5	2.2
tF (s)			2.2		3.5	3.3
p0 queue free %			89		75	66
cM capacity (veh/h)			818		69	394
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NW 1	
Volume Total	782	25	86	695	150	
Volume Left	0	0	86	0	17	
Volume Right	0	25	0	0	133	
cSH	1700	1700	818	1700	256	
Volume to Capacity	0.46	0.01	0.11	0.41	0.59	
Queue Length 95th (ft)	0	0	9	0	84	
Control Delay (s)	0.0	0.0	9.9	0.0	37.1	
Lane LOS			Α		Е	
Approach Delay (s)	0.0		1.1		37.1	
Approach LOS					Е	
Intersection Summary						
Average Delay			3.7			
Intersection Capacity Utilization			63.4%	IC.	U Level of	Service
Analysis Period (min)			15	10	O LCVCI OI	OCI VICC
Analysis r chou (IIIIII)			10			

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	4	لر	•	×	×	₹
Movement	SBL	SBR	NEL	NET	SWT	SWR
Lane Configurations	₩.	JUIN	INLL	4	<u> </u>	JVIN
Traffic Volume (veh/h)	51	14	12	<b>44</b> 7	460	65
Future Volume (Veh/h)	51	14	12	447	460	65
Sign Control	Stop	14	12	Free	Free	03
Grade	0%			0%	0%	
Peak Hour Factor	0.95	0.93	0.95	0.95	0.95	0.95
					508	
Hourly flow rate (vph)	56	16	13	494	508	72
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (ft)				674		
pX, platoon unblocked	0.88					
vC, conflicting volume	1064	544	580			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1005	544	580			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	76	97	99			
cM capacity (veh/h)	233	539	994			
Direction, Lane #	SB 1	NE 1	SW 1			
Volume Total	72	507	580			
Volume Left	56	13	0			
Volume Right	16	0	72			
cSH	266	994	1700			
Volume to Capacity	0.27	0.01	0.34			
Queue Length 95th (ft)	27	1	0			
Control Delay (s)	23.5	0.4	0.0			
Lane LOS	С	Α				
Approach Delay (s)	23.5	0.4	0.0			
Approach LOS	С					
Intersection Summary						
Average Delay			1.6			
Intersection Capacity Utilization			45.4%	IC	U Level of	Service
Analysis Period (min)			15	IC	O LCVCI UI	JOI VICC
Analysis Fellou (IIIIII)			10			

	<u> </u>	<b>→</b>	<b>—</b>	1	<b>\</b>	1
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	£		W	
Traffic Volume (veh/h)	60	454	620	80	75	51
Future Volume (Veh/h)	60	454	620	80	75	51
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	68	513	700	90	85	58
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)		140110	TAOTIC			
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	790				1394	745
vC1, stage 1 conf vol	770				1374	140
vC2, stage 2 conf vol						
vCu, unblocked vol	790				1394	745
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)	4.1				0.4	0.2
tF (s)	2.2				3.5	3.3
p0 queue free %	92				3.5 41	3.3 86
cM capacity (veh/h)	830				143	414
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	581	790	143			
Volume Left	68	0	85			
Volume Right	0	90	58			
cSH	830	1700	195			
Volume to Capacity	0.08	0.46	0.73			
Queue Length 95th (ft)	7	0	119			
Control Delay (s)	2.1	0.0	62.0			
Lane LOS	Α		F			
Approach Delay (s)	2.1	0.0	62.0			
Approach LOS			F			
Intersection Summary						
Average Delay			6.7			
Intersection Capacity Utilization			85.6%	IC	U Level of	Service
Analysis Period (min)			15	10	C LCVCI UI	JOI VICC
Analysis Fellou (IIIIII)			13			

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Movement		<b>→</b>	-	4	<b>←</b>	*	4
Lane Configurations	Movement	EBT	EBR	WBL	WBT	NWL	NWR
Traffic Volume (veh/h)         483         99         39         632         51         31           Future Volume (Veh/h)         483         99         39         632         51         31           Sign Control         Free         Free         Stop         Gas         51         31           Sign Control         Free         Free         Stop         Gas         51         31           Grade         0%         0%         0%         0%         0%           Peak Hour Factor         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93							
Future Volume (Veh/h)		483	99	39			31
Sign Control         Free Grade         Free Own							
Grade 0% 0,93 0,93 0,93 0,93 0,93 0,93 0,93 0,93			,,	0,			01
Peak Hour Factor   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.93   0.							
Hourly flow rate (vph)   545   112   44   714   58   35     Pedestrians   Lane Width (ft)     Walking Speed (ft/s)     Percent Blockage     Right turn flare (veh)     Median type   None   None     Median storage veh)     Upstream signal (ft)     VX, platoon unblocked     VC, conflicting volume   657   1403   601     VC1, stage 1 conf vol     VC2, stage 2 conf vol     VC2, stage 2 conf vol     VC2, stage (s)     IF (s)   2.2   3.5   3.3     Po queue free %   95   60   93     CM capacity (veh/h)   931   147   500     Direction, Lane #   EB 1   WB 1   NW 1     Volume Total   657   758   93     Volume Left   0   44   58     Volume Left   0   44   58     Volume Right   112   0   35     CSH   1700   931   200     Volume Logacity   0.39   0.05   0.47     Queue Length 95th (ft)   0   4   56     Control Delay (s)   0.0   1.2   37.8     Approach LOS   E     Intersection Summary     Average Delay   10   10     Intersection Capacity Utilization   80.1%   ICU Level of Service			0.93	0.93			0.93
Pedestrians   Lane Width (ft)   Walking Speed (ft/s)   Percent Blockage   Right turn flare (veh)   Median type   None   None   Median storage veh   Upstream signal (ft)   pX, platoon unblocked   VC, conflicting volume   657							
Lane Width (ft)  Walking Speed (ft/s)  Percent Blockage Right turn flare (veh)  Median type None  Median storage veh)  Upstream signal (ft) pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC4, single (s) tC, single (s) tF (s) 2.2 3.5 3.3 p0 queue free % 95 60 93 cM capacity (veh/h)  Pirection, Lane # EB 1 WB 1 NW 1  Volume Total 657 758 93 Volume Left 0 44 58 Volume Right 112 0 35 cSH 1700 931 200 Volume to Capacity 0.39 0.05 0.47 Queue Length 95th (ft) 0 4 56 Control Delay (s) 0.0 1.2 37.8 Approach LOS E  Intersection Summary  Average Delay Intersection Capacity Utilization  Intersection Capacity Utilization  Intersection Capacity Utilization  Intersection Capacity Utilization  INDICULE Velor of Service		010	112		, , ,	00	00
Walking Speed (ft/s)         Percent Blockage       Right turn flare (veh)         Median type       None       None         Median storage veh)       Upstream signal (ft)       V. platoon unblocked         VC, conflicting volume       657       1403       601         VC1, stage 1 conf vol       657       1403       601         VC2, stage 2 conf vol       657       1403       601         VC, single (s)       4.1       6.4       6.2         C, 2 stage (s)       4.1       6.4       6.2         IF (s)       2.2       3.5       3.3         p0 queue free %       95       60       93         M capacity (veh/h)       931       147       500         Direction, Lane #       EB1       WB 1       NW 1         Volume Total       657       758       93         Volume Left       0       44       58         Volume Right       112       0       35         cSH       1700       931       200         Volume Post (ft)       0       4       56         Control Delay (s)       0.0       1.2       37.8         Lane LOS       A       E							
Percent Blockage   Right turn flare (veh)							
Right turn flare (veh)  Median type None None  Median storage veh)  Upstream signal (ft) pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC2, stage (s) tF (s) 4.1 6.4 6.2 tC, 2 stage (s) tF (s) 2.2 3.5 3.3 p0 queue free % 95 60 93 cM capacity (veh/h) 931 147 500  Direction, Lane # EB 1 WB 1 NW 1  Volume Total 657 758 93 Volume Left 0 44 58 Volume Right 112 0 35 cSH 1700 931 200 Volume to Capacity 0 0.39 0.05 0.47 Queue Length 95th (ft) 0 4 56 Control Delay (s) 0.0 1.2 37.8 Lane LOS A E Approach Delay (s) 0.0 1.2 37.8 Approach LOS E  Intersection Summary  Average Delay Intersection Capacity Utilization  None  A 54  5 2 4 403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1403 601  1404 6.2  1409 6.57  1403 601  1403 601  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2  1404 6.2							
Median type         None         None           Median storage veh)         Upstream signal (ft)           pX, platoon unblocked         VC, conflicting volume         657         1403         601           vC1, stage 1 conf vol         VC2, stage 2 conf vol         VC2, stage 2 conf vol         VC2, stage (s)         4.1         6.4         6.2           tC, 2 stage (s)         TE         2.2         3.5         3.3         3.3         90 queue free %         95         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         93         60         60         93         60         93         60         60         93         60         93         60							
Median storage veh)       Upstream signal (ft)         pX, platoon unblocked       vC, conflicting volume       657       1403       601         vC1, stage 1 conf vol       vC2, stage 2 conf vol       vCu, unblocked vol       657       1403       601         tC, single (s)       4.1       6.4       6.2       6.2         tC, 2 stage (s)       tF (s)       2.2       3.5       3.3         p0 queue free %       95       60       93         cM capacity (veh/h)       931       147       500         Direction, Lane #       EB 1       WB 1       NW 1         Volume Total       657       758       93         Volume Left       0       44       58         Volume Right       112       0       35         CSH       1700       931       200         Volume to Capacity       0.39       0.05       0.47         Queue Length 95th (ft)       0       4       56         Control Delay (s)       0.0       1.2       37.8         Lane LOS       A       E         Approach LOS       E         Intersection Summary         Average		Mono			Mono		
Upstream signal (ft) pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC2, unblocked vol tC, single (s) tF (s) 2.2 3.5 3.3 p0 queue free % 95 60 93 cM capacity (veh/h) 931 147 500  Direction, Lane # EB 1 WB 1 Volume Total Volume Total Volume Right 112 0 35 cSH 1700 931 200 Volume to Capacity 0.39 0.05 0.47 Queue Length 95th (ft) 0 4 56 Control Delay (s) 1.2 37.8 Approach LOS E Intersection Summary Average Delay Intersection Capacity Utilization  657 1403 601 40. 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 41 40. 40. 40. 40. 40. 40. 40. 40. 40. 40.		INOTIC			None		
pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol tC, single (s) tF (s) p0 queue free % p1 queue free % p1 queue free % p2 queue free % p2 queue free % p3 queue free % p4 queue free % p5 queue free % p6 queue free % p7 queue free % p8 queue free % p9 queue free % p9 queue free % p9 queue free % p1 queue free % p1 queue free % p2 queue free % p3 queue free % p4 queue free % p5 queue free % p6 queue fre							
vC, conflicting volume       657       1403       601         vC1, stage 1 conf vol       vC2, stage 2 conf vol         vCu, unblocked vol       657       1403       601         tC, single (s)       4.1       6.4       6.2         tC, 2 stage (s)       5       5       60       93         cM capacity (veh/h)       95       60       93         cM capacity (veh/h)       931       147       500         Direction, Lane #       EB 1       WB 1       NW 1         Volume Total       657       758       93         Volume Left       0       44       58         Volume Right       112       0       35         cSH       1700       931       200         Volume to Capacity       0.39       0.05       0.47         Queue Length 95th (ft)       0       4       56         Control Delay (s)       0.0       1.2       37.8         Lane LOS       A       E         Approach LOS       E         Intersection Summary         Average Delay       2.9         Intersection Capacity Utilization       80.1%       ICU							
VC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol tC, single (s) tF, (s) tF (s) tF (s) tC, 2 stage (s) tF (s) tF (s) tC, 2 stage (s) tF (s) tF (s) tC, 2 stage (s) tC, 3 stage (s) tC, 4 stage (s				457		1402	601
vC2, stage 2 conf vol         vCu, unblocked vol       657       1403       601         tC, single (s)       4.1       6.4       6.2         tC, 2 stage (s)       tF (s)       2.2       3.5       3.3         p0 queue free %       95       60       93         cM capacity (veh/h)       931       147       500         Direction, Lane #       EB 1       WB 1       NW 1         Volume Total       657       758       93         Volume Left       0       44       58         Volume Right       112       0       35         CSH       1700       931       200         Volume to Capacity       0.39       0.05       0.47         Queue Length 95th (ft)       0       4       56         Control Delay (s)       0.0       1.2       37.8         Lane LOS       A       E         Approach LOS       E         Intersection Summary         Average Delay       2.9         Intersection Capacity Utilization       80.1%       ICU Level of Service				037		1403	001
vCu, unblocked vol       657       1403       601         tC, single (s)       4.1       6.4       6.2         tC, 2 stage (s)       tF (s)       2.2       3.5       3.3         p0 queue free %       95       60       93         cM capacity (veh/h)       931       147       500         Direction, Lane #       EB 1       WB 1       NW 1         Volume Total       657       758       93         Volume Left       0       44       58         Volume Right       112       0       35         CSH       1700       931       200         Volume to Capacity       0.39       0.05       0.47         Queue Length 95th (ft)       0       4       56         Control Delay (s)       0.0       1.2       37.8         Lane LOS       A       E         Approach Delay (s)       0.0       1.2       37.8         Approach LOS       E         Intersection Summary         Average Delay       2.9         Intersection Capacity Utilization       80.1%       ICU Level of Service							
tC, single (s) 4.1 6.4 6.2 tC, 2 stage (s) tF (s) 2.2 3.5 3.3 p0 queue free % 95 60 93 cM capacity (veh/h) 931 147 500  Direction, Lane # EB 1 WB 1 NW 1  Volume Total 657 758 93 Volume Left 0 44 58 Volume Right 112 0 35 cSH 1700 931 200 Volume to Capacity 0.39 0.05 0.47 Queue Length 95th (ft) 0 4 56 Control Delay (s) 0.0 1.2 37.8 Lane LOS A E Approach Delay (s) 0.0 1.2 37.8 Approach LOS  Intersection Summary  Average Delay Intersection Capacity Utilization 80.1% ICU Level of Service				457		1402	<b>601</b>
tC, 2 stage (s)  tF (s)							
tF (s) 2.2 3.5 3.3 p0 queue free % 95 60 93 cM capacity (veh/h) 931 147 500  Direction, Lane # EB 1 WB 1 NW 1  Volume Total 657 758 93  Volume Left 0 44 58  Volume Right 112 0 35 cSH 1700 931 200  Volume to Capacity 0.39 0.05 0.47 Queue Length 95th (ft) 0 4 56 Control Delay (s) 0.0 1.2 37.8 Lane LOS A E Approach Delay (s) 0.0 1.2 37.8 Approach LOS  Intersection Summary  Average Delay Intersection Capacity Utilization 80.1%  ICU Level of Service				4.1		0.4	0.2
p0 queue free %       95       60       93         cM capacity (veh/h)       931       147       500         Direction, Lane #       EB 1       WB 1       NW 1         Volume Total       657       758       93         Volume Left       0       44       58         Volume Right       112       0       35         cSH       1700       931       200         Volume to Capacity       0.39       0.05       0.47         Queue Length 95th (ft)       0       4       56         Control Delay (s)       0.0       1.2       37.8         Lane LOS       A       E         Approach Delay (s)       0.0       1.2       37.8         Approach LOS       E         Intersection Summary         Average Delay       2.9         Intersection Capacity Utilization       80.1%       ICU Level of Service				2.2		2 E	2.2
CM capacity (veh/h)         931         147         500           Direction, Lane #         EB 1         WB 1         NW 1           Volume Total         657         758         93           Volume Left         0         44         58           Volume Right         112         0         35           cSH         1700         931         200           Volume to Capacity         0.39         0.05         0.47           Queue Length 95th (ft)         0         4         56           Control Delay (s)         0.0         1.2         37.8           Lane LOS         A         E           Approach Delay (s)         0.0         1.2         37.8           Approach LOS         E           Intersection Summary           Average Delay         2.9           Intersection Capacity Utilization         80.1%         ICU Level of Service							
Direction, Lane #         EB 1         WB 1         NW 1           Volume Total         657         758         93           Volume Left         0         44         58           Volume Right         112         0         35           cSH         1700         931         200           Volume to Capacity         0.39         0.05         0.47           Queue Length 95th (ft)         0         4         56           Control Delay (s)         0.0         1.2         37.8           Lane LOS         A         E           Approach Delay (s)         0.0         1.2         37.8           Approach LOS         E           Intersection Summary           Average Delay         2.9           Intersection Capacity Utilization         80.1%         ICU Level of Service							
Volume Total         657         758         93           Volume Left         0         44         58           Volume Right         112         0         35           cSH         1700         931         200           Volume to Capacity         0.39         0.05         0.47           Queue Length 95th (ft)         0         4         56           Control Delay (s)         0.0         1.2         37.8           Lane LOS         A         E           Approach Delay (s)         0.0         1.2         37.8           Approach LOS         E           Intersection Summary           Average Delay         2.9           Intersection Capacity Utilization         80.1%         ICU Level of Service						147	500
Volume Left         0         44         58           Volume Right         112         0         35           cSH         1700         931         200           Volume to Capacity         0.39         0.05         0.47           Queue Length 95th (ft)         0         4         56           Control Delay (s)         0.0         1.2         37.8           Lane LOS         A         E           Approach Delay (s)         0.0         1.2         37.8           Approach LOS         E           Intersection Summary           Average Delay         2.9           Intersection Capacity Utilization         80.1%         ICU Level of Service							
Volume Right         112         0         35           cSH         1700         931         200           Volume to Capacity         0.39         0.05         0.47           Queue Length 95th (ft)         0         4         56           Control Delay (s)         0.0         1.2         37.8           Lane LOS         A         E           Approach Delay (s)         0.0         1.2         37.8           Approach LOS         E           Intersection Summary         E           Intersection Capacity Utilization         80.1%         ICU Level of Service							
1700   931   200							
Volume to Capacity         0.39         0.05         0.47           Queue Length 95th (ft)         0         4         56           Control Delay (s)         0.0         1.2         37.8           Lane LOS         A         E           Approach Delay (s)         0.0         1.2         37.8           Approach LOS         E           Intersection Summary           Average Delay         2.9           Intersection Capacity Utilization         80.1%         ICU Level of Service							
Queue Length 95th (ft)         0         4         56           Control Delay (s)         0.0         1.2         37.8           Lane LOS         A         E           Approach Delay (s)         0.0         1.2         37.8           Approach LOS         E           Intersection Summary         E           Average Delay         2.9           Intersection Capacity Utilization         80.1%         ICU Level of Service							
Control Delay (s)         0.0         1.2         37.8           Lane LOS         A         E           Approach Delay (s)         0.0         1.2         37.8           Approach LOS         E           Intersection Summary           Average Delay         2.9           Intersection Capacity Utilization         80.1%         ICU Level of Service							
Lane LOS A E Approach Delay (s) 0.0 1.2 37.8 Approach LOS E  Intersection Summary Average Delay Intersection Capacity Utilization 80.1% ICU Level of Service							
Approach Delay (s) 0.0 1.2 37.8 Approach LOS E  Intersection Summary  Average Delay 2.9 Intersection Capacity Utilization 80.1% ICU Level of Service		0.0		37.8			
Approach LOS E  Intersection Summary  Average Delay 2.9 Intersection Capacity Utilization 80.1% ICU Level of Service				_			
Intersection Summary  Average Delay Intersection Capacity Utilization  2.9 Intersection Capacity Utilization  80.1%  ICU Level of Service	Approach Delay (s)	0.0	1.2	37.8			
Average Delay  2.9 Intersection Capacity Utilization  80.1%  ICU Level of Service	Approach LOS			Е			
Average Delay  Intersection Capacity Utilization  2.9  ICU Level of Service	Intersection Summary						
Intersection Capacity Utilization 80.1% ICU Level of Service				2.9			
Analysis Doring (min)					IC	U Level of	Service
AUGIVAIA ECHUU IIIIIIII	Analysis Period (min)			15	.0		

Warrants Summary Page 1 of 2

				Warra	ants	Summ	arv						
Information							<u>J</u>						
Analyst Agency/Co Date Performed Project ID East/West Street File Name	re	Intersection  Jurisdiction Units Time Period Analyzed North/South Street  Tedesco St and Leggs Hi Rd Marblehead U.S. Customary PM Leggs Hill Rd East-West											
Project Description Route 1A-Vinnin Square Priorit													
General								Roa	dway l	Networ	k		
Major Street Speed	35	4	Pop	ulation	< 10,0	000		Tw	о Мајо	r Route	s		
(mph) Nearest Signal (ft)	1000		Coo	rdinate	d Sign	al Syste	em	We	ekend	Count			
Crashes (per year)	3		Ade	quate 7	Trials o	of Alterna	atives	5-y	r Grow	th Fact	or		1
		<u> </u>	EB			WB			NB			SB	
Geometry and Traffic		LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of lanes, N		0	1	0	0	1	0	0	0	0	0	1	0
Lane usage Vehicle Volume Average	26		LTR			LTR						LTR	
(vph)  Peds (ped/h) / Gaps		0	556	0	0	541	0	0	0	0	0	142	0
(gaps/h)			0/0			0/0			0/0			0/0	
Delay (s/veh) / (veh-hr)			0/0			0/0			0/0			24.5 / 1.4	
Warrant 1: Eight-Hour	Vehi	cular	Volume										<b>✓</b>
1 A. Minimum Vehicular	Volu	mes (l	Both ma	jor app	roache	esand	highe	r mino	or appro	oach) -	-or		<b>✓</b>
1 B. Interruption of Cont			•										<b>✓</b>
1 (56%) Vehicularand-	Inte	errupti	on Volur	nes (B	oth ma	ajor appr	oaches	and	high	er mino	r appr	oach)	<b>✓</b>
Warrant 2: Four-Hour	Vehic	cular \	/olume										<b>✓</b>
2 A. Four-Hour Vehicula	r Vol	umes	(Both m	ajor ap	proach	nesand	d high	er mir	or app	roach)			<b>✓</b>
Warrant 3: Peak Hour													<b>✓</b>
3 A. Peak-Hour Condition	ns (N	/linor o	delaya	nd mi	inor vo	lumea	and tot	al vol	ume ) -	-or			<b>✓</b>
3 B. Peak- Hour Vehicul	ar Vo	lumes	(Both n	najor a _l	pproac	chesar	nd high	ner m	inor ap	proach)	)		<b>✓</b>
Warrant 4: Pedestrian	Volu	me											
4 A. Four Hour Volumes	or-	-											
4 B. One-Hour Volumes													
Warrant 5: School Cros	ssing	9											
5. Student Volumesand													
5. Gaps Same Period													
Warrant 6: Coordinated Signal System													
6. Degree of Platooning (Predominant direction or both directions)													
Warrant 7: Crash Experience													
7 A. Adequate trials of alternatives, observance and enforcement failedand													
7 B. Reported crashes s	usce	ptible	to corre	ction by	/ signa	ıl (12-mo	onth per	iod) -	and				

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7 C. (56%) Volumes for Warrants 1A, 1Bor 4 are satisfied	<b>✓</b>
Warrant 8: Roadway Network	
8 A. Weekday Volume (Peak hour totaland projected warrants 1, 2 or 3)or	
8 B. Weekend Volume (Five hours total)	
Warrant 9: Grade Crossing	
9 A. Grade Crossing within 140 ftand	
9 B. Peak-Hour Vehicular Volumes	

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# **APPENDIX G**

MassDOT Highway Division Project Development Process

#### **Overview of the Project Development Process**

Transportation decision-making is complex and can be influenced by legislative mandates, environmental regulations, financial limitations, agency programmatic commitments, and partnering opportunities. Decision-makers and reviewing agencies, when consulted early and often throughout the project development process, can ensure that all participants understand the potential impact these factors can have on project implementation. Project development is the process that takes a transportation improvement from concept through construction.

The MassDOT Highway Division has developed a comprehensive project development process which is contained in Chapter 2 of the *MassDOT Highway Division's Project Development and Design Guide*. The eight-step process covers a range of activities extending from identification of a project need, through completion of a set of finished contract plans, to construction of the project. The sequence of decisions made through the project development process progressively narrows the project focus and, ultimately, leads to a project that addresses the identified needs. The descriptions provided below are focused on the process for a highway project, but the same basic process will need to be followed for non-highway projects as well.

#### 1. Needs Identification

For each of the locations at which an improvement is to be implemented, MassDOT leads an effort to define the problem, establishes project goals and objectives, and defines the scope of the planning needed for implementation. To that end, it has to complete a Project Need Form (PNF), which states in general terms the deficiencies or needs related to the transportation facility or location. The PNF documents the problems and explains why corrective action is needed. For this study, the information defining the need for the project will be drawn primarily, perhaps exclusively, from the present report. Also, at this point in the process, MassDOT meets with potential participants, such as the Metropolitan Planning Organization (MPO) and community members, to allow for an informal review of the project.

The PNF is reviewed by the MassDOT Highway Division district office whose jurisdiction includes the location of the proposed project. MassDOT also sends the PNF to the MPO, for informational purposes. The outcome of this step determines whether the project requires further planning, whether it is already well supported by prior planning studies, and, therefore, whether it is ready to move forward into the design phase, or whether it should be dismissed from further consideration.

#### 2. Planning

This phase will likely not be required for the implementation of the improvements proposed in this planning study, as this planning report should constitute the outcome of this step. However, in general, the purpose of this implementation step is for the project proponent to identify issues, impacts, and approvals that may need to be obtained, so that the subsequent design and permitting processes are understood.

The level of planning needed will vary widely, based on the complexity of the project. Typical tasks include: define the existing context, confirm project need, establish goals and objectives, initiate public outreach, define the project, collect data, develop and analyze alternatives, make recommendations, and provide documentation. Likely outcomes include consensus on the project definition to enable it to move forward into environmental documentation (if needed) and design, or a recommendation to delay the project or dismiss it from further consideration.

#### 3. Project Initiation

At this point in the process, the proponent, MassDOT Highway Division, fills out a Project Initiation Form (PIF) for each improvement, which is reviewed by its Project Review Committee (PRC) and the MPO. The PRC is composed of the Chief Engineer, each District Highway Director, and representatives of the Project Management, Environmental, Planning, Right-of-Way, Traffic, and Bridge departments, and the MassDOT Federal Aid Program Office (FAPO). The PIF documents the project type and description, summarizes the project planning process, identifies likely funding and project management responsibility, and defines a plan for interagency and public participation. First the PRC reviews and evaluates the proposed project based on the MassDOT's statewide priorities and criteria. If the result is positive, MassDOT Highway Division moves the project forward to the design phase, and to programming review by the MPO. The PRC may provide a Project Management Plan to define roles and responsibilities for subsequent steps. The MPO review includes project evaluation based on the MPO's regional priorities and criteria. The MPO may assign project evaluation criteria score, a Transportation Improvement Program (TIP) year, a tentative project category, and a tentative funding category.

## 4. Environmental Permitting, Design, and Right-of-Way Process

This step has four distinct but closely integrated elements: public outreach, environmental documentation and permitting (if required), design, and right-of-way acquisition (if required). The outcome of this step is a fully designed and permitted project ready for construction. However, a project does not have to be fully designed in order for the MPO to program it in the TIP. The sections below provide more detailed information on the four elements of this step of the project development process.

#### Public Outreach

Continued public outreach in the design and environmental process is essential to maintain public support for the project and to seek meaningful input on the design elements. The public outreach is often in the form of required public hearings, but can also include less formal dialogues with those interested in and affected by a proposed project.

#### **Environmental Documentation and Permitting**

The project proponent, in coordination with the Environmental Services section of the MassDOT Highway Division, will be responsible for identifying and complying with all applicable federal, state, and local environmental laws and requirements. This includes determining the appropriate project category for both the Massachusetts Environmental Protection Act (MEPA) and the National Environmental Protection Act (NEPA). Environmental documentation and permitting is often completed in conjunction with the **Preliminary Design** phase described below.

#### Design

There are three major phases of design. The first is **Preliminary Design**, which is also referred to as the 25-percent submission. The major components of this phase include full survey of the project area, preparation of base plans, development of basic geometric layout, development of preliminary cost estimates, and submission of a functional design report. Preliminary Design, although not required to, is often completed in conjunction with the Environmental Documentation and Permitting. The next phase is **Final Design**, which is also referred to as the 75-percent and 100-percent submission. The major components of this phase include preparation of a subsurface exploratory plan (if required), coordination of utility relocations, development of traffic management plans through construction zones, development of final cost estimates, and refinement and finalization of the construction plans. Once Final Design is complete, a full set of **Plans, Specifications, and Estimates (PS&E)** is developed for the project.

## Right-of-Way Acquisition

A separate set of Right-of-Way plans are required for any project that requires land acquisition or easements. The plans must identify the existing and proposed layout lines, easements, property lines, names of property owners, and the dimensions and areas of estimated takings and easements.

## 5. Programming (Identification of Funding)

Programming, which typically begins during the design phase, can actually occur at any time during the process, from planning to design. In this step, which is distinct from project initiation, the proponent requests that the MPO place the project in the region's Transportation Improvement Program (TIP). The proponent requesting the project's listing on the TIP can be the community or it can be one of the MPO member agencies (the Regional Planning Agency, MassDOT, and the Regional Transit Authority). The MPO then considers the project in terms of state and regional needs, evaluation criteria, and compliance with the regional Transportation Plan and decides whether to place it in the draft TIP for public review and then in the final TIP.

#### 6. Procurement

Following project design and programming of a highway project, the MassDOT Highway Division publishes a request for proposals. It then reviews the bids and awards the contract to the qualified bidder with the lowest bid.

#### 7. Construction

After a construction contract is awarded, MassDOT Highway Division and the contractor develop a public participation plan and a management plan for the construction process.

#### 8. Project Assessment

The purpose of this step is to receive constituents' comments on the project development process and the project's design elements. MassDOT Highway Division can apply what is learned in this process to future projects.

## **Project Development Schematic Timetable**

Description	Schedule Influence	Typical Duration
Step I: Problem/Need/Opportunity Identification The proponent completes a Project Need Form (PNF). This form is then reviewed by the MassDOT District office which provides guidance to the proponent on the subsequent steps	The Project Need Form has been developed so that it can be prepared quickly by the proponent, including any supporting data that is readily available. The District office shall return comments	1 to 3 months
of the process.	to the proponent within one month of PNF submission.	
Step II: Planning Project planning can range from agreement that the problem should be addressed through a clear solution to a detailed analysis of alternatives and their impacts.	For some projects, no planning beyond preparation of the Project Need Form is required. Some projects require a planning study centered on specific project issues associated with the proposed solution or a narrow family of alternatives. More complex projects will likely require a detailed alternatives analysis.	Project Planning Report: 3 to 24+ months
Step III: Project Initiation The proponent prepares and submits a Project Initiation Form (PIF) and a Transportation Evaluation Criteria (TEC) form in this step. The PIF and TEC are informally reviewed by the Metropolitan Planning Organization (MPO) and MassDOT District office, and formally reviewed by the PRC.	The PIF includes refinement of the preliminary information contained in the PNF. Additional information summarizing the results of the planning process, such as the Project Planning Report, are included with the PIF and TEC. The schedule is determined by PRC staff review (dependent on project complexity) and meeting schedule.	1 to 4 months
Step IV: Design, Environmental, and Right of Way The proponent completes the project design. Concurrently, the proponent completes necessary environmental permitting analyses and files applications for permits. Any right of way needed for the project is identified and the acquisition process begins.	The schedule for this step is dependent upon the size of the project and the complexity of the design, permitting, and right-of-way issues. Design review by the MassDOT district and appropriate sections is completed in this step.	3 to 48+ months
Step V: Programming The MPO considers the project in terms of its regional priorities and determines whether or not to include the project in the draft Regional Transportation Improvement Program (TIP) which is then made available for public comment. The TIP includes a project description and funding source.	The schedule for this step is subject to each MPO's programming cycle and meeting schedule. It is also possible that the MPO will not include a project in its Draft TIP based on its review and approval procedures.	3 to 12+ months
<b>Step VI: Procurement</b> The project is advertised for construction and a contract awarded.	Administration of competing projects can influence the advertising schedule.	1 to 12 months
Step VII: Construction The construction process is initiated including public notification and any anticipated public involvement. Construction continues to project completion.	The duration for this step is entirely dependent upon project complexity and phasing.	3 to 60+ months
Step VIII: Project Assessment The construction period is complete and project elements and processes are evaluated on a voluntary basis.  Source: Mess DOT Highway Division Project Days	The duration for this step is dependent upon the proponent's approach to this step and any follow-up required.	1 month

Source: MassDOT Highway Division Project Development and Design Guide